#### GENOA CHARTER TOWNSHIP PLANNING COMMISSION PUBLIC HEARING MARCH 12, 2018 MONDAY 6:30 P.M. AGENDA

#### CALL TO ORDER:

PLEDGE OF ALLEGIANCE:

#### APPROVAL OF AGENDA:

# <u>CALL TO THE PUBLIC: (Note: The Board reserves the right to not begin new business after 10:00 p.m.)</u>

#### OLD BUSINESS:

**OPEN PUBLIC HEARING #1**... Review of a special use, site plan and environmental impact assessment for a proposed commercial outdoor display, sales and storage area for a new Family Farm and Home store. The property in question is located in the former TJ Maxx retail space at 3685 E. Grand River Avenue, Howell 48843. The request is petitioned by Family Farm and Home.

Planning Commission Recommendation of Petition:

- A. Recommendation of Special Use Application.
- B. Recommendation of Environmental Impact Assessment (1/4/18).
- C. Recommendation of Site Plan (2/22/18).

#### **NEW BUSINESS:**

**OPEN PUBLIC HEARING #2**...Review of site plan and environmental impact assessment for a proposed 2,254 sq. ft. addition to the existing ALDI Food Market located at 2260 E. Grand River Ave., Howell. The request is petitioned by ALDI Inc.

Planning Commission Recommendation of Petition:

- A. Recommendation of Environmental Impact Assessment (2-23-18)
- B. Disposition of Site Plan (2-21-18)

**OPEN PUBLIC HEARING #3**... Review of site plan amendment to add 4 additional apartment units and relocate the fitness center for Lakeshore Village Apartments located at 2812 Ontario Ct., Howell. The request is petitioned by Coponen Architects.

Planning Commission Disposition of Petition:

A. Disposition of Site Plan (2-22-18)

#### **ADMINISTRATIVE BUSINESS:**

- Staff Report
- Approval of February 12, 2018 Planning Commission meeting minutes
- Member discussion
- Adjournment



## **GENOA CHARTER TOWNSHIP Special Land Use Application**

This application **must** be accompanied by a site plan review application and the associated submittal requirements. (The Zoning Official may allow a less detailed sketch plan for a change in use.)

APPLICANT NAME & ADDRESS: Martin Renel, ASHMARK Construction						
Submit a letter of Authorization from Property Owner if application is signed by Acting Agent.						
APPLICANT PHONE: ( <b>248</b> ) <b>855-1575</b>	EMAIL: martin@ashmark.com					

<b>OWNER NAME &amp; ADDRESS</b>	<ul> <li>ASHMARK Construction IIc,</li> </ul>	5640 West Maple Road Suite 3	0, West Bloomfield, MI 48322

SITE ADDRESS: 3685 E Grand River Ave PARCEL #(s): 4711-05-400-032

OWNER PHONE: <u>248 855-1575</u> EMAIL:

Location and brief description of site and surroundings:

The site is An existing shopping center known as Grand River Plaza located north of S Latson Road on Grand River Road.

The tenant wishes to improve and occupy a retail space once occupied by TJ Maxx.

Proposed Use:

Hard scaping a side yard (7,331 SF OR .17 AC) to the south for a fenced exterior storage area, a reuse of an existing parking area as storage and the removal of a trash compactor enclosure

screen fencing at the existing building entry and new loading dock door The requested outdoor fenced areas are for the purpose of storing products for sale.

Describe how your request meets the Zoning Ordinance General Review Standards (section 19.03):

a. Describe how the use will be compatible and in accordance with the goals, objectives, and policies of the Genoa Township Comprehensive Plan and subarea plans, and will promote the Statement of Purpose of the zoning district in which the use is proposed.

This project is located in a regional commercial district which allows for outdoor storage under 7.02.02 (d) of the Genoa Township Zoning Ordinance. Further this proposal provides 8 foot and 4 foot high screen walls where storage of products are to be stored or displayed. All stored items will not be stored higher than the screen provided and will

not be visible from any residential district or expressway

b. Describe how the use will be designed, constructed, operated, and maintained to be compatible with, and not significantly alter, the existing or intended character of the general vicinity.

The storage area will screen all equipment and products with a fence area as proposed and is in keeping with providing retail offerings in

a regional commercial district

c. How will the use be served adequately by essential public facilities and services such as highways, streets, police and fire protection, drainage structures, water and sewage facilities, refuse disposal and schools?

There will be no change in complete access that currently exists to the site as no changes to vehicular and emergency circulation, drive isle or parking configurations is being requested.

d. Will the use involve any uses, activities, processes, or materials potentially detrimental to the natural environment, public health, safety, or welfare by reason of excessive production of traffic, noise, vibration, smoke, fumes, odors, glare, or other such nuisance? If so, how will the impacts be mitigated?

No

e. Does the use have specific criteria as listed in the Zoning Ordinance (sections 3.03.02, 7.02.02, & 8.02.02)? If so, describe how the criteria are met.

Yes, this proposal fully complies with 7.02.02 (d) as required for outdoor storage areas

I HEREBY CERTIFY THAT ALL INFORMATION AND DATA ATTACHED TO AND MADE PART OF THIS APPLICATION ARE TRUE AND ACCURATE TO THE BEST OF MY KNOWLEDGE AND BELIEF. I AGREE TO DESIGN, CONSTRUCT AND OPERATE, AND MAINTAIN THESE PREMISES AND THE BUILDINGS, STRUCTURES, AND FACILITIES WHICH ARE GOVERNED BY THIS PERMIT IN ACCORDANCE WITH THE STATED REQUIREMENTS OF THE GENOA TOWNSHIP ZONING ORDINANCE, AND SUCH ADDITIONAL LIMITS AND SAFEGUARDS AS MAY BE MADE A PART OF THIS PERMIT.

THE UNDERSIGNED Martin Renel for property ownership STATES THAT THEY ARE THE FREE OWNER OF THE PROPERTY OF PROPERTIES DESCRIBED ABOVE AND MAKES APPLICATION FOR THIS SPECIAL LAND USE PERMIT.

BY:\_\_\_\_\_

ADDRESS: ASHMARK Construction IIc, 5640 West Maple Road Suite 300, West Bloomfield, MI 48322

**Contact Information -** Review Letters and Correspondence shall be forwarded to the following:

Martin Renel

Name

of ASHMARK Construction LLC at martin@ashmark.com Business Affiliation

Email

#### FEE EXCEEDANCE AGREEMENT

As stated on the site plan review fee schedule, all site plans are allocated two (2) consultant reviews and one (1) Planning Commission meeting. If additional reviews or meetings are necessary, the applicant will be required to pay the actual incurred costs for the additional reviews. If applicable, additional review fee payment will be required concurrent with submittal to the Township Board. By signing below, applicant indicates agreement and full understanding of this policy.

SIGNATURE: DATE: January 3, 2018

PRINT NAME: Martin Renel PHONE: 248 855-1575



# **GENOA CHARTER TOWNSHIP** Application for Site Plan Review

### TO THE GENOA TOWNSHIP PLANNING COMMISSION AND TOWNSHIP BOARD:

APPLICANT NAME & ADDRESS: Family Farm and Home
If applicant is not the owner, a letter of Authorization from Property Owner is needed.
OWNER'S NAME & ADDRESS: ASHMARK Construction for Family Farm and Home
SITE ADDRESS: 3685 E Grand River Ave PARCEL #(s):
APPLICANT PHONE: $(248\ 855-1575$ $\blacksquare$ OWNER PHONE: $()$
OWNER EMAIL:
LOCATION AND BRIEF DESCRIPTION OF SITE: The site is An existing shopping center known as
Grand River Plaza located north of S Latson Road on Grand River Road. The tenant wishes to improve a
retail space once occupied by TJ Maxx.
BRIEF STATEMENT OF PROPOSED USE:
foot print is planned. The improvements include the renovation of the interior retail space as well as hard scaping a side yard (7,331 SF OR .17 AC)
to the south for a fenced exterior storage area, a reuse of an existing parking area as storage and the removal of a trash compactor enclosure,
screen fencing at the existing building entry and new loading dock door The requested outdoor fenced areas are for the purpose of storing products for sale.
THE FOLLOWING BUILDINGS ARE PROPOSED:
I HEREBY CERTIFY THAT ALL INFORMATION AND DATA ATTACHED TO AND MADE PART OF THIS APPLICATION IS TRUE AND ACCURATE TO THE BEST OF MY KNOWLEDGE AND BELIEF.
BY:

ADDRESS: ASHMARK Construction IIc, 5640 West Maple Road Suite 300, West Bloomfield, MI 48322

Contact Information - Rev	iew Letters and Correspondence shall be forward	ed to the following:
1.) Martin Renel	of ASHMARK Construction	at martin@ashmark.com
Name	Business Affiliation	E-mail Address

FEE EXCE	CEDANCE AGREEMENT
one (1) Planning Commission meeting. If addit will be required to pay the actual incurred cost	all site plans are allocated two (2) consultant reviews and tional reviews or meetings are necessary, the applicant s for the additional reviews. If applicable, additional review ubmittal to the Township Board. By signing below, anding of this policy. DATE: January 3, 2018
PRINT NAME: Martin Renel	PHONE: 248 855-1575
	uite 300, West Bloomfield, MI 48322

- The petitioner agrees that they will maintain the landscape around the dog walk area to keep the vegetation alive.
- All requirements of the Brighton Area Fire Authority's letter of February 1, 2018 shall be met.
- Township Board approval of the Impact Assessment and sketch plan.

## The motion carried unanimously.

**Moved** by Commissioner McManus, seconded by Commissioner Grajek, to recommend to the Township Board approval of the Environmental Impact Assessment dated January 22, 2018 for Four Seasons Veterinary Services veterinary clinic. The property in question is located at 6936 Grand River Avenue, Brighton, 48114. **The motion carried unanimously.** 

**Moved** by Commissioner McManus, seconded by Commissioner Mortensen, to recommend to the Township Board approval of the Sketch Plan for Four Seasons Veterinary Services veterinary clinic. The property in question is located at 6936 Grand River Avenue, Brighton, 48114, conditioned upon the following:

- The sketch plan is sufficient instead of a Site Plan. This is an existing building with a modest amount of exterior and landscaping changes.
- Six deciduous trees shall to be planted in the spring in the easement along Grand River
- The petitioner will add a hedge row to screen the dogs using the area. The hedge row shall be reviewed and approved by Township staff.
- Township Board approval of the Special Land Use and Impact Assessment. **The motion carried unanimously.**

OPEN PUBLIC HEARING # 3... Review of a special use, site plan and environmental impact assessment for a proposed commercial outdoor display, sales and storage area for a new Family Farm and Home store. The property in question is located in the former TJ Maxx retail space at 3685 E. Grand River Avenue, Howell 48843. The request is petitioned by Family Farm and Home.

Planning Commission Recommendation of Petition:

- A. Recommendation of Special Use Application.
- B. Recommendation of Environmental Impact Assessment (01/04/18).
- C. Recommendation of Site Plan (01/24/18).

Mr. Martin Renel of Ashmark Construction and Robert Kerr of Metro Group Architects were present.

Mr. Renel provided a review of the project and the proposed use. They would be performing minor renovations on the interior of the building. They would be adding an outdoor display area and reconstructing the dumpster enclosure.

Mr. Borden reviewed his letter of February 6, 2018.

They find that the general standards of the Special Land Use have been met.

The screening for the outdoor storage area is required to be six-feet high and the applicant is proposing four-foot high fencing. The Planning Commission can approve the difference.

He is asking for clarification for what the applicant is calling the outdoor staging area. It appears to be an additional outdoor storage area. Mr. Renel stated that since the submittal was made, they have confirmed with Family Farm and Home that this area will be storage so the fencing will need to be extended to include this, which is an additional 19 feet.

There was a discussion regarding the removal of some of the grass area and then the same amount of grass area being replaced by removing parking spaces and adding turf. Commissioner Mortensen questioned if this would affect the parking. Ms. VanMarter stated there is sufficient parking on this site.

Commissioner Mortensen asked that the proposed white PVC fencing being redesigned. Mr. Borden noted that this fencing is also being proposed for the dumpster enclosure area and it is not an approved material. Mr. Eugene Franks of Family Farm and Home stated that there are dumpsters along the back of the building and none of them have enclosures. Mr. Renel stated they could use wood for the enclosure.

Commissioner Rauch asked staff for their experience with other commercial businesses in the Township who have outdoor storage. Ms. VanMarter stated the Township has had continued problems with businesses that have not complied with the ordinance as it pertains to outdoor storage for retail uses. Commissioner Rauch wants to ensure that there is sufficient screening on the front of this building so the same problem does not occur here.

Mr. Borden noted that the architectural plan and the engineering plan are not consistent.

There was a discussion regarding the completion of the cross access driveway to the property to the east. The applicant has declined to finish the driveway to connect the

two properties. Ms. VanMarter stated that the connection is the responsibility of Meijer; however, at the time it was developed, they were not given an easement from Kroger. Mr. Markstrom stated there is a large grade change in this area so significant work would need to be done to connect these two properties.

Mr. Markstrom reviewed his letter dated January 30, 2018. He would like to see flow direction shown for the storm-water runoff. The trade-off of the paving and turf is a viable alternative; however, he suggested the applicant look at the entire site it could possibly be determined that there may be sufficient impervious surface so there may not be a need to remove the parking spaces.

He needs to see a complete site plan with details, such as foundation details, curbing details, etc.

Chairman Brown reviewed the Brighton Area Fire Authority letter dated February 1, 2018. The applicant has agreed to meet all of their requirements.

The call to the public was made at 8:08 pm.

Mr. Eric Unatin with Mid-American Real Estate has been working on this lease. Their experience over the past 18 months since this site has been vacant has been that Family Farm and Home has been the only company interested. He noted the discussion regarding the material for the fencing and the dumpster enclosure and stated there are costs associated for both the property owner and the tenant when a new owner moves into a space and it must be economical for both of them.

The call to the public was closed at 8:12 pm.

Commissioner McManus agrees with what Mr. Unatin said; however, the Planning Commission needs to ensure that the ordinance is met. He agrees that the proposed fencing needs to be upgraded.

**Moved** by Commissioner Mortensen, seconded by Commissioner Rauch, table the request for a special use, site plan and environmental impact assessment for a proposed commercial outdoor display, sales and storage area for a new Family Farm and Home store located at 3685 E. Grand River Avenue, Howell 48843 until the March 12, 2018 meeting.

## The motion carried unanimously.



March 6, 2018

Planning Commission Genoa Township 2911 Dorr Road Brighton, Michigan 48116

Attention:	Kelly Van Marter, AICP
	Planning Director and Assistant Township Manager
Subject:	Family Farm and Home – Special Land Use and Site Plan Review #3
Location:	3685 East Grand River Avenue – north side of Grand River, west of S. Latson Road
Zoning:	RCD Regional Commercial District

Dear Commissioners:

At the Township's request, we have reviewed the revised submittal from Family Farm and Home for special land use (application dated 1/30/18) and site plan (plans dated 2/22/18) review and approval.

We have reviewed the proposal in accordance with the applicable provisions of the Genoa Township Zoning Ordinance.

#### A. Summary

- 1. In our opinion, the special land use standards of Section 19.03 are generally met; however:
  - a. the use conditions of Section 7.02.02(d) must be met; and
  - b. any comments provided by the Township Engineer or Fire Department must be addressed.
- 2. The use conditions of Section 7.02.02(d) are not entirely met. More specifically:
  - a. the setback requirement for the proposed trailer storage area is not provided;
  - b. the Planning Commission may allow screen fencing in lieu of a buffer zone B; and
  - c. the trailer storage area is only screened on 3 sides.
- 3. The applicant must provide impervious surface calculations.
- 4. The applicant must confirm the amount of parking remaining after the removal of spaces to accommodate their project.
- 5. We suggest a cross-access easement be provided on the subject site allowing future connection with the development to the east.
- 6. We recommend the applicant be required to maintain at least 5' of clearance along the front of the building.
- 7. The parking blocks proposed along the south side of the 8' screen fencing reduce the drive aisle width below Ordinance standards and could pose problems for snow plowing.
- 8. At such time as new signage is proposed, the applicant must obtain approval and a permit from the Township prior to installation.

#### B. Proposal/Process

The project entails a new business within an existing multi-tenant commercial center (the space was formerly home to a TJ Maxx store), as well as outdoor display, sales and storage. Table 7.02 lists outdoor commercial display, sales or storage as a special land use in the RCD. Such uses are also subject to the use conditions of Section 7.02.02(d).

Procedurally, the Planning Commission is to review the special land use, site plan and impact assessment and provide a recommendation on each to the Township Board following a public hearing. (A hearing was held at the February 12, 2018 Planning Commission meeting, though the request was tabled.)

Genoa Township Planning Commission Family Farm and Home Special Land Use and Site Plan Review #3 Page 2



Aerial view of site and surroundings (looking north)

#### C. Special Land Use Review

Section 19.03 of the Zoning Ordinance identifies the review criteria for Special Land Use applications as follows:

1. Master Plan. The Township Master Plan identifies the subject site, as well as the adjacent properties along Grand River, as Regional Commercial. This category is intended for "higher intensity commercial uses that serve the comparison shopping needs of the entire community and the regional market."

The description does not mention anything with respect to outdoor display, sales or storage, though it is worth noting that the large commercial store to the east has a similar accessory outdoor element.

- 2. Compatibility. The area is developed with a variety of commercial uses. While outdoor uses do not appear to be overly prevalent in this area, the development adjacent to the east has a similar outdoor component (as noted above). Provided the use conditions for outdoor display, sales and storage are met, the proposal is generally expected to be compatible with the character of the area.
- **3. Public Facilities and Services.** As a developed site along the main commercial corridor through the Township, we anticipate necessary public facilities and services are in place; however, the Commission should consider any comments provided ty the Township Engineer and Brighton Area Fire Department.
- **4. Impacts.** The use conditions of Section 7.02.02(d) are intended to limit on- or off-site impacts of outdoor display, sales and storage. Provided those standards are met, the proposal should not adversely impact adjacent or surrounding properties and/or uses.
- **5. Mitigation.** If additional concerns arise as part of the review process, the Township may require efforts to mitigate potential adverse impacts.

#### **D.** Use Conditions

Commercial outdoor display, sales or storage uses are subject to the following use conditions of Section 7.02.02(d):

#### 1. Minimum lot area shall be one (1) acre.

The subject site contains a total 12.74 acres. This standard is met.

# 2. Any stockpiles of soils, fertilizer or similar loosely packaged materials shall be sufficiently covered or contained to prevent dust or blowing of materials.

The previous submittal noted that any such materials will be kept in the manufacturer's packaging; thus, there will not be any loosely stored materials.

3. All outdoor storage areas shall be paved with a permanent, durable and dustless surface and shall be graded and drained to dispose stormwater without negatively impact adjacent property. The Township Board, following a recommendation of the Planning Commission and the Township Engineer, may approve a gravel surface for all or part of the display or storage area for low intensity activities, upon a finding that neighboring properties and the environment will not be negatively impacted.

The site plan identifies paved surfaces for each of the outdoor storage and display areas – the main storage area (south of the building), the newly proposed trailer storage area, and the display area in front of the building.

4. No outdoor storage shall be permitted in any required yard (setback) of buildings for the district in which the outdoor display, sales or storage use is located. Any approved outdoor sales or display within a parking lot shall meet the required parking lot setback; provided the Planning Commission may require additional landscaping screening or ornamental fencing.

Section 7.03 requires setbacks of 50 (rear) and 20 feet (side). The main storage area provides 43-foot setbacks from both the east (side) and south (side) lot lines. The display area in front of the store is also well outside of required setbacks. However, the newly proposed trailer storage area is located adjacent to the south (side) lot line and does not comply with this standard.

# 5. The site shall include a building of at least five hundred (500) feet of gross floor area for office use in conjunction with the use.

The plans note that the building space for the proposed business contains 28,151 square feet of floor area.

#### 6. All loading and truck maneuvering shall be accommodated on-site.

The site has existing vehicular access from Grand River and no changes are proposed. The plans show a new overhead door and loading area on the east side of the building facing north.

Sheet 2 of the submittal includes a truck turning movement diagram demonstrating that truck maneuvering can be accommodated on-site.

7. All outdoor storage area property lines adjacent to a residential district shall provide a buffer zone A as described in Section 12.02. A buffer zone B shall be provided on all other sides. The Planning Commission may approve a six (6) foot high screen wall or fence, or a four (4) foot high landscaped berm as an alternative.

The areas of the site impacted are to the south, east and west and include 3 separate outdoor storage/display areas – the main storage area south of the building; the trailer storage area further south; and the display area along the front of the building.

None of these areas provide a full buffer zone B, but are all proposed to be screened with privacy fencing of varying heights -8-foot for the main outdoor storage area and 4-foot for the trailer storage and display area in front of the building. The current plans also incorporate brick piers into the fencing to help break up the long stretches of white vinyl.

As noted under this criterion and previously discussed, the Planning Commission may allow screen fencing in lieu of a buffer zone B. (In our opinion, Section 12.02.13 gives the Commission the ability to allow 4-foot fencing.)

Lastly, it should be noted that the trailer storage area is screened only on 3 sides.

8. The height of all material and equipment stored in an outdoor storage area shall not exceed the height of any landscape screening, wall or fence. Boats and recreational vehicles may exceed the height of the fence provided that they are setback from the fence a distance equal to their height. Storage of materials up to the height of the adjacent building wall may be permitted in the rear yard if it is illustrated on the site plan, the rear yard does not abut a residential district or face an expressway, and such storage is confined to within twenty (20) feet of the building.

The special land use application states that "all stored items will not be stored higher than the screen provided."

#### E. Site Plan Review

1. **Dimensional Requirements.** The only dimensional standards affected by the request are setbacks for the outdoor storage area (addressed above) and an increase in impervious surface lot coverage due to the removal of the landscaped area.

A note on the current submittal states that "the applicant proposes to enlarge the existing detention basin to accommodate the increase in impervious area if necessary. Additional information to be provided in the detailed engineering stage."

In our opinion, this information is pertinent to the site plan review and should not be pushed off to a later date. As such, the applicant must provide impervious surface coverage calculations to ensure Ordinance requirements are met.

- **2.** Building Materials and Design. The proposal includes repairs to and painting of the building. The submittal states that the repairs will match the existing building.
- **3. Parking.** The project will result in the loss of 43 spaces (19 for the outdoor storage area and another 24 for the trailer storage). The submittal includes parking calculations noting that the site requires 428 parking spaces.

The current submittal notes that 487 spaces are provided, although the previous submittal noted 447. We request the applicant confirm how many spaces will be provided after the removal of the 43 spaces to ensure the required amount of parking will be provided.

4. **Pedestrian and Vehicular Circulation.** As discussed at the previous Planning Commission meeting, we suggest the applicant be required to provide a cross-access easement for a future connection to the development east of the subject site.

Additionally, the applicant must maintain a clear pedestrian area in front of the building given the presence of outdoor display. We recommend a minimum clearance of at least 5 feet be provided at all times.

Lastly, the plans indicate that the south side of the main outdoor storage area will be lined with parking blocks. Based on the drawing provided, these blocks will reduce the travel lane width to 22', which is deficient for two-way travel, and could pose problems for snow plowing.

- **5.** Landscaping. The submittal identifies two new planters along the south side of the outdoor storage area. The planters will each contain a 10' tall Arborvitae.
- 6. Waste Receptacle and Enclosure. The plan includes a new waste receptacle/enclosure on the east side of the building. The details on Sheet A5.1 note the use of a wooden enclosure (cedar). The current submittal also identifies the concrete base pad, as required by Section 12.04.
- 7. Exterior Lighting. The revised submittal states that existing site lighting will remain and does not identify any new lighting proposed.
- **8.** Signs. Sign details are not provided, although the building elevation drawing (Sheet A5.1) identifies an area for a new wall sign.

When a new sign is desired for the business, the applicant will need to obtain approval and a permit from the Township prior to installation.

**9. Impact Assessment.** The original submittal included an Impact Assessment prepared by Ashmark Construction, LLC (not dated). In summary, the Assessment notes that the project is not anticipated to adversely impact natural features, public services/utilities, surrounding land uses or traffic.

Should you have any questions concerning this matter, please do not hesitate to contact our office. I can be reached by phone at (248) 586-0505, or via e-mail at <u>borden@lslplanning.com</u>.

Respectfully, LSL PLANNING, A SAFEBUILT COMPANY

Brian V. Borden, AICP Planning Manager



March 7, 2018

Ms. Kelly Van Marter Genoa Township 2911 Dorr Road Brighton, MI 48116

#### Re: Family Farm and Home Site Plan Review #3

Dear Ms. Van Marter:

Tetra Tech conducted a third site plan review of the Family Farm and Home proposed site located at 3685 East Grand River Avenue (Grand River Plaza Shopping Center) in Howell. The petitioner is proposing to modify an existing retail outlet to meet the retail needs of a Family Farm and Home store. The submission included site plans dated February 22, 2018.

In the previous planning commission meeting the possibility of a cross access connection with Meijer was discussed. Although the current grades may be difficult to facilitate connection with the existing stub road on the Meijer site, the concept of a cross access drive should be maintained. The Planning Commission should consider requiring an easement from the Family Farm and Home parcel that allows for future construction of the access drive when and if the Meijer parcel proposes additional site improvements.

The petitioner has added several notes referencing a detailed engineering plan stage for submittal of the storm water management improvements and survey and grading information. Since the Township doesn't have this plan review stage in the site plan process we recommend any motion related to the site plan include a contingency that these plans be submitted and approved prior to the land use permit being issued. The biggest concern is the storm water drainage as they have removed the construction of offsetting green space for the area proposed for the outside storage.

The remaining comment from the second site plan review regarding the slope of the proposed concrete area has been addressed in the revised drawings on sheet SP1.0. The plans direct the contractor to appropriately slope the new paved area to the existing storm water catch basins and underground conveyance system. With the addition of this information to the site plan we have no further engineering related concerns.

Please call if you have any questions.

Sincerely,

Gary J. Markstrom, P.E. Unit Vice President

copy: Martin Renel, Ashmark Construction

**BRIGHTON AREA FIRE AUTHORITY** 



615 W. Grand River Ave. Brighton, MI 48116 o: 810-229-6640 f: 810-229-1619

March 1, 2018

Kelly VanMarter Genoa Township 2911 Dorr Road Brighton, MI 48116

RE: Family Farm & Home 3685 E. Grand River Avenue Howell, MI 48843

Dear Kelly:

The Brighton Area Fire Department has reviewed the above mentioned site plan. The plans were received for review on February 27, 2018 and the drawings are dated January 23, 2018 with latest revisions dated February 22, 2018. The project is for the redevelopment of an existing 28,151 square foot Mercantile use occupancy that will changing occupant and renovating the structure to meet their needs. They are also proposing to redevelop a large greenbelt area and 19 parking spaces to be utilized as an outside storage and staging area. The plan review is based on the requirements of the International Fire Code (IFC) 2018 edition.

The applicant contacted the fire authority or clarification and revised the submittal. All concerns identified on the previous submittal are shown to be corrected and or addressed.

The fire authority has no further comments for the project at this time.

Additional comments will be given during the building plan review process (specific to the building plans and occupancy). The applicant is reminded that the fire authority must review the fire protection systems submittals (sprinkler & alarm) prior to permit issuance by the Building Department and that the authority will also review the building plans for life safety requirements in conjunction with the Building Department.

If you have any questions about the comments on this plan review please contact me at 810-229-6640.

Cordially,

Rick Boisvert, CFPS Fire Marshal

# ASHMARK Construction, LLC

## Family Farm & Home Impact Assessment

a. Name(s) and address(es) of person(s) responsible for preparation of the impact assessment and a brief statement of their qualifications.

Martin J. Renel Principal at ASHMARK Construction LLC 5640 West Maple Suite 300 West Bloomfield, MI 48322 28 years of Commercial Development and Construction experience.

b. **Map(s) and written description/analysis of the project site** including all existing structures, manmade facilities, and natural features. The analysis shall also included information for areas within 10 feet of the property. An aerial photograph or drawing may be used to delineate these areas.

See submitted plans by Metro Group Architects detailing these areas.

c. **Impact on natural features:** A written description of the environmental characteristics of the site prior to development and following development, i.e., topography, soils, wildlife, woodlands, mature trees (eight-inch caliper or greater), wetlands, drainage, lakes, streams, creeks or ponds. Documentation by a qualified wetland specialist shall be required wherever the Township determines that there is a potential regulated wetland. Reduced copies of the Existing Conditions Map(s) or aerial photographs may accompany written material.

The current site is an existing shopping center; the tenant is taking over the previous TJ Maxx location. We are requesting approval of creating an outside storage area for Family, Farm & Home Store. We do not intend any impact on the existing environmental conditions.

d. **Impact on stormwater management:** Description of measures to control soil erosion and sedimentation during grading and construction operations and until a permanent ground cover is established. Recommendations for such measures may be obtained from the Livingston County Drain Commission at (517) 546-0040.

We will install soil erosion control as required by Livingston County Drain Commission during construction activities of the exterior storage area. We will insure proper dust control during construction activities.

e. **Impact on surrounding land used:** Description of the types of proposed uses and other man-made facilities, including any project phasing, and an indication of how the proposed use conforms or conflicts with existing and potential development patterns. A description shall be provided of any increases of light, noise or air pollution which could negatively impact adjacent properties.

We do not intend to have any impact on surrounding lands; since the use is not changing.

f. **Impact on public facilities and services:** Describe the number of expected residents, employees, visitors, or patrons, and the anticipated impact on public schools, police protection and fire protection. Letters from the appropriate agencies may be provided, as appropriate.

The impact on public facilities and services shall be consistent with the former use.

g. Impact on public utilities: Describe the method to be used to service the development with water and sanitary sewer facilities, the method to be used to control drainage on the site and from the site, including runoff control during periods of construction. For sites serviced with sanitary sewer, calculations for pre- and post development flows shall be provided in comparison with sewer line capacity. Expected sewage rates shall be provided in equivalents to a single family home. Where septic systems are proposed, documentation or permits from the Livingston County Health Department shall be provided.

Please see Alpine Engineering letter dated January 3<sup>rd</sup> 2018 stating no impact on the existing stormwater system.

5640 W. Maple, Suite 101 West Bloomfield, MI 48322-3717 Phone: (248) 855-1575 Fax: (248) 538-5234 martin@ashmark.com

#### January 4, 2018

h. Storage and handling of any hazardous materials: A description of any hazardous substances expected to be used, stored or disposed of on the site. The information shall describe the type of materials, location within the site and method of containment. Documentation of compliance with federal and state requirements, and a Pollution Incident Prevention Plan (PIPP) shall be submitted,

Outside storage area to consist of: landscape mulch & bark, landscape products, patio stone/rock/gravel, peat/soil/earth products, Pots & Planters.

i. Impact on Traffic and Pedestrians: A description of the traffic volumes to be generated based on national reference documents, such as the most recent edition of the *Institute of Transportation Engineers Trip Generation Manual*, other published studies or actual counts of similar uses in Michigan. A detailed traffic impact study shall be submitted for any site over ten (10) acres in size which would be expected to generate 100 directional vehicle trips (i.e. 100 inbound or 100 outbound trips) during the peak hour of traffic of the generator or on the adjacent streets. The contents of the detailed study shall include:

Description of existing daily and peak hour traffic on adjacent street(s) and a description of any sight distance limitations along the right-of-way frontage of the site.

#### The impact on traffic and Pedestrians shall be consistent with the former use.

Forecasted trip generation of the proposed use for the a.m. and p.m. peak hour and average daily traffic generated.

#### The impact on trip generation shall be consistent with the former use.

 $\Box$  For any project with a completion date beyond one year at the time of site plan approval, the analysis shall also include a scenario analyzing forecasted traffic at date of completion along the adjacent street network using a forecast based either on historic annual percentage increases and/or on expected development in the area.

#### Our Completion date will not be beyond one year of approval.

Projected traffic generated shall be distributed (inbound v. outbound, left turn v. right turn) onto the existing street network to project turning movements at site driveways and nearby intersections. Rationale for the distribution shall be provided.

#### The impact on traffic (inbound/outbound, left/right turns) shall be consistent with the former use.

Capacity analysis at the proposed driveway(s) using the procedures outlined in the most recent edition of the *Highway Capacity Manual* published by the Transportation Research Board. Capacity analyses shall be provided for all street intersections where the excepted traffic will comprise at least five-percent (5%) of the existing intersection capacity.

#### The capacity analysis shall be consistent with the previously approved shopping center.

 $\Box$  Accident data for the previous three (3) years for roadway sections and intersections experiencing congestion or a relatively high accident rate, as determined by the township or staff from the Livingston County Road Commission or Michigan Department of Transportation.

#### N/A – Since this site has been previously approved.

Analysis of any mitigation measures warranted by the anticipated traffic impacts. Where appropriate, documentation shall be provided from the appropriate road agency regarding time schedule for improvements and method of funding.

#### N/A – Since this site has been previously approved.

A map illustrating the location and design of proposed access, including any sight distance limitations, dimensions from adjacent driveways and intersections within 250 feet of the edge of the property frontage, and other data to demonstrate that the driveway(s) will provide safe and efficient traffic operation and be in accordance with Article 15.

#### N/A - Since this site has been previously approved.

j. Special Provisions: General description of any deed restrictions, protective covenants, master deed or association bylaws.

None required.



46892 West Road, Suite 109 Novi, Michigan 48377 Phone: (248) 926-3701 Fax: (248) 926-3765 Web: www.alpine-inc.net

January 2, 2018

Martin Renel Ashmark Construction, LLC. 5640 West Maple Road, Suite 300 West Bloomfield, MI 48322

#### Re: **Proposed Family Farm & Home Interior Renovation** 3685 East Grand River Ave. Howell, MI 48843

Dear Martin:

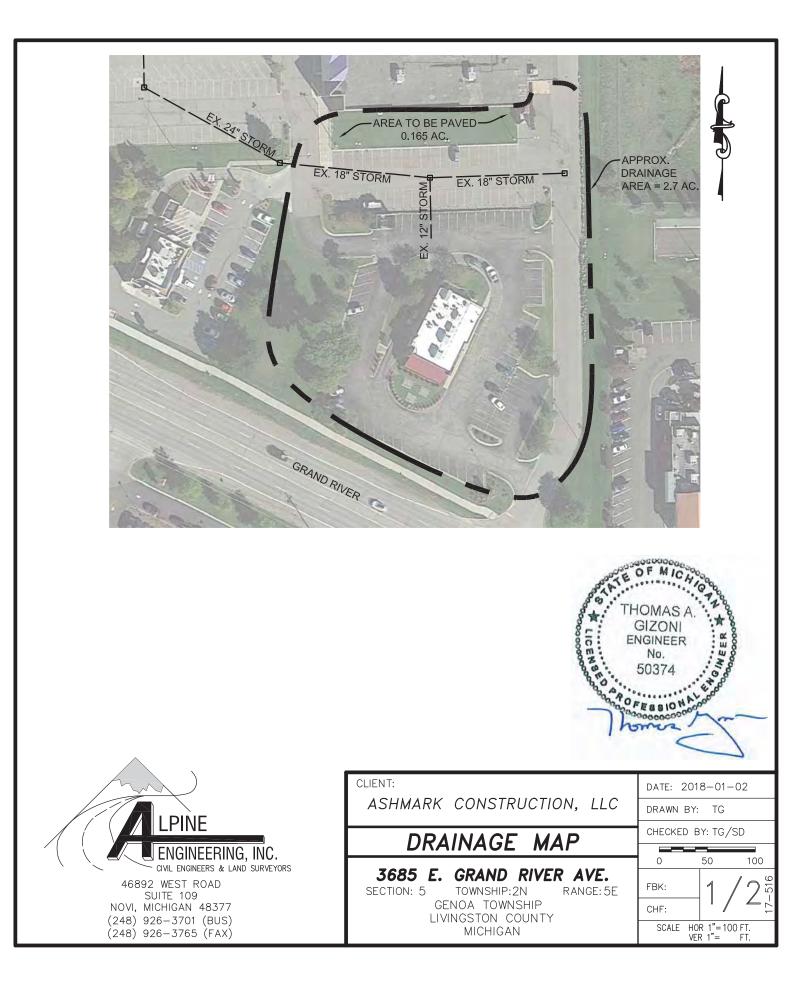
It's our understanding you desire to pave the existing greenbelt area located south of and adjacent to the proposed tenant's building space. There is an existing catch basin located south of the greenbelt area in the parking lot drive aisle which would receive the storm water runoff from the area to be paved. The 1996 Site Plan for the existing development shows an existing 18" diameter storm sewer outlet for the existing catch basin. Per your request we reviewed the existing 18" storm sewer capacity against the proposed paving improvement and found it to have adequate capacity. Drainage map and storm sewer calculation is attached.

If you have any questions or require additional information, please feel free to call our office at (248) 926-3701.

Regards, Alpine Engineering, Inc.

Thomas Gizoni, PE

Attachment: Drainage Map Storm Sewer Calculation



# EXISTING 18" STORM SEWER CAPACITY ANALYSIS

POST-DEVELOPMENT

RUNOFF COEFFICIENT CALCULATION

LAND USE	AREA (A) (acres)	RUNOFF COEFFICIENT ( C )
PAVEMENT	2.11	0.90
GRASS	0.59	0.20
AREA	2.70	

#### CALCULATE THE WEIGHTED RUNOFF COEFFICIENT:

C=SUM (Ai x Ci) / A =  $(2.11 \times 0.90) + (0.59 \times 0.20)$ 2.70

= 0.75

CALCULATE THE 10-YR DESIGN STORM FLOW RATE TRIBUTARY TO THE EXISTING 18" STORM SEWER HEADING WEST:

=

8.82

cfs

Q=C<sub>w</sub> I A

C <sub>w</sub> =	0.75			
=	(175/(T+25))	T=	15	
A=	2.70			

10 YEAR FLOW:

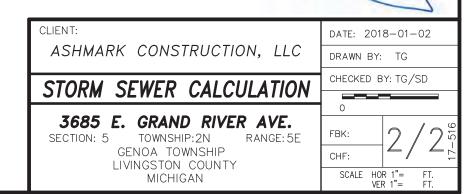
#### EX. 18" PIPE CAPACITY

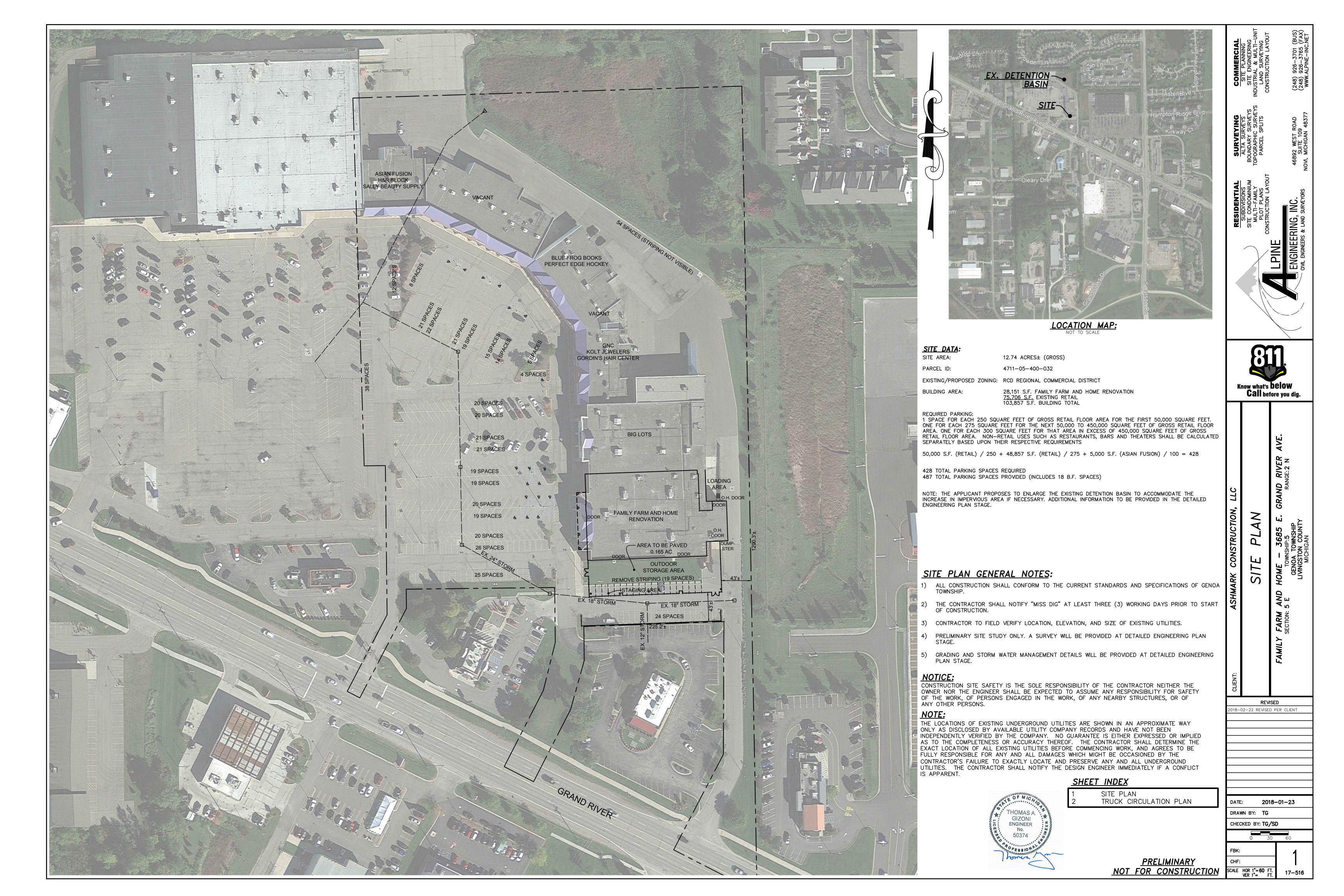
10 YEAR FLOW FROM CALC.'S:	8.82 cfs			
PIPE SIZE:	18	inch		
SLOPE:	0.75	%		
AREA:	1.77	s.f.		
n:	0.013			
CAPACITY:	9.12	c.f.s.		

OK









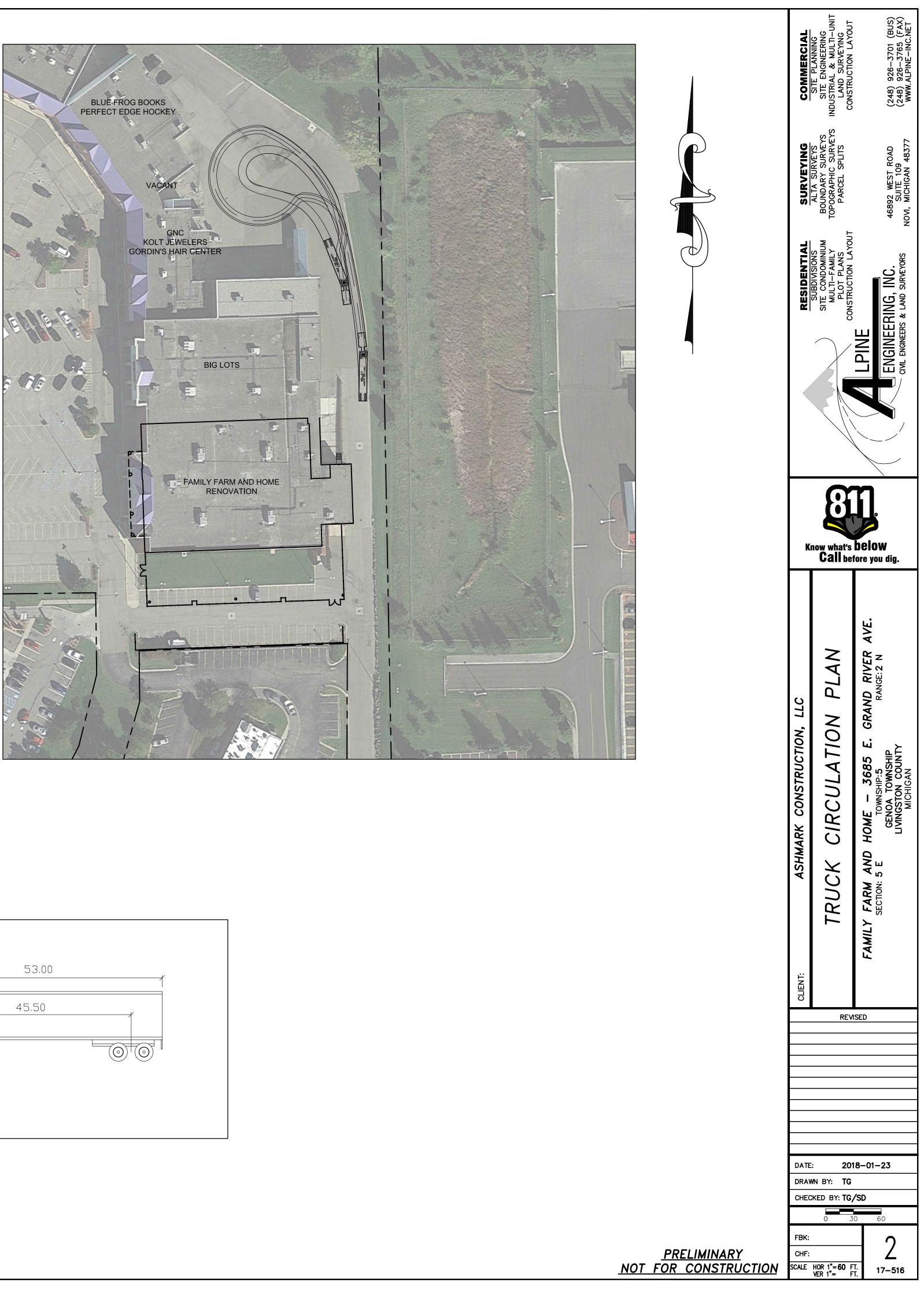


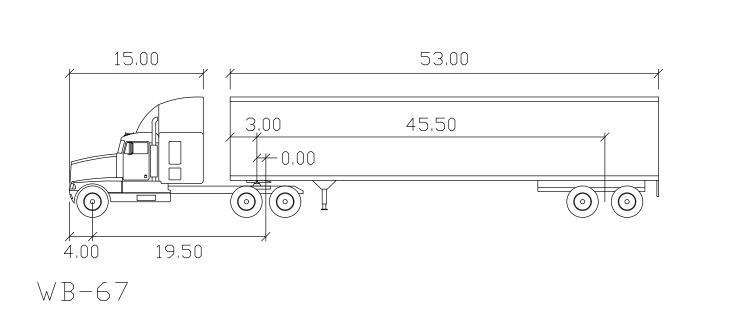
## NOTICE:

CONSTRUCTION SITE SAFETY IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR NEITHER THE OWNER NOR THE ENGINEER SHALL BE EXPECTED TO ASSUME ANY RESPONSIBILITY FOR SAFETY OF THE WORK, OF PERSONS ENGAGED IN THE WORK, OF ANY NEARBY STRUCTURES, OR OF ANY OTHER PERSONS.

## NOTE:

THE LOCATIONS OF EXISTING UNDERGROUND UTILITIES ARE SHOWN IN AN APPROXIMATE WAY ONLY AS DISCLOSED BY AVAILABLE UTILITY COMPANY RECORDS AND HAVE NOT BEEN INDEPENDENTLY VERIFIED BY THE COMPANY. NO GUARANTEE IS EITHER EXPRESSED OR IMPLIED AS TO THE COMPLETENESS OR ACCURACY THEREOF. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL EXISTING UTILITIES BEFORE COMMENCING WORK, AND AGREES TO BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE OCCASIONED BY THE CONTRACTOR'S FAILURE TO EXACTLY LOCATE AND PRESERVE ANY AND ALL UNDERGROUND UTILITIES. THE CONTRACTOR SHALL NOTIFY THE DESIGN ENGINEER IMMEDIATELY IF A CONFLICT IS APPARENT.





# 17 **0** Family Farm & Home Interior Renovation 3685 East Grand River Ave Howell MI 48843 14 **o** Tenant: Family Farm and Home c/o ASHMARK Construction LLC 12 **o** 5640 West Maple, Suite 101 West Bloomfield, MI 48332 P: (248) 855-1575- Martin Renel 110 Contractor : ASHMARK Construction LLC 5640 West Maple, Suite 101 West Bloomfield, MI 48332 P: (248) 855-1575- Martin Renel Architect: Metro Group Architects P.O. Box 7363 Ann Arbor, MI 48107 P: (734) 747-8999 - Robert Kerr 50 4 O 30 2 **o** Location Map 10

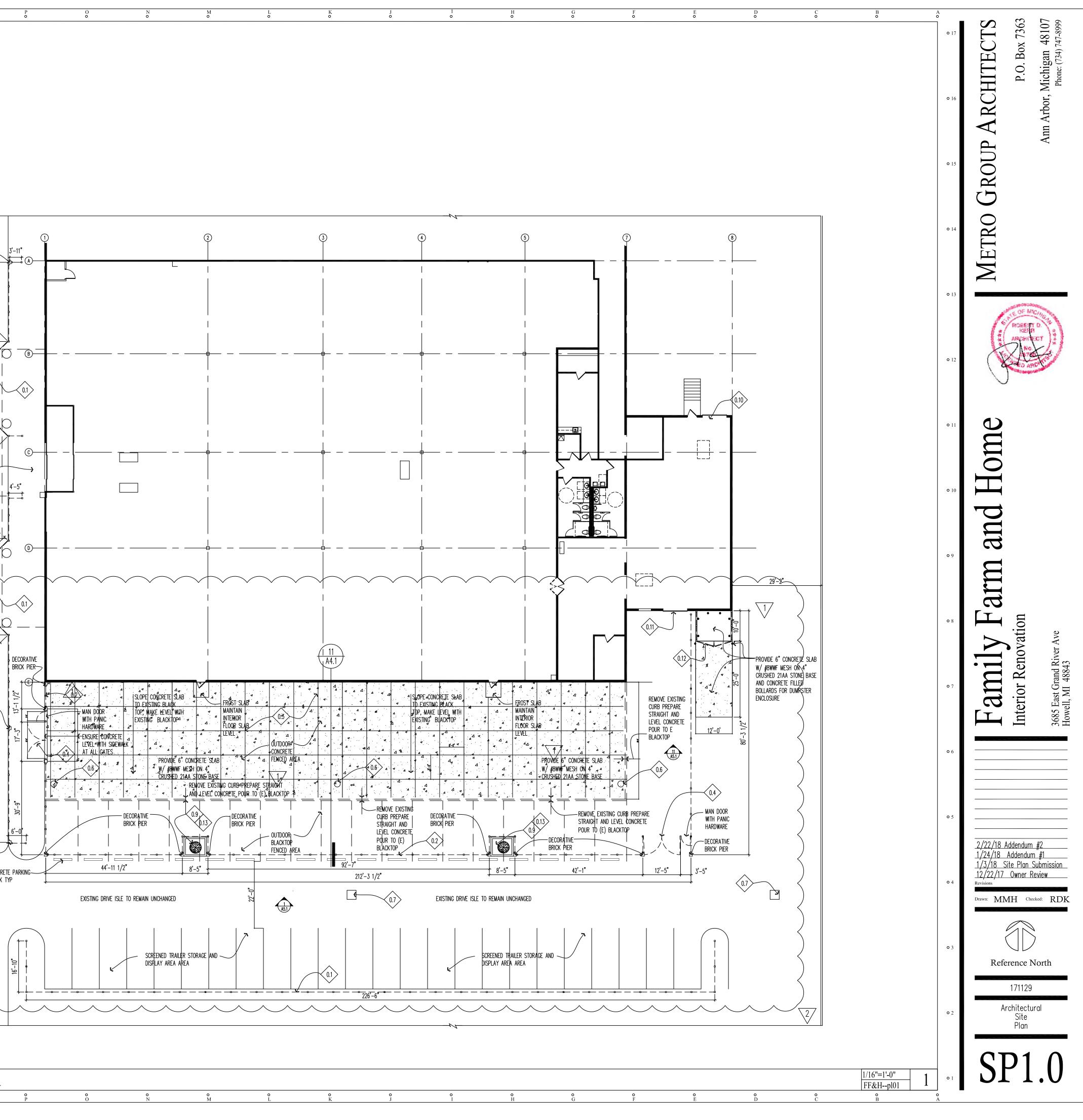
O P

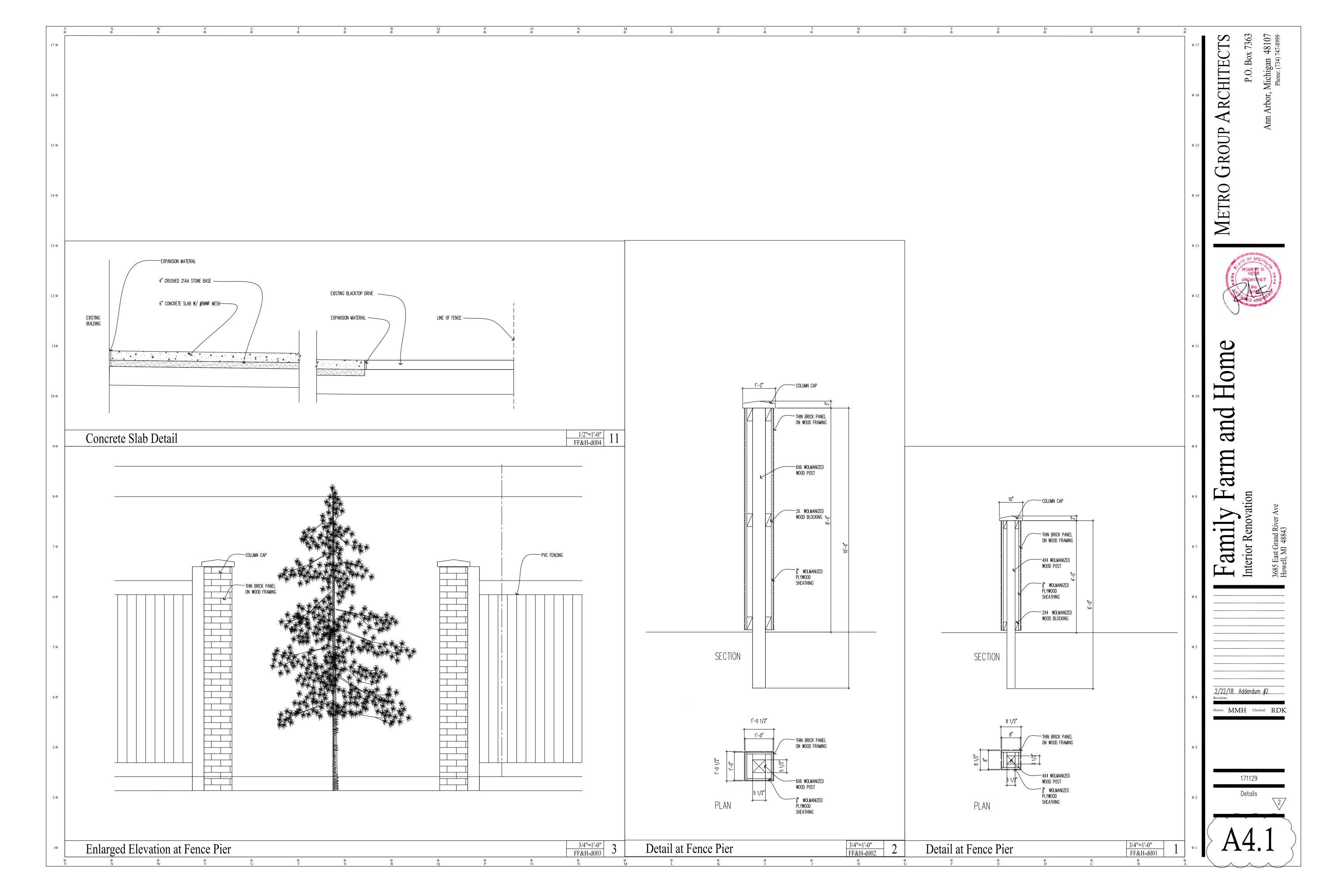
0	N o	M o	L o	K o	J	I 0	H o	G o
	0	At		JB	Junction box		Drav	ving Sheet Index
	A ABV	Ampere Above		JST JT	Joist Joint			
	AC ACOUS	Alternating Current Acoustical		L	Long or Length			
	ADA ADJ	Americans w/Disabilit Adjacent or adjustabl		LAM LAV	Laminate Lavatory			
	AFF AISC	Above finish floor American Institute of		LP	Low Point		SHEET TITLE	
	APPROX ARCH	Approximately Architect(ural)		MAT'L MAX	Material Maximum			nitectural
	ASTM	American Society of	Testing Materials	MECH	Mechanical		A0.0 Cover S	Sheet / Code / Notes
	BD	Board		MTD	Minimum or minute Mounted			ibility Guidelines ectural Site Plan
	BLK BM	Block Beam		M.O.	Masonry Opening			Site Plan Notes and Legends
	BOCA	Building Officials & C Administrator Internat		N N/A	North Not applicable		A1.1 Demoli	tion Plan
	BOT	Bottom		NÉC NECES	National Electrical Co Necessary	de		ioned/Keyed Note Plan ed Ceiling Plan
	CABT CFM	Cabinet Cubic feet per minute	9	NIC NTS	Not in contract Not to scale		A4.1 Details	r Elevations
	CIP CJ	Cast iron pipe or Cas Control Joint		OC	On center			
	CL CLG	Centerline Ceiling		OH OPG	Overhead Opening			
	CNTR CO	Counter Clean out		OPP OS	Opposite Hand Overflow Scupper			
	CPT COL	Carpet Column		PART	Partition			
	CONC	Concrete		PART BD	Particle board			
	CONT CRS	Continuous Carpet Reducer Strip		P. LAM PLBG	Plastic Laminate Plumbing			
	C.T. CW	Ceramic Tile Cold water or Cool w	hite	PR PSF	Pair Pounds per square fo	oot		
	D	Deep		PSI PTD	Pounds per square ir Painted	ich		
	DET DEMO	Detail Demolition		PVC	Polyvinylchloride pipe			
	DR DS	Door Down Spout		Q.T.	Quarry Tile			
	DSS	Down Spout w/ Scup	per	R RA	Riser Return air			
	DRWG	Drawing		REF REJ	Reference			
	EA EF	Each Exhaust Fan		REINF	Roof Expansion Joint Reinforced			
	EJ EIFS	Expansion Joint Exterior Wall Insulatio	n and Finish System	REQ'D RET	Required Return			
	ELEV ELEC	Elevation Electrical		REV RM	Revised Room			
	EMER EQ	Emergency Equal		RO RS	Rough opening Roof Sump			
	EQUIP EXIST or EX	Equipment Existing		RTU	Roof-top unit			
	EXP EXT	Exposed Exterior		S SA	South Supply air			
	FD FFE	Floor Drain Finish Floor Elevation		SECT SF	Section Supply fan or Square	foot		
	FIN FL	Finish		SHT SIM	Sheet Similar	1001		O: Owner C: Contract
	FR	Floor Frame		SPEC	Specifications		Shee	et Index
	FRMG FRP	Framing Fiberglass Reinforced	Panel	STD STRUCT	Standard Structural			
	FT FTG	Foot; Feet Footing		T&B	Top and Bottom		ICC/ANSI A117.1. 2	an Building Code 2015) Effective April 009 & Michigan Barrier Free Design Li an Building Code 2015) – Chapter 13 &
	FV	Field Verify		T&G TEMP	Tongue & Groove Tempered (Glass)		Michigan Uniform I MPC 2015 (Michiga	Energy Code, Part 10a. Rules (ANSI/AS an Plumbing Code 2015) Effective Apri
	G GC	Gas or Gutter General Contractor		TH TOS	Thick Top of Steel		IFGC 2015 (Internat	an Mechanical Code 2015) Effective A tional Fuel Gas Code 2015) Effective A
	GA GFI	Gauge Ground fault interrup	ter	TS TWS	Tube steel Through-wall Scupper	-	IFC 2012 (Internatio	igan Rehabilitation Code for Existing I onal Fire Code 2012), 2015 Michigan I ted at this time by the fire code officia
	GR GYP	Grade Gypsum		TYP	Typical		NEC 2014 (State of	Michigan Electrical Code) 2014 Natio ommercial: NFPA 13 (2013)
	GYP BD	Gypsum board		UL	Underwriters Laborat	ories		rcial: NFPA 72 (2013)
	H HD	Hot or High Fire Alarm Heat Dete	-+	UNFIN UON	Unfinished Unless otherwise not	ed	Building Type Use Group Tenant Area	II-B 5 M A 28,151 G
	HC	Hollow core	CLOP	V	Vent or Volt		SPRIKLERED Common Path of tr	Fully
	HDWE HGT	Hardware Height		VCT VERT	Vinyl composition tile Vertical	9	EXIT Distance Bearing Walls- Exte	
	HM HORIZ	Hollow metal Horizontal		VIF VTR	Verify In Field Vent Through Roof		Bearing Walls-Inter Tenant Separation Roof Assembly	
	HP HT	High Point or Horsep Height		w/	With		Floor Assembly Structural Framing	о н о н
	HVAC HW	Heating, Ventilation & Hot water	c Air Conditioning	WD w/o	Wood Without		Occupant Load	Gross Factor
	IN	Inch		WH NOTE:	Water Heater		Sales Area Restrooms / Offices	60 \$ 30
	INCL	Include Insulation			cal & Electrical Drawings for		Stock Room	300
	INT	Interior Inch per second			breviation listings.			
	DTL	'	DOOR	DOOR NUMBER (ROOM #) . (DOOL	R) (EXISTING)	)	Egress Width Requ	ired Occupants F
	DTL SHT	DTL SHEET WHERE	DOOR BUBBLE	(NOOM #) . (DOO		,		440
tree-		DETAIL IS DRAWN	TH L		(NEW)		Min Number Exits	-
	DTL ELEVATION I	BUBBLE	FIN	DENOTATION			Exit 1 Exit 2	72 36
		DETAIL NUMBER	FLOOR – FINISH BUBBLE	FINISH NUMBER			Exit 3 Exit 4	36 36
	<u>(DTL</u> K) SHT <sub>R</sub>	DIRECTION OF DETAIL SHEET WHERE					SEE SHEET A2.4	180
States and States	DETAIL BU	DETAIL IS DRAWN	WALL BASE				Plumbing Fixtures	-
The second second		ELEVATION NUMBER	FINISH BUBBLE				Merchantile Factor	Wate 
ne	(DTL K) SHJ		<b>W</b>	WALL FINISH DENOTATION			Calculated Occupar Occupants / Factor	nts 220
		SHEET WHERE	WALL FINISH					1
Territoria	ENLARGED		BUBBLE				Dura data d	
A ANTIS HOM		E DRAWING SCALE	FIN	WALL FINISH DENOTATION			Provided Standard Bathroom	3
Ser.	1/4'=1'-0" <sup>K</sup> MGA-dt01		MILLWORK					'
	DETAIL NUN		FINISH BUBBLE		NT. C. 1			
cale 3	Abbi	reviation Inde	X		No Scale FFH_Abbrev	- /		DE REVIEW
<b>o</b> O	o N	o M	o L	o K	o J	o I	o H	o G

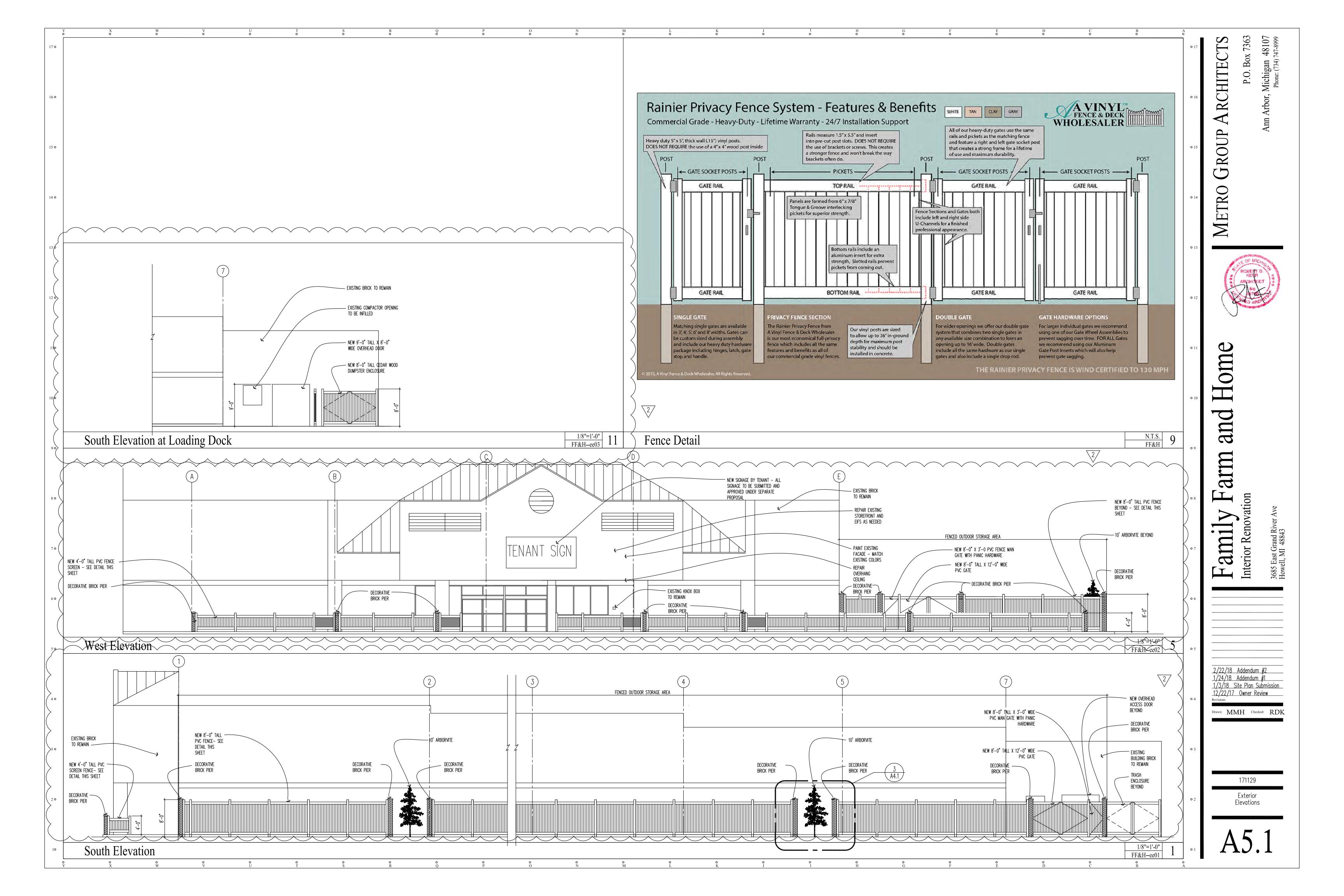
01/03/18 Site Plan 1/24/18 o =		#1	2/22/18 Addendum #2	D •			С о		B o			A • 17 • 16	RCHITECTS	P.O. Box 7363 Ann Arbor, Michigan 48107 Phone: (734) 747-8999
0/C/P/T 0/C/P/T 0/C/P/T 0/C/P/T 0/C/P/T 0/C/P/T	0/C/P/T 0/C/P/T 0/C/P/T 0/C/P/T 0/C/P/T 0/C/P/T	0/C/P/T			· · · · · · · · · · · · · · · · · · ·	· · · · · ·	· · · · · · ·	· · · · · · · · · · · · · · · · · · ·	· · · · ·		· · · · · · · · · · · · · · · · · · ·	• 15 • 14	METRO GROUP ARCHITECTS	Ann /
												• 13 • 12		OF MICHIG THE ROBERT D. KEIR ROBITECT
L: Lar		Т: Теп		Permits (1								• 11 • 10	and Home	
C 2015 (Mik E 90.1-2013 2017 .2, 2017 .0, 2017 ngs 2015) E ng Code rei	chigan Unif4 3) Effective Effective De ferences th de with Par	is amended om Energy September Kember 13, e 2015 IFC (	l. • Code 2015 • 20, 2017 , 2016 (but that do nents Effect	i) – Chapter 4	4 &				No S Detail	Scale I. D.	2	• 9 • 8	arm	Interior Renovation 3685 East Grand River Ave Howell, MI 48843
nklers Tbl nklers Tbl ng Require ng Require ng Require ng Require ng Require 23,774 1,022 2,924	1017.2 ad ad ad ad ad ad ad	0CCUPANT 396 34 10	15	Men 198 17 5	Women 198 17 5	M M M						• 6 • 5		1nt( 3685 Howe
r per Occ (1 0.2 1 -500 ljsted Widt 66 33 33 33 165	nches) Re	440 Total Widt guired (Incl 28.01 2.00 Factor 0.2 0.2 0.2 0.2 0.2	hes) Wid Exits n	220 iided Egress ith (inches) 180 req. Per 1006 330 165 165 165 165 825	1 sets- Do Single 36 Single 36 Single 36	ouble 36" E " Exit Door " Exit Door " Exit Door " Exit Door cpants Prov						o 4	<u>1/24/18 Ad</u> 1/3/18 Site	Plan Submission wner Review
ets Female 500 220 0.44 1 3		Male 750 220 0.29 1	Lavatories 	Female 750 220 0.29 1 2	Shower:	s Founts 1000 440 0.44 1	0 1	k				• 3 • 2	She Loca Abbr	71129 et Index tion Map reviations
								1	I			ı 🔳	A	0.0

	X W V U T S o o o o o o	R Q o o
17 <b>o</b>		
16 <b>o</b>		
15 0		
		<b></b>
14 <b>o</b>		
		DECORATIVE BRICK PIER
13 0	General Site Plan Notes:	26'-1"
	1. PLANS AND SPECIFICATIONS DO NOT FULLY REPRESENT ALL NEW WORK. THE CONSTRUCTION DOCUMENTS ARE INTENDED TO SERVE	5-8 M
	AS GENERAL GUIDELINES. 2. REPORT ANY DISCREPANCIES BETWEEN STRUCTURAL, MECHANICAL, ELECTRICAL, PLUMBING & ARCHITECTURAL DRAWINGS TO	DECORATIVE BRICK PIER
12 <b>o</b>	ARCHITECT IMMEDIATELY. 3. COORDINATE ALL WORK WITH STRUCTURAL, MECHANICAL, PLUMBING AND ELECTRICAL CONSTRUCTION DOCUMENTS, AND WITH	
	APPLICABLE STRUCTURAL, MECHANICAL, PLUMBING AND ELECTRICAL CONTRACTORS. 4. REFERENCE THE FOLLOWING SHEETS: GENERAL CONDITIONS & PLAN SYMBOLS, ACCESSIBILITY GUIDELINES, GENERAL	21,-2"
	SPECIFICATIONS. 5. MASONRY CONTRACTOR SHALL COORDINATE WITH ALL OTHER TRADES	
110	6. SEE STRUCTURAL DRAWINGS FOR STRUCTURAL INFORMATION	DECORATIVE BRICK PIER
	7. SEE MEP DRAWINGS FOR MECHANICAL, ELECTRICAL, & PLUMBING INFORMATION. 8. MASONRY STEEL LINTELS ABOVE DOORS WINDOWS, ETC TO BE SHOP PRIMED & PAINTED TO MATCH COLORS OF FRAMES.	DRIUM PIER 5.
10 0	9. PRIOR TO START OF WORK CONTRACTOR TO VERIFY DIMENSIONS OF BUILDING AND SITE & NOTIFY ARCHITECT IMMEDIATELY OF ANY CONFLICTS.	
	<u>General:</u>	
9 <b>o</b>	0.1 NEW 48" HIGH SCREEN	DECORATIVE
	0.2 NEW 8'-0" HIGH PVC FENCE	$\frac{7}{\sqrt{2}}$
	0.3 NEW CONCRETE PAD FOR EXTERIOR STORAGE AREA	20,
80	0.4 NEW 12'-0" WDE GATE	DECORATIVE BRICK PER
	0.5 REMOVE STRIPING FOR STAGING AREA	BRICK PIER 5.
	0.6 EXISTING 40'-0" HIGH LIGHT POLE	
7 <b>o</b>	CO.7 EXISTING STORM DRAIN	
	0.8 PATCH, REPAIR AND/OR REPLACE ALL DAMAGED SIDEWALK	DECORATIVE BRICK PIER
60	0.9 NEW PLANTER	68'-3 1/4"
	<0.10 EXISTNG LOADING DOCK	
	0.11 NEW LOADING AREA	
50	0.12 NEW DUMPSTER ENCLOSURE	
	<0.13 NEW 10'-0" ARBORVITAE	
		DECORATIVE BRICK PIER
4 <b>o</b>	Legend:	CONCRE
	COLUMN DESIGNATION NEW FENCING	
	NEW WALL	
30	EXISTING LIGHT POLE	
	EXISTING DOOR (TO REMAIN)	
	EXISTING WALL	
20	(TO REMAIN)	
10	Keyed Site Plan Notes	Architectural Site Plan
c		0 0

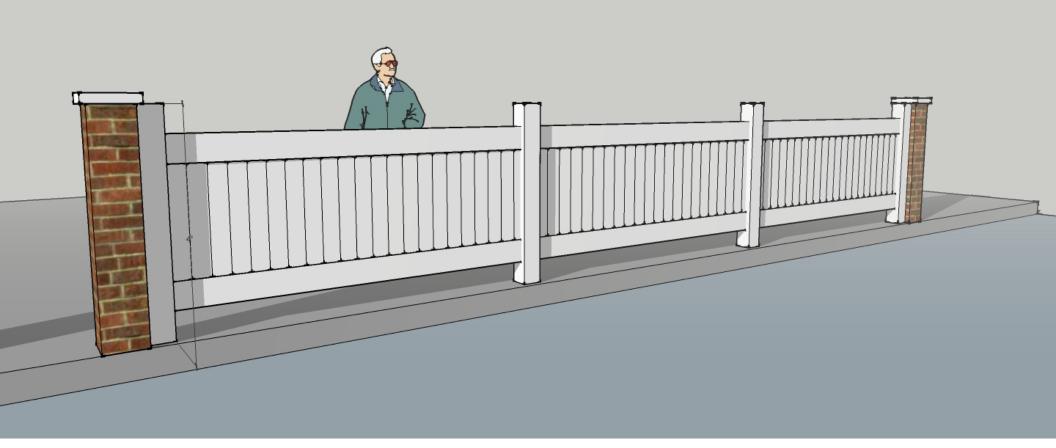


















# **GENOA CHARTER TOWNSHIP Application for Site Plan Review**

## TO THE GENOA TOWNSHIP PLANNING COMMISSION AND TOWNSHIP BOARD:

APPLICANT NAME & ADDRESS: ALDI Inc. 2625 N. Stockbridge Road Webberville, MI 48892 If applicant is not the owner, a letter of Authorization from Property Owner is needed.

OWNER'S NAME & ADDRESS: ALDI Inc. 2625 N. Stockbridge Road Webberville, MI 48892

SITE ADDRESS: 2260 E. Grand River \_\_\_\_\_PARCEL #(s): 4711-06-100-044

APPLICANT PHONE: (517) 521-3907 OWNER PHONE: (517) 521-3907

OWNER EMAIL: sam.glennen@aldi.us

LOCATION AND BRIEF DESCRIPTION OF SITE: Southwest corner of Grand River and Golf Club Road intersection. Site contains an existing

ALDI Food Market, parking area and associated infrastructure.

BRIEF STATEMENT OF PROPOSED USE: Continue existing use as an ALDI Food Market.

THE FOLLOWING BUILDINGS ARE PROPOSED: +/- 2,254 SF addition to east side of existing building.

I HEREBY CERTIFY THAT ALL INFORMATION AND DATA ATTACHED TO AND MADE RT OF THIS APPLICATION IS TRUE AND ACCURATE TO THE BEST OF MY **KNOWLEDGE AND BELIEF.** 

BY: Sam Glennen, Director of Real Estate, ALDI Inc.

ADDRESS: 2625 N. Stockbridge Road Webberville, MI 48892

Contact Information - Review Letters and Correspondence shall be forwarded to the following:

1.) Chris Grzenkowicz

of DESINE Inc. Business Affiliation

at\_chrisg@desineinc.com

E-mail Address

FEE EXCEEDANCE AGREEMENT						
As stated on the site plan review fee schedule, all site plans are allocated two (2) consultant reviews and one (1) Planning Commission meeting. If additional reviews or meetings are necessary, the applicant will be required to pay the actual incurred costs for the additional reviews. If applicable, additional review fee payment will be required concurrent with submittal to the Township Board. By signing below, applicant indicates agreement and full understanding of this policy.						
SIGNATURE: Summer / Ill	Дате: 1-17-)8					
PRINT NAME: Sam Glennen Address: 2625 N. Stockbridge Road Webbe	PHONE: (517) 521-3907					
ADDRESS: 2625 N. Stockbridge Road Webberville, MI 48892						



February 28, 2018

Planning Commission Genoa Township 2911 Dorr Road Brighton, Michigan 48116

Attention:	Kelly Van Marter, AICP				
	Planning Director and Assistant Township Manager				
Subject:	Aldi Food Market – Site Plan Review #2				
Location:	2260 E. Grand River Avenue - south side of Grand River, west of Chilson Road				
Zoning:	GCD General Commercial District				

Dear Commissioners:

At the Township's request, we have reviewed the revised site plan (cover sheet dated 2/21/18) proposing an expansion of the existing Aldi Food Market at 2260 E. Grand River Avenue.

We have reviewed the proposal in accordance with the applicable provisions of the Genoa Township Zoning Ordinance.

#### A. Summary

- 1. Planning Commission has the discretion to waive the side and rear parking setback where a shared access/parking easement is provided -6 of the proposed spaces are within such an easement.
- 2. Planning Commission has approval authority over the building elevations, including materials and colors.
- 3. Planning Commission approval is needed for the amount of parking proposed (125% of the minimum Ordinance standard). The applicant notes that the 4 additional spaces are needed to meet a corporate requirement.

#### B. Proposal

The applicant requests site plan review and approval for a 2,254 square foot addition to the existing 16,657 square foot grocery store.

Retail establishments with up to 30,000 square feet of gross floor area are permitted by right in the GCD. As such, the project requires site plan review/approval given that the size of the expansion (approximately 13%) exceeds the threshold for a "minor" deviation from an approved plan.

Genoa Township Planning Commission Aldi Food Market Site Plan Review #2 Page 2



Aerial view of site and surroundings (looking north)

#### C. Site Plan Review

**1. Dimensional Requirements.** The proposal has been reviewed for compliance with the dimensional standards of the GCD, as follows:

Lot Size			Minimum Setbacks (feet)					
District	Lot Area (acres)	Width (feet)	Front Yard	Side Yard	Rear Yard	Parking	Max. Height	Lot Coverage
GCD	1	150	70	15	50	20 front 10 side/rear	35' 2 stories	35% building 75% impervious
Proposal	2.66	358	91.2	15.4 (W) 135 (E)	57.6	4-29 front 10 side/rear*	24.5' 1 story	16.3% building 61.8% impervious

\* The majority of the existing and proposed parking meets the side/rear setback standard; however, there are 6 spaces proposed at the rear of the site that cross over the property line. This area is noted as being covered by an easement for "parking, landscaping and public utilities." As such, Section 14.06.11 gives the Planning Commission discretion to waive or modify the parking setback requirement.

With the exception of the east side yard, the building and parking setbacks noted are existing conditions. The reduced parking front setback was allowed as part of the original development in 2008 via variance from the ZBA.

**2.** Building Materials and Design. The proposed elevations, including colors and materials, are subject to review and approval by the Planning Commission.

Building materials include brick and a CMU base to match the existing building. New materials include decorative wood panels and metal architectural panels (replacing glass) on the upper level of the store entrance.

In our opinion, the proposed building provides a complementary mix of materials that match the existing building, while also introducing new materials to modernize the overall design.

**3. Parking.** Per Section 14.04, 76 parking spaces are required, while 95 are proposed, including the 5 required barrier free spaces.

The amount of parking proposed slightly exceeds that allowed by Ordinance. More specifically, Section 14.02.06 requires Planning Commission approval for parking that exceeds 120% of the minimum requirement.

The proposal is 125% of the minimum requirement. The applicant has indicated that the 4 additional spaces are needed to meet the current corporate requirement.

The parking spaces and drive aisles meet or exceed the dimensional standards of Section 14.06 and the use of looped striping is proposed.

- 4. Pedestrian Circulation. There is an existing public sidewalk along Grand River, as well as existing walks along the north and east sides of the building. The proposal includes widening of the sidewalk along the east side of the building and crosswalk striping across the parking lot connecting the public and private walks.
- **5.** Vehicular Circulation. The site has direct access to Grand River via a restricted turning movement drive, as well as indirectly by a shared access easement with the County complex to the east. No changes are proposed to the existing circulation patterns.
- **6.** Loading. The development has an existing truck well at the rear of the building for deliveries. The size, design and location are all in accordance with current standards and no changes are proposed.
- 7. Waste Receptacle and Enclosure. There is an existing waste receptacle and enclosure within the truck well at the rear of the building. Our original review of the site plan (2008) indicated that the receptacle/enclosure complied with the standards of Section 12.03 and no changes are currently proposed.
- **8.** Landscaping. We reviewed the landscape plan for compliance with the standards of Section 12.02, as noted in the following table:

Location	Requirements	Proposed	Comments
Greenbelt	20' width	0-29' width (existing)	Requirements met
	9 canopy trees	4 existing trees	_
	2' tall hedgerow	5 canopy trees (proposed)	
Buffer Zone C (E)	10' width	10' width	Requirements met
	23 canopy trees OR	7 existing trees	-
	23 evergreen trees OR	3 canopy trees (proposed)	
	90 shrubs	6 evergreen trees (proposed)	
		28 shrubs (proposed)	
Buffer Zone C (W)	10' width	10' width	Requirements met
	16 canopy trees OR	8 existing trees	_
	16 evergreen trees OR	2 evergreen trees (proposed)	
	64 shrubs	24 existing shrubs	
Parking lot	950 SF landscaped area	1,458 SF landscaped area	Requirements met
	10 canopy trees	2 existing trees	_
		5 canopy trees (proposed)	
		2 evergreen trees	
		1 ornamental tree	
Detention pond	8 trees (deciduous or	10 trees (existing)	Requirements met
	evergreen)	73 shrubs (existing)	_
	80 shrubs	7 shrubs (proposed)	

Genoa Township Planning Commission Aldi Food Market Site Plan Review #2 Page 4

**9.** Exterior Lighting. The submittal includes a lighting plan (Sheet LT1), which includes 9 light poles throughout the parking lot, as well as 24 wall mounted/under canopy fixtures.

The proposed lighting plan, including pole heights, fixture details and light intensities, complies with current Township standards.

**10. Signs.** The existing monument sign is to remain, though replacement of the existing sign cabinet is proposed. This will not alter the sign height or area, both of which comply with current standards.

The building elevation drawings show the two existing wall signs in the northeast corner of the building. As a side note, the 2<sup>nd</sup> wall sign was allowed by the Planning Commission as part of the 2008 approval.

**11. Impact Assessment.** The submittal includes an amended Impact Assessment (January 25, 2018). In summary, the Assessment notes that the project is not anticipated to adversely impact natural features, public services/utilities, surrounding land uses or traffic.

Should you have any questions concerning this matter, please do not hesitate to contact our office. I can be reached by phone at (248) 586-0505, or via e-mail at <u>borden@lslplanning.com</u>.

Respectfully, LSL PLANNING, A SAFEBUILT COMPANY

Brian V. Borden, AICP Planning Manager



February 28, 2018

Ms. Kelly Van Marter Genoa Township 2911 Dorr Road Brighton, MI 48116

## Re: Aldi Expansion Site Plan Review #2

Dear Ms. Van Marter:

Tetra Tech conducted a site plan review of the updated Aldi expansion plans submitted by Desine, Inc. The revised plans are dated February 21, 2018.

The petitioner has updated the stormwater calculations to show that more storage is provided in the existing detention basin than originally submitted and provided a detail to modify the outlet control structure to adequately meet the new storage amounts. The third concern, adequate free board, was addressed by providing updated spot elevations along the top of the detention basin embankment. These spot elevations all meet the 1-foot free board amount required by the Livingston County Drain Commission. With the corrections to the stormwater calculations and heavy-duty pavement section we have no further engineering related comments for the Aldi expansion site plan.

Please call if you have any questions.

Sincerely,

Gary J. Markstrom, P.E. Unit Vice President

copy: Christopher A. Grzenkowicz, P.E., DESINE, Inc.

Marguerite K. Davenport Project Engineer

**BRIGHTON AREA FIRE AUTHORITY** 



615 W. Grand River Ave. Brighton, MI 48116 o: 810-229-6640 f: 810-229-1619

March 1, 2018

Kelly VanMarter Genoa Township 2911 Dorr Road Brighton, MI 48116

RE: Aldi expansion 2260 E. Grand River Genoa Twp., MI

Dear Kelly:

The Brighton Area Fire Authority has reviewed the above mentioned site plan. The plans were received for review on February 27, 2018 and the drawings are dated February 21, 2018. The project is based on an existing 16,657 sq.ft. Mercantile occupancy that will undergo site alteration to accommodate a new 2,254 sq.ft. addition. The plan review is based on the requirements of the International Fire Code (IFC) 2018 edition.

The applicant contacted the fire authority or clarification and revised the most recent submittal. All concerns identified on the previous submittal are shown to be corrected and or addressed.

The fire authority has no further comments for the project at this time.

Additional comments will be given during the building plan review process (specific to the building plans and occupancy). The applicant is reminded that the fire authority must review the fire protection systems submittals (sprinkler & alarm) prior to permit issuance by the Building Department and that the authority will also review the building plans for life safety requirements in conjunction with the Building Department.

If you have any questions about the comments on this plan review please contact me at 810-229-6640.

Cordially,

Rick Boisvert, CFPS Fire Marshal

## **IMPACT ASSESSMENT**

for the

## ALDI FOOD MARKET SITE DEVELOPMENT

Developer: ALDI Inc. 2625 North Stockbridge Road Webberville, Michigan 48892

Prepared by: DESINE Inc. 2183 Pless Drive Brighton, Michigan 48114

April 30, 2008 Amended January 25, 2018

## Introduction

This impact assessment has been prepared pursuant to Section 18.07 of the Genoa Township Zoning Ordinance. This assessment addresses the impact of development of the proposed ALDI Food Market Commercial Development on the natural features, economic condition and social environment of the Township.

The project site consists of approximately 3.13 acres of property zoned **General Commercial District (GCD)** located on the south side of Grand River Avenue, west of Golf Club Road, in Section 6 as shown in Figure 1. The site is currently developed with a 16,657 SF single story commercial building, paved parking area and associated infrastructure. The existing building contains an ALDI Food Market limited assortment grocery store. The existing parking area has a limited access right in / right out driveway on Grand River Avenue and a full access driveway along the Livingston County Complex driveway.

The existing building, paved parking area and infrastructure will remain. A 2,254 SF building addition is proposed along the east side of the existing building within a portion of the existing parking area to provide a total of 18,911 SF that will continue to be utilized as an ALDI Food Market. Minor modifications of the existing parking area are proposed to accommodate the proposed building addition. 24 new parking spaces are proposed along the existing rear drive. The proposed site improvements will increase the parking count from 71 to 95 spaces to accommodate the increase in customer base since 2008. Additional site improvements include site lighting and landscaping improvements as depicted on the Site Plan.

The subject property is located in a significantly developed area, surrounded by restaurant, retail, office and industrial uses. An aerial photograph (circa 2017) of the surrounding area and existing conditions is provided in Figure 2.

This impact assessment has been prepared under the direction of and by:

Christopher A. Grzenkowicz, P.E. DESINE Inc. 2183 Pless Drive Brighton, Michigan 48114 (810) 227-9533

The civil engineering / surveying firm of DESINE Inc. has been in practice since 1989. Mr. Grzenkowicz is a licensed Civil Engineer with experience in private and municipal developments including a number of projects within Genoa Township and Livingston County.

## A. IMPACT ON NATURAL FEATURES

Prior to the construction of the existing ALDI Food Market, the subject property was previously developed with a fast food restaurant and a bank. The majority of the site had been disturbed during the construction of the previous development. The previous buildings, parking areas and infrastructure was removed to allow for construction of the existing ALDI Food Market development. The southern portion of the site contained brush and small trees prior to the existing development. This area was utilized to construct a detention basin for treatment of storm water runoff from the ALDI development. Following construction of the detention basin, this area was stabilized with vegetative growth and now contains a mixture of grasses, shrubs and mature trees. The existing topography of the site generally slopes from Grand River in the northeast to the railroad corridor in the southwest. Slopes are ranging from 1 percent to 5 percent. Elevation difference across the property is approximately 8 feet. Surface water runoff from the subject and adjacent properties flows generally from northeast to southwest toward the existing drainage swale adjacent to the railroad tracks. An existing underground storm sewer system collects runoff from the subject property and directs it to an existing detention basin which discharges at an agricultural rate to the serving the existing drainage swale adjacent to the railroad tracks. The existing site topography is depicted on the Existing Conditions Plan.

The soils on the subject property are primarily Boyer-Oshtemo loamy sands. These soils are generally well-drained, moderately permeable, loamy sands. Soil classifications are prepared by the United States Department of Agriculture, National Resource Conservation Service. The Soils Map, shown in Figure 3, shows the locations of specific soil types as classified.

Previous development of the subject property required land balancing to establish final

grades and provide proper drainage. The proposed building addition and parking improvements are designed to mesh with the existing site improvements as close as possible. Minimal demolition and site grading is necessary to accommodate the proposed building and site improvements.

The limits of disturbance have been depicted on the Soil Erosion and Sedimentation Control Plan. Grading for this project will maintain the general character of the existing site conditions. Development of the proposed site improvements will require minimal exporting of excess topsoil and minimal importing of additional granular fill material.

Vegetation in the area of the proposed parking area improvements will be disturbed. Existing vegetation in these areas consists of open lawn and landscape trees planted during the original development of the existing ALDI Food Market. No landmark trees have been identified on the site. Existing landscape trees consist of spruce, pine, pear, locust, maple and ginko. Existing on-site mature trees consist of elm, cottonwood, box elder, poplar, apple, aspen and oak. The existing detention basin. No existing mature trees will be removed as a part of this project. Existing landscape trees within the proposed building addition area and proposed parking areas will be removed to accommodate the proposed site improvements. Those trees to be removed will be replaced with new plantings. Additional landscaping is proposed to ensure the site is in conformance with the current Zoning Ordinance requirements. Existing trees, including those trees to be removed, are depicted on the Existing Conditions Plan. Proposed replacement trees and other proposed landscaping improvements are depicted on the Landscape Plan.

No wetland areas are present on the project site.

Surface drainage characteristics of the site will not be significantly impacted. The proposed site improvements are designed to direct storm water runoff to the existing storm water management system. The proposed site improvements will result in a minimal reduction of permeable area on the subject property as compared to existing conditions and a minimal increase in the surface water runoff generated from the development site. The proposed increase in lot coverage is approximately 2.8%. The overall proposed lot coverage is 61.8%, well below the 75% allowable. Surface water runoff generated by the proposed site improvements will be directed to the existing detention basin.

The existing detention basin was designed and constructed to accept storm water runoff from the project site in accordance with Genoa Township and Livingston County Drain Commissioner rules and regulations. An analysis of the existing storm water management system has been performed to verify that the existing storm water management system can accommodate the runoff to be generated by the proposed site improvements. The storm sewer analysis calculations are provided on sheet UT2 of the Site Plan.

The minimal impact of surface drainage alterations will not significantly impact local aquifer characteristics or groundwater recharge capacity. Surface water runoff from the developed site will flow into the existing detention basin which discharges along the historical drainage path adjacent to the railroad tracks.

Wildlife habitats exist primarily on the southern portion of the property which includes the existing detention basin, tall grasses, brush, shrubs and mature trees. Wildlife supported in these habitats are generally smaller woodland creatures, field animals and birds. Larger animals, such as deer, may traverse the site. The existing developed portion of the site does not contain any significant wildlife habitat areas. The proposed site improvements do not result in a significant impact to the existing wildlife habitats. No site improvements or disturbance is proposed in the southern portion of the site.

The project site does not currently support significant wildlife habitats and development of the proposed site improvements will not have a significant impact on the overall habitat quality. No significant adverse impact to existing natural features are anticipated due to the proposed site improvements for this property.

## B. IMPACT ON STORM WATER MANAGEMENT

The subject property contains an existing storm water management system consisting of an underground storm sewer network, a sedimentation basin and a detention basin. Minimal earthwork and site grading is necessary to accommodate the proposed site improvements and to direct surface drainage from the proposed site improvements to the existing storm water management system. Earthwork will be limited to the areas of proposed building and parking improvements.

Catch basins will collect surface water from parking and drive areas, building areas, and open space areas. Grading for the proposed site improvements will mesh with existing grades immediately surrounding the improvement areas. No adverse impact to adjoining properties is anticipated due to grading or construction of the proposed site improvements.

Soil erosion and sedimentation control is governed by the Soil Erosion Control Act No. 347 of the Public Acts of 1972, as amended as administered by the Livingston County Drain Commissioner. Silt fencing, sediment inlet filters and other soil erosion control measures will be required around the areas of the proposed site improvements as depicted on the Soil Erosion and Sedimentation Control Plan. The Contractor shall be responsible for initiating

and maintaining adequate dust control measures during construction. The Contractor shall also be responsible for complying with all soil erosion and sedimentation control regulations during and after construction until the project site is fully stabilized and vegetative cover is established within the disturbed areas outside of hard surfacing.

Impact to adjoining properties due to the construction of the site improvements will be minimized by implementing soil erosion and sedimentation control measures. No adverse impact to adjacent properties due to surface water runoff will be created as a result of the proposed site improvements.

## C. IMPACT ON SURROUNDING LAND USES

Property to the North of the site is zoned Neighborhood Service (NS) and is occupied by CVS Pharmacy. Property to the East is zoned General Commercial District (GCD) and serves as the entryway to the Livingston County East Complex, east of that is PNC Bank. The Livingston County East Complex is located south of the subject parcel and is zoned GCD. West of the property is Mourad's Grill, also zoned GCD.

The Genoa Township Future Land Use Plan designates the subject property for General Commercial uses. The surrounding property is also designated for General Commercial use.

The existing use is consistent with the existing development in the surrounding area and is consistent with the long term planning within the Township. The proposed landscaping and architecture is consistent with the existing site development to allow this site to continue to be in harmony with the surrounding area. No adverse impact to the surrounding area is anticipated due to the proposed site improvements. All areas disturbed by construction will require restoration.

The continued use of the property as a general commercial use will not create any significant emissions of smoke, airborne solids, odors, gases, vibrations, noise or glare discernable and substantially annoying or injurious to persons and/or property beyond the lot lines. Radioactive emissions and electromagnetic radiation shall not be emitted in excess of quantities established as safe in accordance with the ordinance when measured at the property lines. No flammable liquids, gases or explosives shall be stored or used on the property. No underground or aboveground storage tanks are proposed on the property.

## D. IMPACT ON PUBLIC FACILITIES AND SERVICES

Police protection is and will continue to be provided by the Livingston County Sheriff and the Michigan State Police. Additional services required to accommodate the proposed site improvements are anticipated to be minimal.

Fire protection is provided by the Brighton Area Fire Department. The subject property is within the water district and fire hydrants are readily accessible for utilization in the event of a fire. Adequate fire protection systems are provided within the existing building and are proposed within the building addition. Additional fire protection services required to accommodate the proposed site improvements are anticipated to be minor.

Construction of the proposed site improvements will not create any direct adverse impact on the schools.

## E. IMPACT ON PUBLIC UTILITIES

The subject property is located within the municipal sewer and water districts. Existing water main and sanitary sewer are located in easements to the south and east of the existing building. The existing building is connected to the existing sanitary sewer and water main.

Water service to the building is provided from the existing water main located within an easement that traverses the property south of the existing building. No significant increase in water use is anticipated as a result of the proposed site improvements. Capacity is available within the existing water system to provide adequate service to this development.

Sanitary sewer service for the development is provided by the existing sanitary sewer main which traverses the site south of the existing building. No significant increase in sanitary sewage flow is anticipated as a result of the proposed site improvements. Capacity is available within the existing sanitary sewer system to provide adequate service to this development.

The site is serviced by electric, gas, phone and cable TV systems located along Grand River Avenue. No significant increase in demand or use of the existing public utility systems is anticipated as a result of the proposed site improvements.

## F. STORAGE AND HANDLING OF ANY HAZARDOUS MATERIALS

The proposed general commercial space will not utilize or store any potentially hazardous or polluting materials other than standard household type cleaning products. All solid wastes should be properly disposed of through a licensed waste disposal firm on a regular basis.

## G. IMPACT ON TRAFFIC AND PEDESTRIANS

The project site is located along the Grand River Avenue corridor. Grand River Avenue, in the area of the proposed development, is major arterial public street consisting of four directional traffic lanes and a center left turn lane. Access to the site from Grand River Avenue is provided via a limited access, right in – right out driveway near the west side of the site and thru the driveway serving the Livingston County East Complex.

Traffic Engineering Associates, Inc. located in Lansing Michigan has prepared a Traffic Assessment for the existing development (Attachment 1). The Traffic Assessment has been provided to and reviewed by both the Livingston County Road Commission and Michigan Department of Transportation.

The Traffic Assessment projected the ALDI development to generate 49 vehicle trips in the AM Peak hour, 208 vehicle trips in the PM peak hour and 1534 vehicle trips daily. A significant amount of the trips are classified as Pass-By Trips. Pass-by trips are already present in the existing passing traffic.

The actual traffic generated by the development of the ALDI Food Market is now existing in the traffic stream. The existing ingress and egress points and signalized intersection of Grand River / Golf Club / Livingston County Complex Driveway operate at an acceptable level of service. The proposed site improvements are not anticipated to result in a significant increase in traffic. No significant impact on the major thoroughfares of Livingston County is anticipated as a result of the proposed site improvements.

The existing development provides service to pedestrian traffic through a pedestrian sidewalk connection from the existing sidewalk along Grand River to the building entrance. The pedestrian access will be maintained as a part of the proposed site improvements. No significant increase in pedestrian traffic or adverse impact upon pedestrian traffic is anticipated as a result of the proposed site improvements.

January 25, 2018

## H. SPECIAL PROVISIONS

A variance has been obtained for the reduction of the parking setback from the Right-of – Way of Grand River Avenue. A reduction of 16 feet was granted to allow the 4-foot parking setback from the R.O.W for the portion of the parking area adjacent to the 75-foot wide  $\frac{1}{2}$  R.O.W.

An easement agreement to utilize the driveway to the Livingston County East Complex has been entered into. No additional special provisions or requirements are currently proposed for this facility.

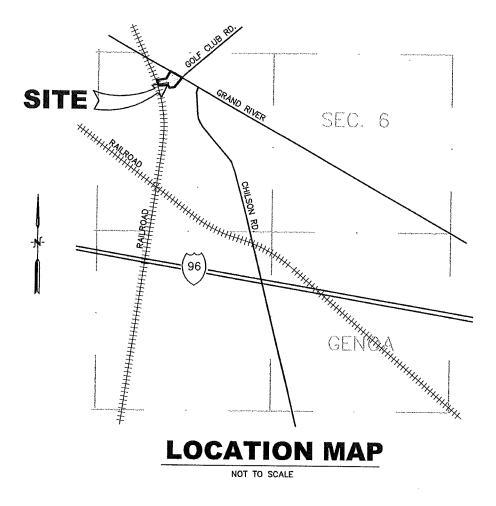
## I. SITE LIGHTING

All site lighting shall meet the requirements of the Genoa Township ordinances. Exterior building mounted site lighting shall be shielded and down directed. Pole mounted site lighting is proposed for this project. All pole-mounted lighting shall be shielded and down directed on the site. General site lighting, excluding safety and emergency lighting, shall be used between the times from dusk to 12:00 a.m. and from 5:00 a.m. to dawn.

## J. HOURS OF OPERATION

Hours of operation will generally be between 9:00 am to 8:00 pm seven (7) days a week. Corporate deliveries are the bulk of the product line and are scheduled when the store is closed. Corporate deliveries typically do not exceed one (1) delivery per day. Supplier delivery trucks deliver milk, bread, pop and similar products periodically during regular business hours. Supplier deliveries typically occur once or twice a week depending on the product delivered.

FIGURE #1



Site Address 2260 E. Grand River Avenue Howell, MI 48843 FIGURE #2



# AERIAL PHOTOGRAPH

NO SCALE

FIGURE #3



## **SITE SOILS**

## NO SCALE

Source: Web Soil Survey United States Department of Agriculture, Natural Resources Conservation Service

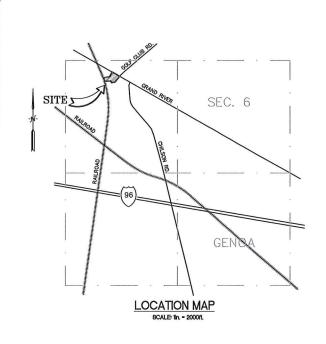
## Soil Legend:

- BtE Boyer-Oshtemo loamy sands, 18 to 25 percent slopes
- BtA Boyer-Oshtemo loamy sands, 0 to 2 percent slopes
- MoE Miami loam, 18 to 25 percent slopes

January 25, 2018

# ATTACHMENT #1

12



## LEGAL DESCRIPTION:

LEGAL DESCRIPTION: Commencing at the Northwest Corner of Section 6, Town 2 North, Ronge 5 East, Genoa Township, Livingston County, Michigan; thence SB707'30"E 841.85 feet (SB702'00"E 844.70 feet record) along the North line of said Section 6; thence S600°6'45"E 609.89 feet (606.69 feet record) along the centerline of Grand River Avenue to the PLACE OF BEGINNING; thence continuing S600°6'45"E 508.81 feet (sole.69 feet record) along the centerline of Grand River Avenue to the PLACE OF BEGINNING; thence continuing S600°6'45"E 609.89 feet (sole.69 feet record) along the Avenue (variable width Right-of-way); thence S84'35'37"W 100.61 feet; thence N03'34'18"W 14.51 feet; thence N60'06'45"W 85.63 feet; thence S69'20'23"W 346.13 feet (slos recorded as S69'18'10"W 346.39 feet); thence S69'20'23"W 345.92 feet, also recorded as S69'18'10"W 345.39 feet); thence N35'10"W 120.00 feet along the Easterly line of the Tuscola and Saginaw Bay Railway (66 foot wide Right-of-Way), formerly known as Ann Arbor Railroad; thence (66 foot wide Right-G-Way), formerly known as Ann Arbor Railroad; thence N68/26'35"E 254.80 feet; thence N29'37'5"E 300.00 feet to the Place of Beginning. Being a part of the Northwest 1/4 of Section 6, Town 2 North, Range 5 East, Genca Township, Livingston County, Michigan. Containing 3.14 acres of land, more or less. Subject to the rights of the public over that portion thereof token for Grand River Avenue, also subject to and together with all easements and restrictions affecting title to the above described oranges

## BENCHMARKS:

DATUM BASED ON PREVIOUS BENCHMARK AS DEPICTED ON AS-BUILT PLANS PREPARED BY DESINE INC., JOB No. 81389, REVISED DATE DECEMBER 3, 2008. DATUM AS DEPICTED = NGVD

## PRIMARY (OFF SITE)

AT HOWELL, LIVINGSTON COUNTY, ON THE ANN ARBOR RAILROAD, 35 YARDS EAST OF THE NORTHEAST CORNER OF THE STATION; 60 YARDS WEST OF A HIGHWAY BRIDGE OVER THE TRACK; 7 YARDS NORTH OF THE WEST RAIL: A STANDARD TABLET STAMPED "G 105 1934" AND SET IN THE TOP OF A CONCRETE POST. ELEVATION = 921.931 (NGVD) REF: SEE ABOVE

## BENCHMARK #201

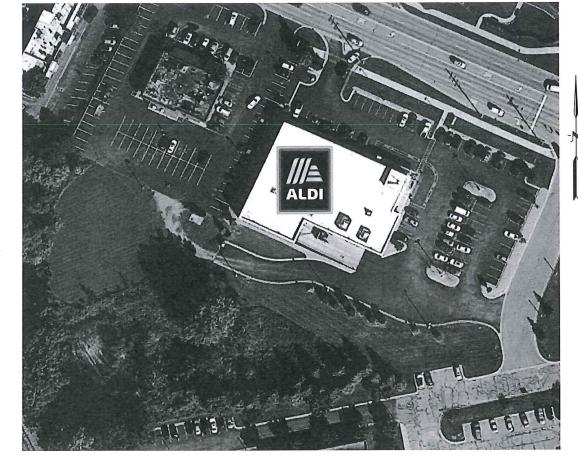
PREMOUSLY DESCRIBED AS: RAILROAD SPIKE IN THE NORTH SIDE OF AN UTILITY POLE. LOCATED ON THE SOUTH SIDE OF GRAND RIVER AND NEAR ABANDONED MCDONALDS RESTAURANT CURRENTLY: LOCATED NEAR THE EASTERLY SIDE OF THE NORTHERLY ENTRANCE FROM GRAND RIVER TO "ALDI" STORE. ELEVATION = 925.46 (NGVD) REF: SEE ABOVE FIELD BOOK D405, PG. 43 POINT #201

## BENCHMARK #20

ARROW OF HYDRANT, LOCATED 57± FEET NORTHERLY OF THE NORTHEASTERLY BUILDING CORNER "ALDI " ELEVATION = 930.09 (NGVD) REF: POINT #200

# SITE PLAN ALDI FOOD MARKET #51 REMODEL AND EXPANSION

2260 EAST GRAND RIVER AVENUE HOWELL, MI 48843 A PART OF THE NORTHWEST 1/4 OF SECTION 6, T2N, R5E GENOA TOWNSHIP, LIVINGSTON COUNTY, MICHIGAN



**AERIAL PHOTOGRAPH** 

Google maps

## ENGINEER/SURVEYOR

DESINE INC. 2183 PLESS DRIVE BRIGHTON, MICHIGAN 48114 PHONE: (810) 227-9533

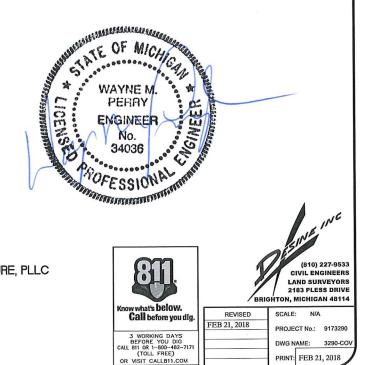
## **DEVELOPER / APPLICANT**

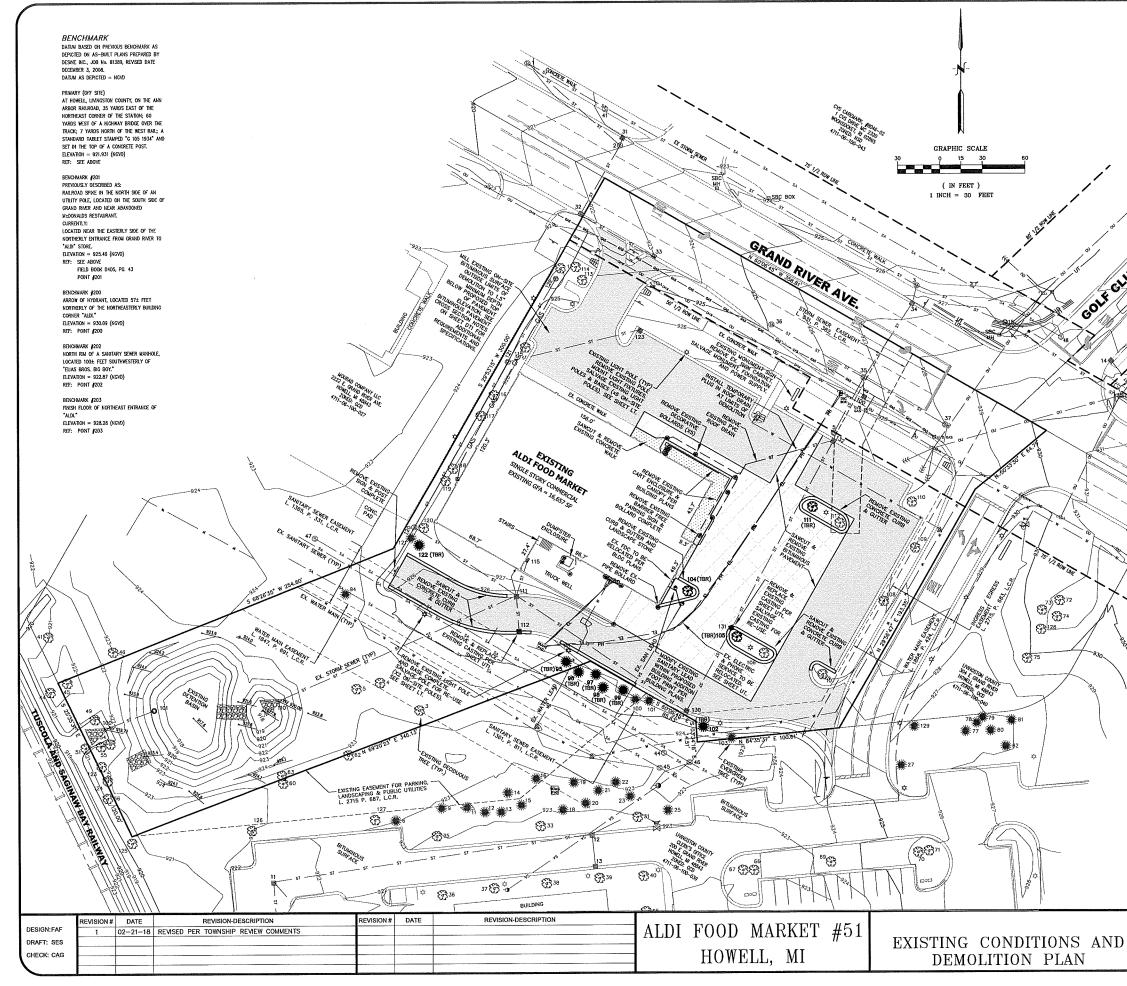
ALDI Inc. 2625 N. STOCKBRIDGE ROAD WEBBERVILLE, MICHIGAN 48892 PHONE: (517) 521-3907

APD ENGINEERING AND ARCHITECTURE, PLLC 615 FISHERS RUN VICTOR, NEW YORK 14564 PHONE: (585) 742-2222

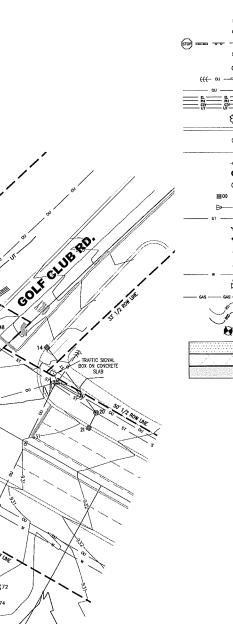
## SHEET INDEX

- EX1 **EXISTING CONDITIONS & DEMOLITION PLAN**
- EX2 **EXISTING SITE DATA & DEMOLITION NOTES**
- SITE PLAN
- UTILITY PLAN UT1
- UT2 STORM WATER MANAGEMENT CALCULATIONS
- GRADING PLAN GR
- SOIL EROSION & SEDIMENTATION CONTROL PLAN SE1
- SOIL EROSION & SEDIMENTATION CONTROL NOTES & DETAILS SE2 LANDSCAPE PLAN LA1
- LANDSCAPE NOTES & DETAILS LA2
- SITE LIGHTING PLAN LT1
- LT2 SITE LIGHTING NOTES AND DETAILS
- SITE PAVEMENT NOTES & DETAILS DT1
- DT2 SIGNAGE & PAVEMENT MARKING NOTES & DETAILS
- STORM SEWER NOTES & DETAILS DT3
- WATER MAIN & SANITARY SEWER NOTES & DETAILS DT4
- M.H.O.G. STANDARD DETAILS DT5
- DT6 M.H.O.G. STANDARD DETAILS
- **BUILDING FLOOR PLAN / OPERATIONS PLAN** A-131
- EXTERIOR ELEVATIONS A-201









LE	GEND
0	MISC. STRUCTURE (AS LABELED)
	BOLLARD
	SIGN
¢	Light Base
0	UTILITY WANHOLE (AS LABELED)
	UTILITY POLE W/GUY WRE
	OVERHEAD UTILITY LINES (ELECTRIC/PHONE/CABLE
	U/G LINES (ELECTRIC/PHONE/CABLE)
÷.	DECIDUOUS TREE W/IDENTIFIER
	EX CONCRETE CURB (UNLESS OTHERWISE STATED)
	sanitary sewer manhole w/identifier sanitary sewer pipe
	CLEAN OUT
-9 O	CONTROL STRUCTURE
	STORM WATER WANHOLE W/DENTIFIER
	CATCH BASIN W/IDENTIFIER
⊳	FLARED END SECTION
	EX STORM WATER DRAMAGE PIPE
	HYDRANT
v	WATER SHUT OFF
۲	WATER VALVE
2	WATER VALVE BOX
	WATER MAIN
X <sup>3</sup>	CAS SHUT OFF
	U/G GAS
AN I	1' CONTOUR
*	5' CONTOUR
<b>9</b> 58-1	SOIL BORING W/ INDENTIFIER
~	
	CONCRETE WALK TO BE REMOVED
	BITUMINOUS PAVEMENT TO BE REMOVED
	BITUMINOUS PAVEMENT TO BE WILLED

RI ( . INC now what's below. Call before you dig (810) 227-9533 3 WORKING DAYS BEFORE YOU DIG CALL 811 OR 1-800-482-717 (TOLL FREE) OR VISIT CALL811.COM CIVIL ENGINEERS LAND SURVEYORS 2183 PLESS DRIVE HTON, MICHIGAN 48114 CLIENT: SCALE: 1\*=30' ROJECT No.: 9173290 ALDI Inc. 2625 N. STOCKBRIDGE RD. WG NAME: 3290 EX

SUED: FEB 21, 2018

WEBBERVILLE, MICHIGAN 48892

(517) 521-3907

EX1

EXISTING TREE SCHEDULE:
1 8" MAPLE 3 18" ELM
4 24" COTTONWOOD 5 18" COTTONWOOD
6 9" PINE 0 8" PINE
11 9" PINE 12 8" PINE
13 9" PINE 14 8" PINE
16 07 000
15 9" PINE 16 8" PINE 18 8" PINE 19 9" PINE 20 9" PINE 21 10" PINE
20 9" PINE 21 10" PINE
22 10" PINE
23 6" PINE 25 6" PINE 27 6" PINE
31 11" CRABAPPLE 33 12" CRABAPPLE
35 9" CRABAPPLE 36 8" CRABAPPLE
37 12" CRABAPPLE
38 9" CRABAPPLE 39 9" CRABAPPLE
40 7" CRABAPPLE 41 10" BOX ELDER
44 7" TWIN OAK 45 11" BOX ELDER
46 12" TWIN BOX ELDER 47 12" TWIN BOX ELDER
48 24" TWIN LOCUST 49 18" POPLAR
48 24° TWIN LOCUST 49 18° POPLAR 50 18° LOCUST - DEAD 51 12° POPLAR 52 12° POPLAR 54 20° POPLAR 55 22° POPLAR 56 13° TWIN BOX ELDER 56 13° TWIN BOX ELDER
52 12" POPLAR 54 20" POPLAR
55 22" POPLAR 56 13" TWIN BOX ELDER
60 12" POPLAR - DEAD 62 10" APPLE
64 7" CRABAPPLE
65 7" CRABAPPLE 66 3" CHERRY
67 3" CHERRY 69 3" CHERRY
70 4" CHERRY 71 4" CHERRY
72 9" CRABAPPLE 73 5" CRABAPPLE
74 9" CRABAPPLE 75 7" CRABAPPLE
77 7" PINE 78 8" PINE
77 7 PINE 78 8 PINE 80 8 PINE 81 8 PINE 82 6 PINE 83 6 TWIN PINE 84 60 PINE
81 8" PINE 82 6" PINE
85 9" PINE 86 9" PINE
87 9" PINE 88 9" PINE
89 6" CRABAPPLE 90 6" CRABAPPLE
91 6" CRABAPPLE 92 6" CRABAPPLE
93 6" CRABAPPLE 94 6" PINE
95 6" PINE (TBR) 96 6" PINE (TBR)
97 6" PINE (TBR) 98 6" PINE (TBR)
99 6" PINE (TBR) 100 6" PINE
101 6" PINE 102 6" SPRUCE (TBR)
102 6" SPRUCE 103 6" SPRUCE 104 5" PEAR (TBR)
IAS AT LOCUST (TOP)
105 4 LOCUST 106 4* LOCUST 107 4* PEAR 108 5* GINKO 109 3* GINKO
109 3" GINKO 110 6" PEAR
111 4" LOCUST (TBR) 112 3" LOCUST
113 6" PEAR
114 6" PEAR 115 5" PEAR 116 6" PEAR
116 6" PEAR 117 6" PEAR 118 6" PEAR
119 6" PEAR
120 6" PEAR 121 6" SPRUCE
122 6" SPRUCE (TBR) 123 11" TRIPLE OAK
124 9" TWIN OAK 125 9" TWIN OAK
126 12" TWIN OAK 127 12" TWIN OAK 128 7" CRABAPPLE
128 7" CRABAPPLE 129 6" PINE

EX	ISTING STRUCTURE INVENTO	<u>K1</u>
storm manhole <b>(</b> 1 811 920,89	CATCH BASN #14 RM 928.13	C8-100 RM 926.60
storn manhole #2 811 921.98	YARDBASIN #15 RIM 926.12	CS-101 RM 922.67 15" SW RCP 919.92
storm wanhole #3 8W 926.74	YARDBASIN #20 Rim 930.43	CB-111 RM 925.08
storni manihole #5 201 926.44 Northeast 15" RCP 914.69	CATCH BASN #21 RM 931.09	12" N PVC 920.58 12" NW HDPE-S 920.7 12" NE RCP 920.74
SOUTH 8" RCP 914.64	CATCH BASN #22 RM 934.08	15" SE RCP 920.73 12" S RCP 921.00
SNU 921.77 SOUTHWEST 15" ROP	YARDHASH #23 RM 934.28	18" SW RCP 920.45 C8-112
CATCH BASIN #32 BM 921.69	CATCH BASN #24 RM 934.60	RM 925.07 12" N RCP 921.08
NORTHEAST 15" RCP 916.29 SOUTHEAST 15" RCP 916.49	CATCH BASN #25 RM 933.14	C8-115 RM 923.97 12° SW RCP 921.87
Catch Basin #33 RM 922.96 Northwest 15" RCP 920.91	CATCH BASN #26 RM 921.38	MH-120 R34 924.97
CATCH BASIN 434 RM 926.63	SOUTHEAST 15" HDPE 918.24 NORTHWEST 24" RCP 918.24	12" NE HDPE-S 921.1 12" SE HDPE-S 921.1
Southwest 15" RCP WANHOLE #35	SANITARY SEWER WANHOLE #41 RM 922.15	MH-121 RM 925.01
RM 926,70 Northwest 12° RCP 920,6 Northeast 15° RCP 914,10 Southeast 12° RCP 922,10 Southwest 15° RCP 914,25	SANITARY SEVER WANHOLE #42 0 Rim 925.13	12" NE RCP 921.51 12" E RCP 921.49 12" SW HDPE-S 921.4
Southeast 12" RCP 922.10 Southwest 15" RCP 914.2	5 SANITARY SEVER WANHOLE 143 5 RW 924.83	YB-122 R3N 923.89
NANHOLE <b>(</b> 36 RM 925.01 Southeast 12° RCP 921.86	SANTARY SEWER WANHOLE #44 RM 923.14 NORTHWEST SOR TOP OF PIPE 918.04	12" SW RCP 921.69
WANHOLE #37 RNI 927.96	SOUTHEAST SDR TOP OF PIPE 917.79	CB-123 R3M 926.02 12" W RCP 921.93
NORTHWEST 12" RCP 922.6		CB-130 RM 925.63
CATCH BASH #11 RM 921.31 E-NORTHEAST 24" ROP 916	SANITARY SEVER WANHOLE #46 RM 923.03 5.11 SOUTHEAST 8° PVC 913.67	12" NE RCP 921.39 15" NW RCP 921.23
South 24" RCP 916.11 Southeast 8" PVC 917.06		C8-131 R3M 926.60
CATCH BASIN #12 RIM 921.42 #-Southwest 24" RCP 91	SANTARY SEVER MANHOLE 147 RM 923.57 7.37 SOUTHEAST 8 PVC 912.02 7.22 NORTHWEST 8 CLAY 911.92	12" NE RCP 921.67 12" SW RCP 921.58
E-NORTHEAST 24" RCP 91 S-SOUTHWEST 12" RCP 917	7.22 NORTHWEST 8" CLAY 911.92 7.37 SANITARY SEWER WANHOLE #48	CB132 RM 926.50 8" W PVC 922.51
CATCH BASIN ∦13 RM 921.09 N—NORTHEAST 12" RCP 917	RM 928.05	12" SW RCP 922.16

z. conuacio
local under
demolition
to this firm
No guarante
depths of al
in the field.
3. Contracte
nhone cabl

4. Contractor shall contact the appropriate Agencies to coordinate removal and/or relocation of any underground and/or overhead public utility lines as necessary prior to performing demolition work.

6. All bituminous and concrete pavement to be removed shall be saw cut at the limits of removal to provide for a clean straight edge for future abutment. The existing pavement that is to be removed, shall remain in place as long as possible to be used as a soil erosion control measure. Contractor shall coordinate the pavement removal work with the appropriate stages of construction. Coordination work shall include removing the existing pavement in sections / stages as necessary to minimize impacts to the adjacent parcel.

7. All existing irrigation lines to be removed shall be terminated at the limits of demolition or as necessary to allow for construction of the proposed site improvements. Ends of pipe shall be capped and the location of marked for future connection.

8. All existing water main and sanitary sewer to be removed shall be terminated at the limits of 8. All existing water main and sanitary server to be removed snail ob terminated at the limits of demolition or as indicated on the project plans. Temporary plugs shall be installed in the ends of pipe in accordance with the appropriate Agency and the locations of marked for future connection. Permanent plugs shall be installed in the ends of pipe in accordance with the appropriate Agency. The Contractor shall record the location of all permanent plugs and provide the location information to the appropriate Agency.

9. All existing storm sewer to be removed shall be terminated at the limits of demolition or as indicated on the project plans. Temporary plugs shall be installed in the ends of pipe in accordance with the appropriate Agency and the locations of marked for future connection. Permanent bulkheads shall be installed in the ends of pipe and/or openings in terminating structures in accordance with the appropriate Agency. The Contractor shall record the location of all permanent bulkheads and provide the location to the appropriate Agency.

10. All existing light sources to be removed shall have their power cables removed up to the power source or properly terminated for future connection at the limits of demolition or as necessary to allow for construction of the proposed site improvements. Removal and termination of power cables shall be performed in accordance with local electric codes.

11. All existing utility meters to be removed shall be properly removed to allow for reuse. Any existing utility meters that are not to be reused as a part of this project shall be returned to the appropriate Agency.

12. All trenches and/or excavations resulting from the demolition of underground utilities, building foundations, etc., that are located within the 1 on 1 influence zone of proposed structures, paved areas and/or other areas subject to vehicular traffic shall be backfilled with MDOT Class III granular material (or better) to the proposed subgrade elevation. Backfill shall be shall be placed using the controlled density method (12" maximum lifts, compacted to 95% maximum unit weight, Modified Proctor).

DESIGN:FAF	REVISION # DATE	REVISION-DESCRIPTION REVISED PER TOWNSHIP REVIEW COMMENTS	REVISION # E	DATE	REVISION-DESCRIPTION	ALDI FOOD MARKET	451	EXISTING
DRAFT: SES						4	#01	SITE DATA &
CHECK: CAG						HOWELL, MI		DEMOLITION NOTES

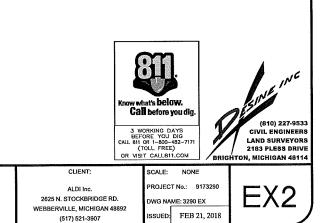
## DEMOLITION NOTES:

The demolition specifications of the Local Municipality are a part of this work. Refer to the General Notes on the project plans for additional requirements.

2. Contractor shall contact the 811 Underground Public Utility Locating System or other appropriate local underground utility locating Agency, a minimum of three (3) working days prior to performing demolition work. Existing utility information on the project plans may be from information disclosed to this firm by the Utility Companies, Local, County or State Agencies, and/or various other sources. No guarantee is given as to the completeness or accuracy thereof. Prior to construction, locations and depths of all existing utilities (in possible conflict with the proposed improvements) shall be verified in the fade.

Contractor shall contact the appropriate Agencies to coordinate disconnect of the electric, gas, phone, cable and other public utilities as necessary prior to performing demolition work.

Contractor shall recycle and/or dispose of all demolition material and debris in accordance with the appropriate Local, County, State and Federal regulations.





3. The Contractor shall coordinate all site work with their Subcontractor(s), the Sign Contractor(s) and the sign work to ensure that all sign work is performed in a timely manner and in a proper sequence in accordance with shandard acceptable construction practices and to ensure that all sign work is completed prior to the scheduled store acceptance acceptance. YARDS WEST OF A HIGHWAY BROCE OVER THE TRACK; 7 YARDS NORTH OF THE WEST RALL A STANDARD TABLET STAUPED "G 105 1934" AND SET IN THE TOP OF A CONCRETE POST. re-opening date.

4. The Contractor shall maintain the project site during the construction period. Maintenance shall include, but is not limited to, routine sweeping of the parking area and removal of trash and debris on an as needed basis and/or as directed by ALDI Inc.

BENCHWARK (201) PREVIOUSLY DESCRIBED AS: RALROAD SPIKE IN THE NORTH SIDE OF AN 5. The existing concrete pavement within the truck well shall be power washed. All existing joints and cracks shall be re-caulked with joint sealant in accordance with the Project Manual. UTILITY POLE, LOCATED ON THE SOUTH SDE OF

All barrier free ramps shall be constructed with detectable warnings in accordance with MDOT Standard Plan R-28, latest edition.

7. A variance to the right of way parking setback requirements was granted by the Genoa Township Zoning Board of Appeals in 2008 to permit a 4-foot parking setback to the Grand River Road right of way.

All parking spaces shall be double striped with 4" wide stripes spaced 18" apart on center in accordance with Genoa Township requirements.

9. The existing building is fire supressed. The fire supression system shall be modified and expanded to provide fire protection for the building addition in accordance with NFPA 13 and the Brighton Area Fire Authority standards and specifications.

10. The Contractor shall maintain fire truck and emergency vehicle access to the site during the construction period.

WATER WANN EACHENT

DETENTING

REVISION-DESCRIPTION

02-21-18 REVISED PER TOWNSHIP REVIEW COMMENTS

STORN SEWER (TP.)

SETBACK (TY

EX WATER SAULT

 $\square$ 

SANITARY SEWER F

÷#

3

3

BUILDING

REVISION-DESCRIPTION

(TAP) HOHT

8

10 B

DATE

Ð 3

-- EXISTING EASEMENT FOR PARKING. LANDSCAPING & PUBLIC UTILITIES L. 2715 P. 687, L.C.R.

÷

EVISION #

BI TUMINOUS SURFACE

BENCHWARK #202 NORTH RIM OF A SANITARY SEWER WANHOLE, LOCATED 100± FEET SOUTHWESTERLY OF

REF: POINT 200

₿

DESIGN FAF

DRAFT: SES

CHECK: CAG

SAGINAN BALI

FRAILUNAL

VISION # DATE

ELEVATION = 921.931 (NGVD)

GRAND RIVER AND NEAR ABANDONED

NORTHERLY ENTRANCE FROM GRAND RIVER TO

BENCHWARK #200 ARROW OF HYDRANT, LOCATED 57± FEET NORTHERAY OF THE NORTHEASTERAY BUILDING CORRER "ALDI." ELEVATION = 930.09 (NGYD)

COONALDS RESTAURANT. CURRENTLY: LOCATED NEAR THE EASTERLY SIDE OF THE

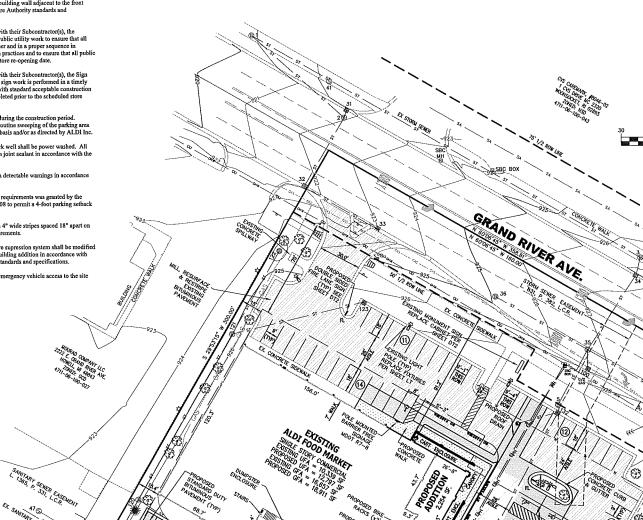
"ALDI" STORE. ELEVATION = 925.46 (NGVD)

ELEVATION = 925.46 (NOV) REF: SEE ABOVE HELD BOOK D405, PG. 43 PONT #201

REF: SEE ABOVE

"ELIAS BROS. BIG BOY." ELEVATION = 922.87 (NGVD) REF: POINT #202

BENCHWARK \$203 FINISH FLOOR OF NORTHEAST ENTRANCE OF "ALDL" ELEVATION = 928.28 (NGVD) REF: POINT #203





GRAPHIC SCALE

15

( IN FEET )

a

Ű.

1 a

BITUMINOUS SURFACE

6363

ALDI FOOD MARKET #51

HOWELL, MI

3

3

¢ 🔁

14411 0.011 0.020

BB

3

ES)

Â

5 37

ROW LAW

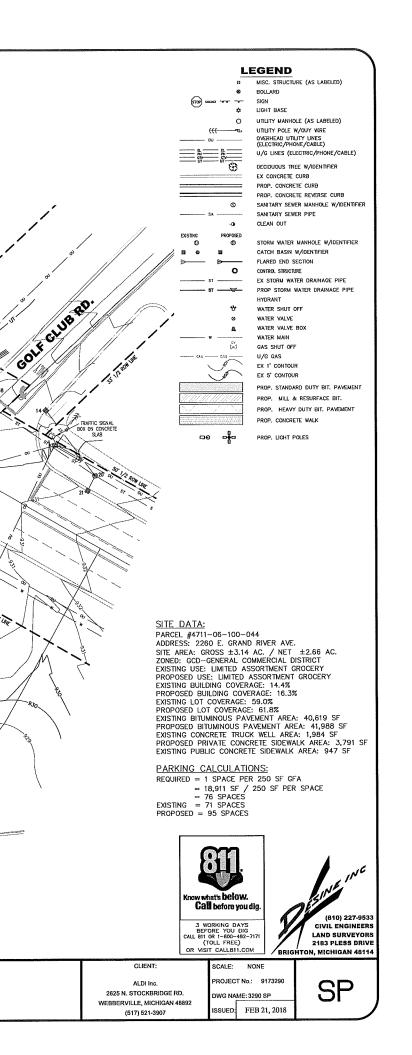
E3

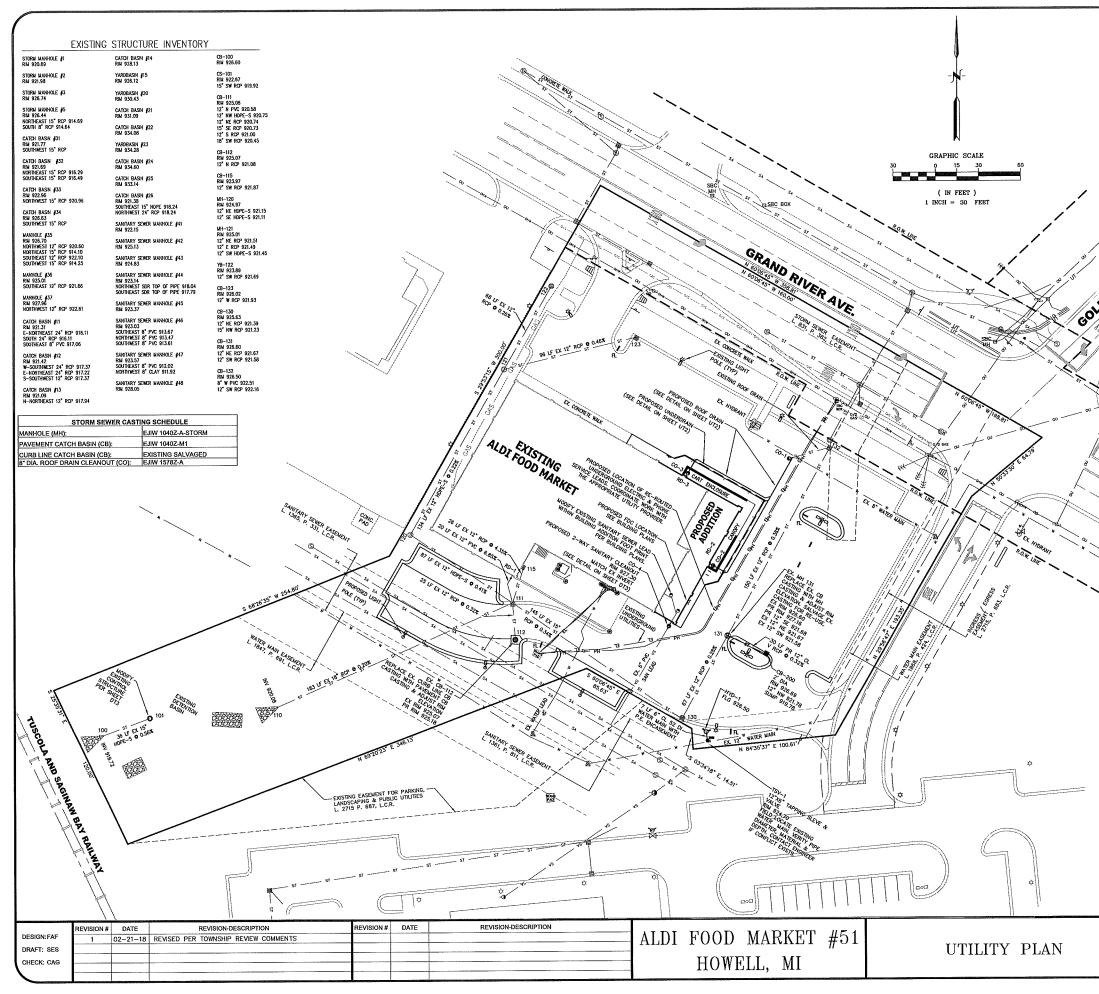
. <del>@</del>/#

83L

舒

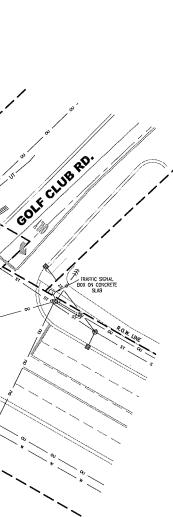
1 INCH = 30 FEET





## LEGEND

뙵



MISC. STRUCTURE (AS LABELED) BOLLARD sign Light base UTULTY METERS & BOXES (ELECTRIC METER, GAS METER, WATER METER PHONE BOX, CATV BOX, MAL, BOX) OVERHEAD UTILITY LINES (ELECTRIC/PHONE/ U/G UTILITY LINES (ELECTRIC/PHONE/CABLE) DECENSIONS TREE WADENTERED Conferous tree w/dentifier CONCRETE CURB (UNLESS OTHERWISE STATED) SWITLAY SEWER WANHOLE WIDENTIFIER SANTARY SEWER PARAHOLE WIDENTIFIER SLANT OUT CONTROL STRUCTURE \$ -0 STORM WATER MANHOLE W/IDENTIFIER 0 CATCH BASIN W/DENTIFIER CATCH BASN W/DENTI STORM DRAMAGE LINE HYDRANT WATER SHUT OFF WATER VALVE WATER VALVE BOX Ъ. 2 WATER MAIN WATER MAIN EASEMENT GAS SHUT OFF N N U/G GAS U/G GAS PROP. CONCRETE CURB PROP. REVERSE CONCRETE CURB PROP. REVERSE CONCRETE CURB PROP. CLEAN QUT PROP. CLEAN QUT PROP. CLEAN QUT PROP. CLEAN QUT PROP. LIGHT POLE -0 BENCHMARK DATUM BASED ON PREMOUS BENCHMARK AS DEPICTED ON AS-BUILT PLANS PREPARED BY DESINE INC., JOB No. 81389, REVISED DATE DECEMBER 3, 2008. DATUM AS DEPICTED = NGVD PRIMARY (OFF SITE) AT HOWELL, LIVINGSTON COUNTY, ON THE ANN ARBOR RAILROAD, 35 YARDS EAST OF THE ARBOR RALROAD, 33 YARDS EAST OF THE NORTHEAST CORNER OF THE STATUR, 60 YARDS WEST OF A HIGHWAY BRIDGE OVER THE TRACK; 7 YARDS NORTH OF THE MEST RAL; A STANDARD THALET STALPED '0 (105 1334' AND SET IN THE TOP OF A CONCRETE POST. ELEVATION = 921.931 (NGVD) REF: SEE ABOVE BENCHWARK (201 PREVIOUSLY DESORBED AS: RAILROAD SPIKE IN THE NORTH SIDE OF AN UTILITY POLE, LOCATED ON THE SOUTH SIDE O GRAND RIVER AND NEAR ABAND MCDONALDS RESTAURANT. MEDROUDS RESIDUARS. LOCATED HEAR THE EXSTERTY SDE OF THE NORTHERLY BUTKINGE FROM GRAND RIVER TO "ALD" STORE. ELEVATION = 925.46 (NNO) REF: SEE ADDVE PELD BOOK PAGE, PC, 43 POULT "ADDVE POINT #201

BENCHWARK (200 ARROW OF HYDRANT, LOCATED 57± FEET NORTHERLY OF THE NORTHEASTERLY BUILDING CORNER "ALDL" ELEVATION = 930.09 (NGVD) REF: POINT #200

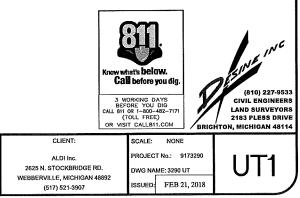
UTILITY NOTES:

The existing on-site storm sewer system shall be cleaned, including jetting of storm pipes and vacuuming of storm structures and sumps. Roof drains and/or other storm pipes less than 12<sup>n</sup> diamete that cannot be properly jetted shall be snaked.

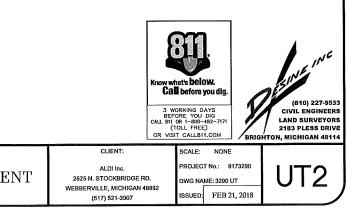
2. All on-site storm sewer structures shall be inspected for cracks, leaks and damage and shall be repaired accordingly in accordance with standard acceptable construction practices and as acceptal ALD Inc. including, but not limited to: tuck pointing of pine poenings, seams, cracks and casting replacement of broken and/or damaged brick, block and grade rings and other repairs as necessary

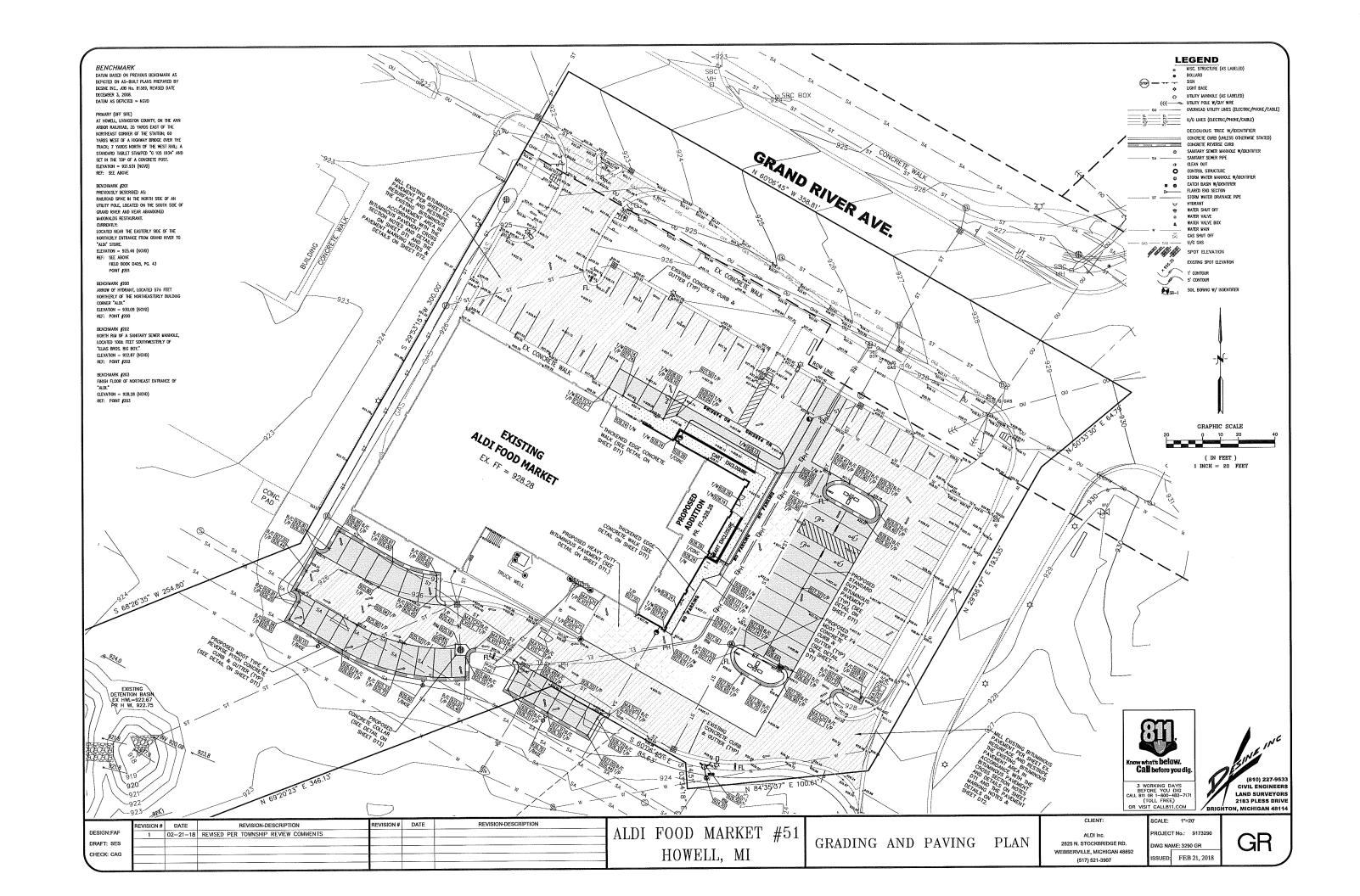
3. The Contractor shall take the necessary precautions to protect any existing underdrain th from damage during site construction, including pavement restoration work. Any existing remain that is damaged during construction shall be repaired and/or replaced as necessary. erdrain that is to rem

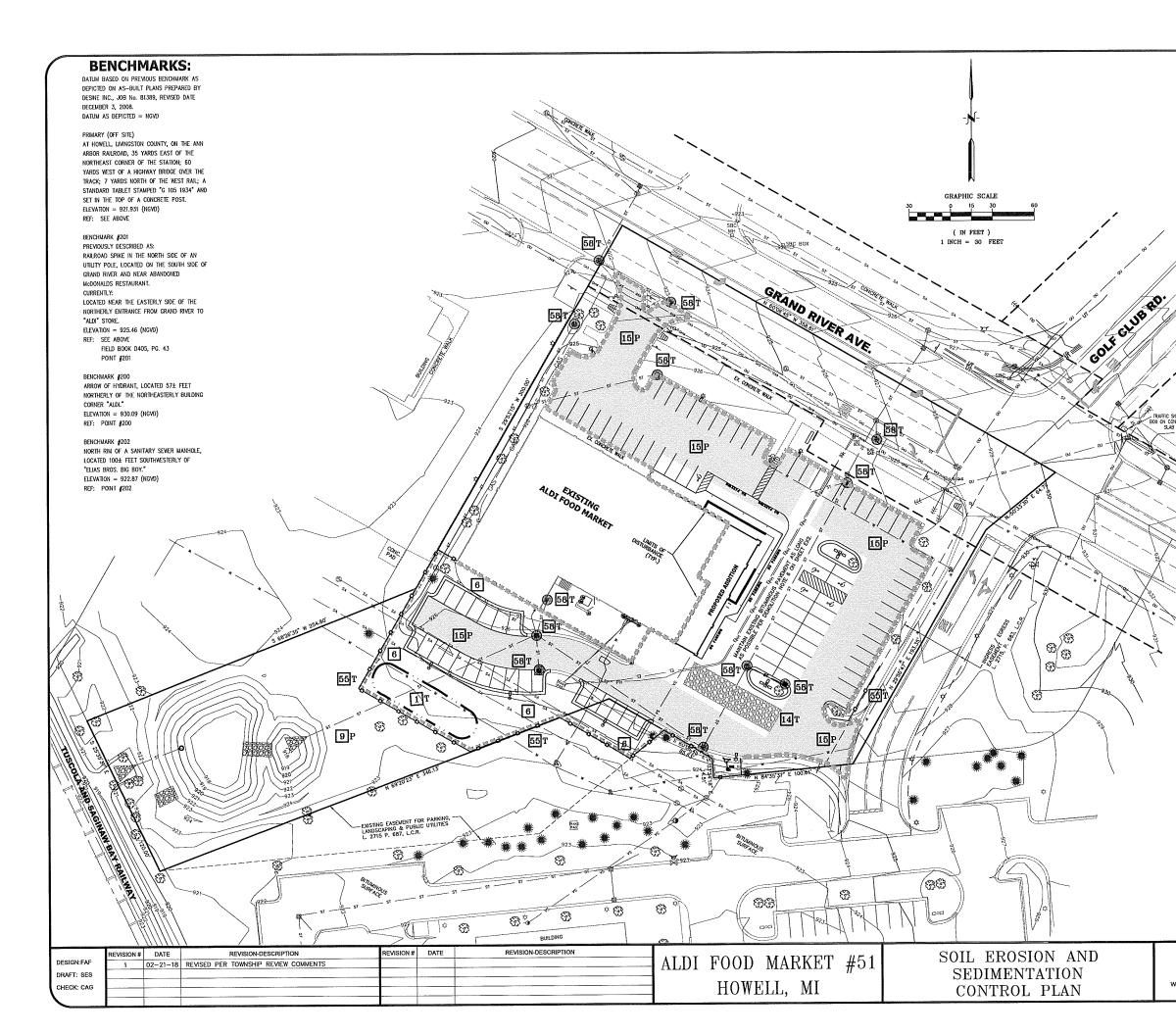
4. Any existing catch basin inlet castings and grates that are to be salvaged for re-use shall be inspecte by the Contractor. Any broken, damaged, missing or inoperable parts shall be replaced prior to re-use

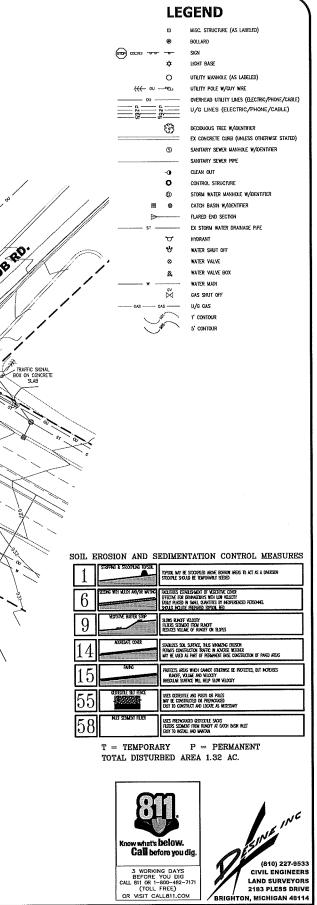


ALDI FOOD MARKET #51 DRAINAGE AREA CALCULATIONS	Project: ALDI FOOD MARKET #51 REMODEL & EXPANSION	Project: # 173290	ALDI FOOD MARKET #51
EXISTING CONDITIONS           Structure         0.90         0.80         0.20         (ACRES)         C* Factor         C x A           RD-3         0.60         0.01         0.00         0.01         0.80         0.01           RD-2         0.00         0.01         0.00         0.01         0.80         0.01           RD-1         0.00         0.01         0.80         0.31         0.13         0.00         0.39         0.80         0.31           132         0.13         0.00         0.04         0.17         0.74         0.13           130         0.20         0.00         0.01         0.24         0.87         0.18           122         0.06         0.00         0.01         0.21         0.87         0.18           121         0.00         0.00         0.00         0.00         0.00         0.14           122         0.08         0.00         0.00         0.00         0.00         0.00         10.44           121         0.06         0.00         0.00         0.00         0.00         0.00         10.04           111         0.40         0.00         0.051         0.51	MH#         MH#         Acres         Area         Time         Inch         (CIA)         of         pipe           CB#         CB#         CB         To%         CC         CA         CA         Min.         Hour         c.f.s.         inch         pipe           FES#         TX*         CC*         CA         CA         Min.         Hour         c.f.s.         inch         inch         inch         pipe           RD 2         WYE 1         0.01         0.80         0.01         0.01         15.0         4.38         0.04         8**         0.04         8**         0.04         8**         0.04         8**         0.00         0.00         0.02         15.4         4.33         0.09         8**         0.01         0.01         0.00         0.00         0.02         15.6         4.31         0.09         8**         0.01 </th <th>Stope pipe         Length H.G.         Vet. of full         Time for full         Cap of flow flow         H.G. of pipe         Ground Elev. Upper end         Invert Elev. Upper end         Invert Elev.           %         %         ft.         flow flow         of flow         of flow         of flow         of pipe         Elev.         Upper end         Lower end         Upper end         Lower end         Upper end         Lower end         23.72         923.72         922.99           0.80         0.01         75         3.10         0.4         1.08         923.67         928.28         927.92         923.59         922.99           0.80         0.01         32         3.10         0.2         1.08         923.19         927.92         927.45         922.99         922.73           0.80         0.01         27         3.10         0.1         1.08         923.18         927.45         922.99         922.73</th> <th>Dependence of the following interval of the formula of the formula</th>	Stope pipe         Length H.G.         Vet. of full         Time for full         Cap of flow flow         H.G. of pipe         Ground Elev. Upper end         Invert Elev. Upper end         Invert Elev.           %         %         ft.         flow flow         of flow         of flow         of flow         of pipe         Elev.         Upper end         Lower end         Upper end         Lower end         Upper end         Lower end         23.72         923.72         922.99           0.80         0.01         75         3.10         0.4         1.08         923.67         928.28         927.92         923.59         922.99           0.80         0.01         32         3.10         0.2         1.08         923.19         927.92         927.45         922.99         922.73           0.80         0.01         27         3.10         0.1         1.08         923.18         927.45         922.99         922.73	Dependence of the following interval of the formula
CRAPHIC SCALE ( IN FEET ) 1 INCH = 10 FEET	ROOF DRAIN & UNDERDRAIN DETAIL PLAN VIEW	ALI Food Market #31 Remodel & Expansion         Control Structure Calculations         C38-101         Thirdray Area:	<section-header><section-header><section-header><section-header><section-header><section-header><section-header><section-header></section-header></section-header></section-header></section-header></section-header></section-header></section-header></section-header>
DESIGN:FAF DRAFT: SES CHECK: CAG	REVISION # DATE REVISION-DESCRIPTION A		OR VISIT CALLBIT.COM     / BRIGHTON, MICHIGA       OR VISIT CALLBIT.COM     / BRIGHTON, MICHIGA       CLIENT:     ALDI Inc.       ALDI Inc.     PROJECT NO.: 9173290       DWG NAME: 3280 UT     UT       LATIONS     (517) 521-3907









CLIENT: ALDI Inc. 2625 N. STOCKBRIDGE RD.

PROJECT No.: 9173290 WG NAME: 3290 SE WEBBERVILLE, MICHIGAN 48892 SSUED: FEB 21, 2018 (517) 521-3907

SCALE: 1"=30'

SE1

SOIL FROSION CONTROL AND CONSTRUCTION SEQUENCE:

Obtain all necessary Soil Erosion and Sedimentation Control related permits from the appropriate Local, County and/or State Agencies. Refer to the General Notes on the project plans for additional requirements.

Prior to commencement of any earth disruption, install Mud Tracking Control Devices and Inlet Sedime Filters at existing storm sewer eatch basins in accordance with the Soil Envision and Sedimentation Control Plan and the Soil Envision and Sedimentation Control Permit.

3. Inspect and maintain all Soil Erosion Control Measures daily. Maintain all Soil Erosion Control Measure

as necessary and as directed by the Engineer and/or the Permitting Agency Perform demolition work. Install appropriate Soil Erosion Control Measures in accordance with the Soil Erosion and Sedimentation Control Plan and/or as directed by the Engineer and/or the Permitting Agency.

5. Construct building in accordance with the Site Plan and Architectural Plans. Install appropriate Soil Erosion Control Measures in accordance with the Soil Erosion and Sedimentation Control Plan and/or as

directed by the Engineer and/or the Permitting Agency.

6. Construct underground utilities including roof drains and underdrains. Install appropriate Soil Erosion Control Measures in accordance with the Soil Erosion and Sedimentation Control Plan and/or as directed by the Engineer and/or the Permiting Agency.

Install light pole bases and fixtures and underground electric. Install appropriate Soil Erosion Control Measures in accordance with the Soil Erosion and Sedimentation Control Plan and/or as directed by the Engineer and/or the Permitting Agency.

8. Construct curb & gutter, sidewalk and paved parking and roadway areas. Install appropriate Soil Erosion Control Measures in accordance with the Soil Erosion and Sedimentation Control Plan and/or as directed by the Engineer and/or the Permitting Agency.

9. Backfill curb and sidewalks and finish grade all disturbed areas outside of pavement areas. Install appropriate Soil Erosion Control Measures in accordance with the Soil Erosion and Sedimentation Co Plan and/or as directed by the Engineer and/or the Permitting Agency.

10. Place topsoil, seed, and mulch within 5 days of finish grade for establishment of vegetative ground cover outside of pavement and decorative stone landscape bed areas. Install appropriate Soil Erosion Control Measures in accordance with the Soil Erosion and Sedimentation Control Plan and/or as directed by the Engineer and/or the Permitting Agency.

11. Landscape site in accordance with the Project Landscape Plan. Install appropriate Soil Erosion Control Measures in accordance with the Soil Erosion and Sedimentation Control Plan and/or as directed by the Engineer and/or the Permitting Agency.

12. Following establishment of sufficient vegetative ground cover and receipt of approval from the Permitting Agency, remove all *temporary* Soil Erosion Control Measures, clean all storm sewer structures, and repair any permanent Soil Erosion Control Measures as directed by the Engineer and/or the Permitting

SOIL EROSION AND SEDIMENTATION CONTROL NOTES:

The Soil Erosion and Sedimentation Control Specifications of the appropriate Local, County and/or State Agencies are a part of this work. Refer to the General Notes on the Project Plans for additional

2. The Soil Frosion and Sedimentation Control (SESC) Permit Holder shall be responsible for compliance with the SESC Permit requirements for the duration of the project and until receipt of final approval from the Permitting Agency. For any site with an earth disturbance area of 1 acre or greater, the SESC Permit Holder shall retian a Certified Storm Water Operator in accordance with the SESC Permit requirements. The Certified Storm Water Operator in accordance with the SESC Permit requirements. The Certified Storm Water Operator shall perform routine inspections of the site and the SESC measures and file inspection reports in accordance with the SESC Permit Holder shall flot a National Pollutant Discharge Elimination System (NPDES) Notice of Coverage Form with the State DEQ prior to any earth disruption.

3. The Contractor shall install the appropriate Soil Erosion Control Measures in accordance with the Project Plans prior to massive earth disruption, including but not limited top mult articking control mats and sediment filters on existing storm sewer structures. Demolition work may be necessary prior to installation of some soil erosion control measures. In such cases, postpone installation of affected soil erosion control measures until immediately following demolition work. Refer to the Project Plans and the Soil Erosion Control and Construction Sequence for additional requirements.

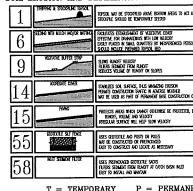
4. The Contractor shall schedule work so as to minimize the period of time that an area is exposed and disturbed. The Contractor shall observe the grading limits and limits of disturbance in accordance with the Project Plans. The Contractor shall maintain an undisturbed vegetative buffer around the work when shown on the Project Plans.

5. The Contractor shall install and maintain Soil Erosion Control Measures in accordance with the Project Plans during the appropriate phases of construction. The Project Plans show the minimum requirements f Soil Erosion Control Measures. The Contractor shall install additional Soil Erosion Control Measures as necessary due to site conditions and as directed by the Permitting Agency and/or Enginee. The Contract shall perform routine inspection and maintenance of all Soil Erosion Control Measures to ensure compliant in the four state of the contract of the Soil Brosion Control Measures to ensure compliant in the four state of the source of the Soil Measures. with the permit requirements and proper operation of the Soil Erosion Control N

6. All disturbed areas outside of paved areas shall be restored within 5 days of finish grading. Proposed vegetative areas shall be restored with a minimum of 3-inches of topsoil, then seeded and mulched, unless noted otherwise on the Project Plans. During the non-growing eason, temporary tabilitzation shall be provided using straw matting or as directed by the Permitting Agency and/or the Engineer.

7. Following complete site restoration and stabilization; sediment shall be removed from all storm sewer structures, paved areas and storm basins. The SESC Permit Holder shall contact the Permitting Agency to request closure of the SESC Permit. For any site with an earth disturbance area of 5 arers or greater, the SESC Permit Holder shall file a NPDES Notice of Termination Form with the State DEQ.

SOIL EROSION AND SEDIMENTATION CONTROL MEASURES



RED AREA 1.32 AC

									TUTAL DISTURBED AREA 1.52 AC.
		REVISION #	DATE	REVISION-DESCRIPTION	<b>REVISION #</b>	DATE	REVISION-DESCRIPTION		SOIL EROSION AND
DE	SIGN: FAF	1	02-21-18	REVISED PER TOWNSHIP REVIEW COMMENTS				ALDI FOOD MARKET #51	
DR	AFT: SES								SEDIMENTATION CONTRO
СН	ECK: CAG							HOWELL, MI	NOTES & DETAILS
									NULES & DETAILS

## TIME LINE OF SOIL EROSION CONTROL AND CONSTRUCTION SEQUENCE

CONSTRUCTION & WORK							C	DN	ST	RL	ICT	ric	N	PE	RI	OD	)								
CATEGORIES*	Month		Ma	Ircl	h		A	oril			Ma	ay			Ju	ne			Jı	ıly		ŀ	۱uç	ប្រទ	st
	Week	1	2	3	4	1	2	3	4	1	2	3	4	1	2	3	4	1	2	3	4	1	2	3	4
1. OBTAIN PERMITS																									
2. INSTALL INITIAL SESC MEASURES	i																					_			
3. INSPECT & MAINTAIN SESC MEAS	URES							(910)				<u>.</u>													
4. DEMOLITION WORK																									
5. BUILDING CONSTRUCTION																									L
6. UNDERGROUND UTILITY WORK																									
7. SITE LIGHTING WORK																			_						
8. CURB, SIDEWALK & PAVEMENT W	ORK		1																					-	
9. BACKFILL & FINISH GRADE WORK	[																								
10. TOPSOIL, SEED & MULCH																									
11. LANDSCAPE WORK																									
12. REMOVE TEMPORARY SESC MEA	SURES									I						L									

\*REFER TO THE MAJOR WORK ITEMS OUTLINED IN THE SOIL EROSION CONTROL AND CONSTRUCTION SEQUENCE NOTES.

SOILS, PROVIDE GEOTEXTILE

EXISTING 

MATCH WIDTH ( CONSTRUCTION ENTRANCE RD; 12' MIN.

2 EACH DUMP STRAP

EXPANSION RESTRAINT

FLAT WASH

NN NN

75' MI

2" DIA. CRUSHED STONE ACCREGATE COMPACTED

14 MUD TRACKING CONTROL DEVICE

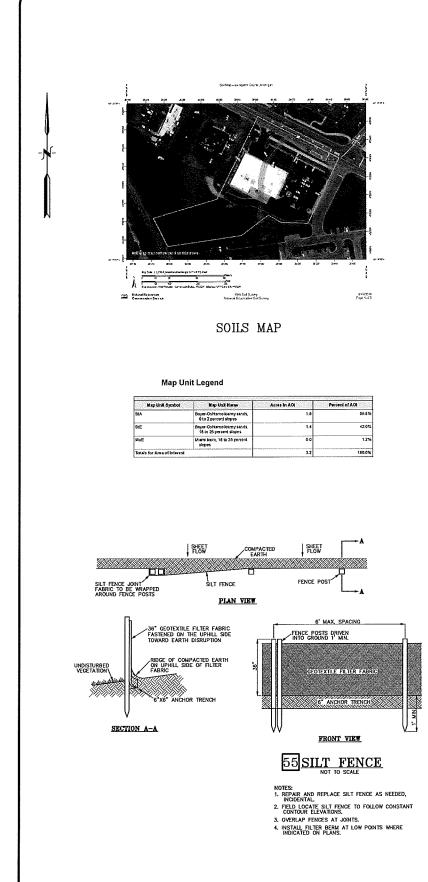
BAG DETAIL

NOTE: WHEN ACCEPTABLE TO ENGINEER, CONTRACTOR MAY INSTALL STONE BELOW THE SUBGRADE ELEVATION; THUS STONE MAY BE LEFT IN PLACE BELOW PAYEMENT.

58 INLET SEDIMENT FILTER

MOUNTABLE BERM, 6 HIGI

INSTALLATION DETAIL



is a dhersion	]
NFI.	
	]
of paid areas	
rt rjenegn tu	
ENT	

SOL

MAINTENANCE NOTES FOR SOIL EROSION CONTROL MEASURES:

The Construction Site and all Soil Erosion Control Measures shall be inspected periodically in accordance with the appropriate local municipality/authority and the MDEQ NPDES rules and regulations. At a MINIMUM, inspections shall be performed once a week and within 24 hours following a storm event resulting in 10 or partater. Inspections shall be performed throughout the duration of the construction process and until the site is completely stabilized. Following construction, the owner (or its assigned) shall periodically inspect all permanent soil erosion control measures to ensure proper operation.

BASIN PERFORATED STANDPIPES / CONTROL STRUCTURES: Standpipes shall be inspected for soil accumulation, soil eaking and mechanical failure/damage. The filter stone around the standpipe shall be removed and replaced each time it becomes clogged with sediment. All mechanical failure/damage shall be repaired immediately.

CATCH BASINS: Catch basins shall be inspected for accumulation of solids and sediment. Solids and sediment shall be removed from the catch basins by vacuum or adductor cleaning. Cleaning should be performed before the catch basin sumps are half full.

MUD TRACKING CONTROL DEVICE / CONSTRUCTION ACCESS: Mud tracking NUD INACKING COMMOD DEVICED CONTROL TO THE CONTROL the public rights or way and/or paved access route shall be performed as necessary to maintain the access route free of sediment and debris.

DETENTION BASIN (WET BOTTOM): Wet bottom detention basins shall be inspected to DETENTION BASIN (WET BOTTOM): Wet bottom detention basins shall be inspected to ensure erosion is not occurring along the inlet locations, banks and/or bottom of the basin and for sediment and/or algae accumulation. Regular maintenance of the basin includes routine mowing of the buffer/filter strip and side slopes and removal of litter and debris accumulation. Address vegetation and/or ension concerns as soon as weather permits. Remove sediment from basin every 5 to 10 years or sconer if sediment accumulation adversely affects the operation of the basin. Sediment that is removed shall be disposed of offsite or at an upland area and sublized so that it does not re-enter the drainage course. Excessive algae shall be removed as necessary to prevent odors and to maintain nutrient removal cancerity. removal capacity

SEDIMENTATION BASINS: Sedimentation basins shall be inspected to ensure erosion is not occurring along the inlet locations, banks and/or bottom of the basin and for piping, seepage, sediment accumulation and/or other mechanical damage. Regular maintenance of the basin includes routine moving of the buffer/filter strip, side slopes and basin floor and removal of litter and debris accumulation. Address vegetation and/or ension concerns as soon as weather permits. Sediment shall be removed hall be disposed of offsite or at an upland area and stabilized so that it does not re-enter the drainage course.

RIPRAP: Inspect riprap immediately following the first rainfall event following installation of the riprap. Continue to perform inspections of the riprap at each periodic site inspection. Riprap shall be inspected to ensure erosion is not occurring within and/or around the riprap. The discharge point shall be inspected to ensure that concurring the not eauxing erosion downstream. Displaced riprap shall be removed from downstream locations and the riprap beds shall be repaired or replaced. Significant sediment buildup shall be removed from riprap beds. Repair or replace failing or displaced riprap immediately. Address vegetation and/or erosion concerns as soon as weather permits.

SEEDING: Newly seeded areas shall be inspected until substantial vegetative growth is obtained. Seeded areas shall be inspected to ensure erosion is not occurring in the seeded area and vegetative growth is promoted. Eroded areas shall be finish graded as an eccessary to removal erosion channels or gulleys and new seed placed as soon as weather permits.

SILT FENCE: Silt fencing shall be inspected for soil accumulation/clogging, undercutting, overtopping and sagging. Soil accumulation shall be removed from the face of the silt fence each time it reaches half the bright of the fence. Removed sediment shall be disposed of in a stable uphand site or added to a spoils stockpile. When undercutting occurs, grade out a stable uphand site or added to a spoils stockpile. a stable upland site of added to a spoils stockpile. When undercluing occurs, guade our areas of concernated flow upstream of the silf faces to remove channels and/or guilleys and repair or replace silf fence ensuring proper trenching techniques are utilized. Silt fencing, which sags, falls over or is not staked in shall be repaired or replaced immediately. Silt fencing fabric, which decomposes or becomes ineffective, shall be removed and replaced reficing fabric, which decomposes of becomes induced with set and the imposed and the set of the se

SOD: Newly sodded areas shall be inspected to ensure sod is maturing. Sod shall be inspected for failure, erosion or damage. Slipping or eroding sod on ateep slopes shall be immediately replaced and staked in place. Damaged or failed sod shall be immediately replaced.

SPILLWAYS: Spillways shall be inspected to ensure that erosion is not occurring within and/or around the spillway. The discharge point shall be inspected to ensure that concentrated flows are not causing crosion downstream. Inspect the spillway for cracked concrete, unverse and/or excessive settiling and proper function. Repair or replace failing spillways immediately. Address vegetation and/or erosion concerns as soon as weather

STOCKPILES: Temporary and permanent topsoil and spoils stockpiles shall be seeded to promote vegetative growth. Stockpiles shall be inspected to ensure excessive erosion has not occurred. When runoff or wind erosion is evident, reduce the side slopes of the stockpile tor stabilize the stockpile with pieces of stated sod laid perpendicular to the slope. When filter fencing is used around a stockpile, the fencing shall be inspected to ensure piping has not occurred under the fencing and to ensure the fencing has not collapsed due to soil slippage or access by construction equipment. Repair or replace damaged fancing immediately. Berms at the base of stockpiles, which become damaged, shall be replaced.

STORM STRUCTURE INLET FILTER: Inlet filters shall be inspected for sediment SI OKM SI ROCI OKE INEET FLEIRE, inter inters and or inspected to sconnait accumulation, clogging and damage. When store is used in conjunction with linker filter fabrie, replace the stone each time it becomes clogged with sediment. Clean or replace the inlef filter fabries acht mine it becomes clogged with sediment. Existall or replace fallen filter fabries immediately. Replace damaged filter fabries immediately.

## NOTE: SEE LANDSCAPE PLAN FOR SEED AND SOD REQUIREMENTS.



(TOLL FREE)

OR VISIT CALLBIT.COM

CALE:

SUED

NONE ROJECT No.: 9173290

FEB 21, 2018

WG NAME: 3290 SE

CLIENT

ALDI Inc.

2625 N STOCKBRIDGE RD.

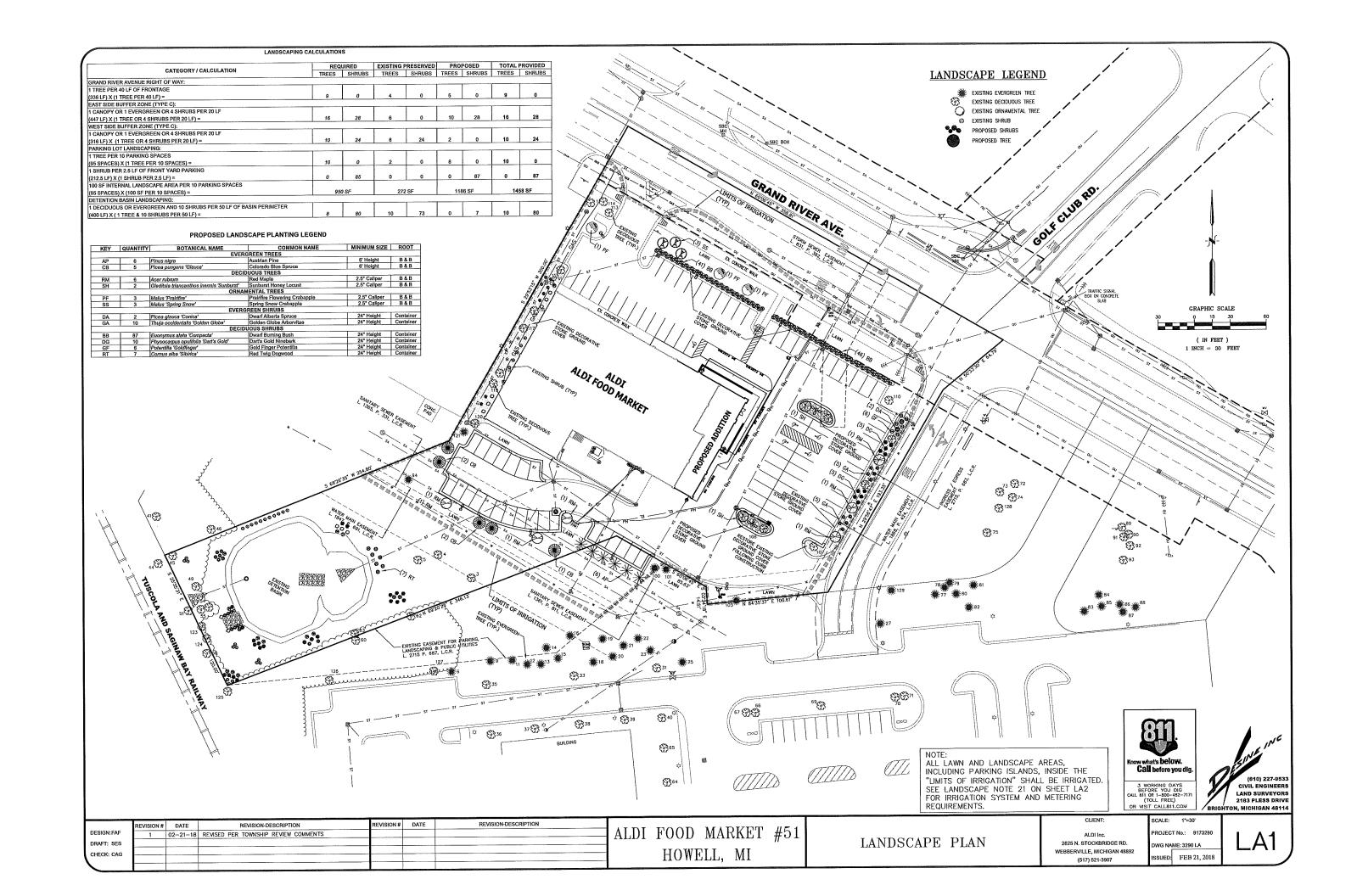
WEBBERVILLE, MICHIGAN 48892

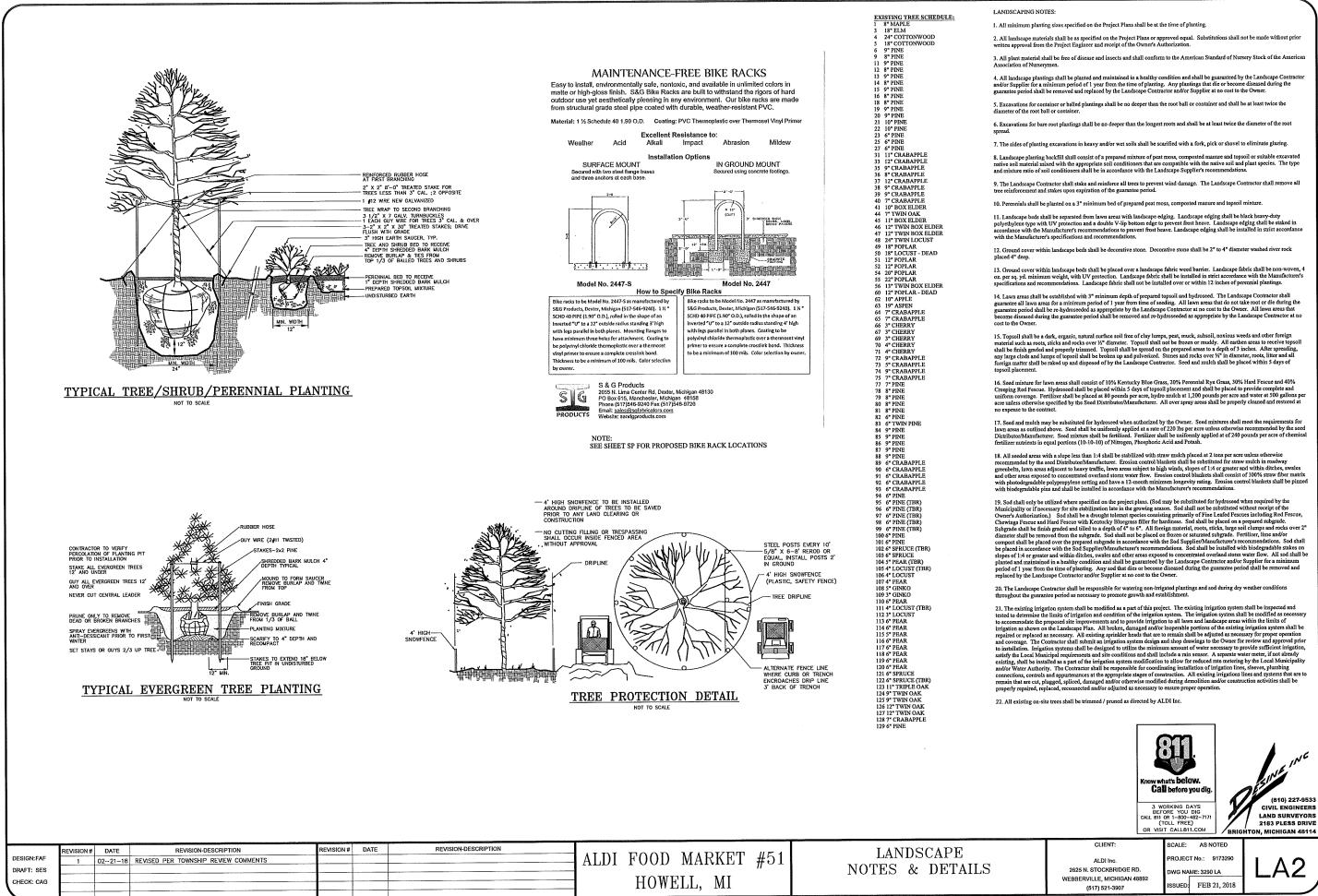
(517) 521-3907

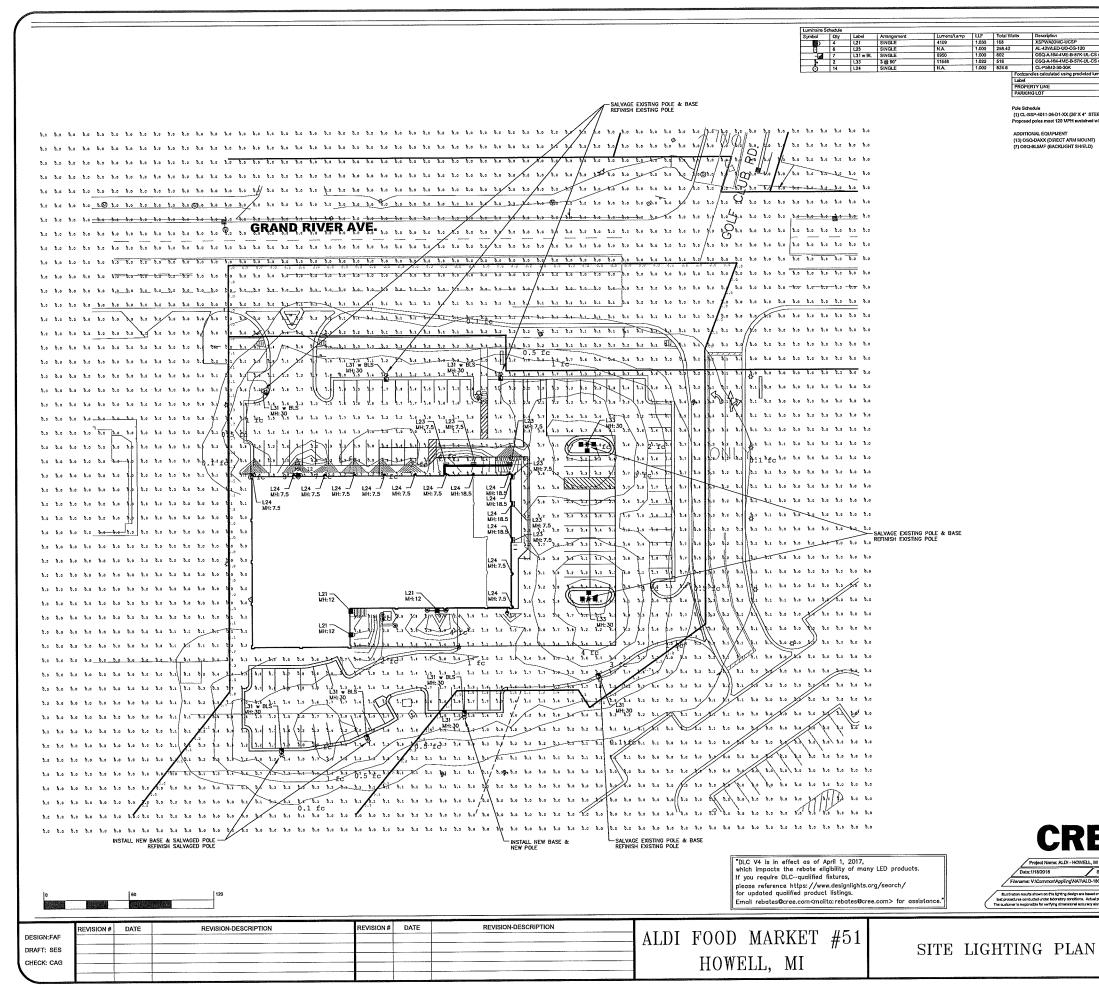


LAND SURVEYOR 2183 PLESS DRIV BRIGHTON, MICHIGAN 4811

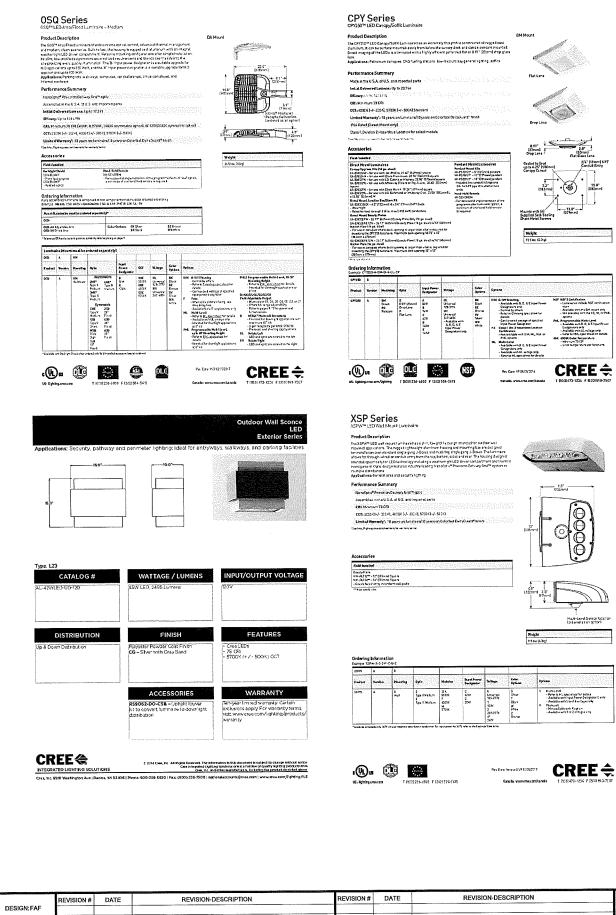


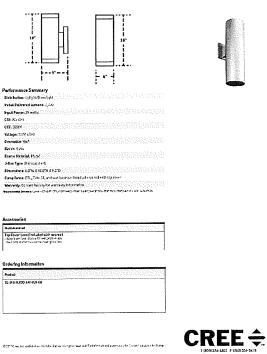


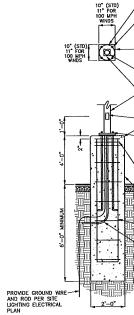




w/05Q-DACS OSC-BLSWF           w/05Q-DACS           w/05Q-DACS           men values after 50K hours of openation           Avg         Max           0.34         1.8           0.34         1.8           0.34         1.8	
w0SQ-DACS men vulues after 50K hours of operation May 1 Max 1 Min / Avg/Min 1 Max/Min	
w0SQ-DACS men vulues after 50K hours of operation May 1 Max 1 Min / Avg/Min 1 Max/Min	
w0SQ-DACS men vulues after 50K hours of operation May 1 Max 1 Min / Avg/Min 1 Max/Min	
Avg Max Min Avg/Min Max/Min	
ANG MAX Mat Arginal Matrices	
2.32 9.5 0.4 5.60 23.75	
EL SQUARE POLE)	
vinds.	
	1
	— <b> </b>
	.
Know what's belo	w
Call before y	ou dig.
	1
3 WORKING D/ BEFORE YOU CALL BIO RI 1-800 (TOLL FREE)	DIG 82-7171
(TOLL FREE OR VISIT CALLBI	.сом
	ING
1200 92/04 Street - Stationert, MI 531/7 ver ansame (2003 226 6600	
	227-9533
CIVIL EN	GINEERS
project conditions differing from heas and an personal may affect field results.	S DRIVE
/ BRIGHTON, MICHIO	N 48114
CLIENT: SCALE: NONE	
ALDI Inc. PROJECT No.: 9173290	4
2625 N. STOCKBRIDGE RD. DWG NAME: 3290 LT WEBBERVILLE, MICHIGAN 48892	1 }
(517) 521-3907 ISSUED: JAN 29, 2018	







All existing on-site pole mounted light fixtures are to be removed in accordance with the project plana. The existing light poles and bases are to be salvaged where feasible. The existing electric supply is to be salvaged unless noted otherwise on the Site Electrical Plan.

2. All existing light poles shall be refinished. Remove all existing rust, scale, chipped and/or peeling finish. Prepare the surface, prime and paint all light poles in accordance with the Project Manual painting specifications and requirements for exterior finishes. Paint color shall be slate gray.

3. The proposed pole mounted light fixtures shall be mounted onto the existing light poles in accordance with the light fixture manufacturer's recommendations. Any new mounting holes shall be drilled and all other necessary modifications to the existing poles shall be performed prior to refinishing of the existing poles.

Existing exterior building mounted light fixtures shall be removed and replace in accordance with the project plans. See the building plans for additional information and specifications.

5. Contact the ALDI National Account Lighting Distributor to order fixtures, mounting accessories and pole(s): Mike Kreiner Strategic Director - National Accounts Cree Lighting 9201 Washington Avenue Racine, WI 35406 Offlice: 262-504-5037 Cult 202. 651. 1661

Cell: 224-250-1561

6. CAUTION!

6. CAUTION! This site contains existing underground public and private utilities. See the project plans for locations of the known existing and proposed underground utility locations. Existing utility information provided on the project plans may be from information disclosed to this firm by the Utility Companies, Local, County or State Agencies, and/or various other sources. No guarantee is given as to the completeness or accuracy thereof. The Site Electrical Contractor shall contact the 811 Underground Public Utility Locating System or other appropriate local underground utility locating, Agency, a minimum of three (3) working days prior to performing the site electrical work. The Site Electrical Contractor shall field locate all potential utility conflicts and shares to the mages to the site integration to a posid damage to the rest of the other sources to rest many states to the site integration utilities. And assays to the source of the other sources to a posid damage to the rest integration utility. performing the site electrical work. The Site Electrical Contractor shall field locate all potential utility conflict and take the necessary precautions to avoid damage to the existing underground utilities. Any damage to the existing utilities as a result of the site electrical work shall be immediately brought to the attention of the General Contractor and shall be repaired as acceptable to the Owner, the General Contractor and the Appropriate Utility Provider. If the existing site conditions create a conflict and/or prevent the Site Electrical Contractor from performing the site electrical work, then contact the Engineer of Record.

7. Any existing light pole that cannot be salvaged due to damage, corrosion and/or other conditions shall be replaced. Replacement poles shall be CREE Straight Steel Poles 26'x4', unless directed otherwise by CREE. The Contractor shall field measure the bolt pattern of the existing base and order the replacement pole with the correct base installed. The Contractors that provide ALDI with an alternate bill price per pole to remove an existing pole and install a new replacement pole in lieu of refinishing. The bid price shall include a credit for light pole of robibing. light pole refinishing.

8. Any existing anchor bolts, within an existing light pole base that is to remain, that are in need of replacement due to corrosion, damage and/or other circumstances, shall be removed and replaced with retrofit anchor bolts (Hill or equal) in accordance with the light pole and anchor bolt manufacturer's specifications and recommendations. The Contractor shall provide ALDI with an alternate bid price per anchor bolt to remove an existing anchor bolt from an existing light pole base and install a retrofit anchor bolt.

9. Any existing light pole bases that cannot be salvaged due to damage, corrosion and/or other conditions shall be replaced. Replacement bases shall be constructed in accordance with the specifications provided on the project plans. The Contractor shall provide ALD with an alternate bid price pr base to remove an existing light pole base and install a new cast in place concrete light pole base in accordance with details provided on the provided in the specifications.

No Alternate Work shall be performed without written receipt of authorization from ALDI to complete said alternate work.

	REVISION #	DATE	REVISION-DESCRIPTION	<b>REVISION #</b>	DATE	REVISION-DESCRIPTION			11 1 1	SITE LIGHTING
DESIGN: FAF							-IALDI FOOD M	MARKET	#51	
DRAFT: SES										NOTES AND DETAILS
CHECK: CAG							HOWEI	LL, MI		

SITE LIGHTING NOTES:

18" Cylinder LED Sconce

mike kreiner@cree.com <mailto:mike kreiner@cree.com>

NOT TO SCALE

NOTES 1. FOUNDATION SHOWN IS A TYPICAL DESIGN. WND LOADS MORE THAN 100 MPH AND/OR UNSTABLE SOL CONDITIONS MAY RECURSE AN ALITERNATE DESIGN. VENETY CONDITION OF SOLS WITH SOLS REPORT. 2. FOUNDATIONS SHALL EXTEND BELOW FROST DEPTH PER LOCAL CODES.

3. CONCRETE SHALL HAVE WIN 3000 PSI COMPRESSIVE STRENGTH AT 28 DAYS.

4. SEE SITE LIGHTING ELECTRICAL PLAN WITHIN ARCHITECTURAL PLANS FOR ADDITIONAL REQUIREMENTS. 5. VERFY BASE PLATE BOLT PATTERN WITH POLE MANUFACTURER AND / OR SUPPLIER PRIOR TO CONSTRUCTION.

PLAN VIEW

STRAIGHT SQUARE-

HAND HOLE GROUND LUG BASE COVER BASE PLATE FOUNDATION (SEE DETAIL).

ENVISION CRADE



z

-3/4" TO 1" GAP BETWEEN BOTTOM OF BASE PLATE AND TOP OF FOUNDATION, FILL GAP WITH NON-SHRINK GROUT.

9 1/2" BOLT CIR. (STD.) /11" CIR. (100 M.P.H. WINDLOADS)

(4) 1 DIA. HOLES (STD.) (4) 1 1/4\*x11 3/4\* SLOTTED HOLES (100 M.P.H. WNDLOADS)

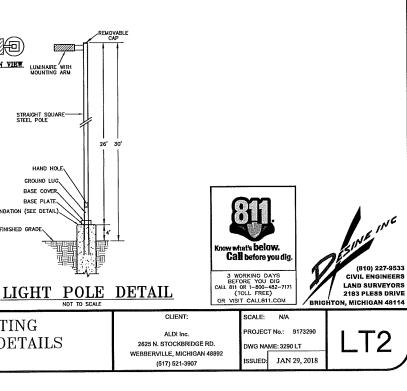
-3/4" STL BASE PLATE (STD.) 1" (100 M.P.H.+ WINDLOADS) 11" SQ. (100 M.P.H.+ WINDLOADS) "3" DIA. HOLE IN BASE AND FOR CONDUIT ACCESS.

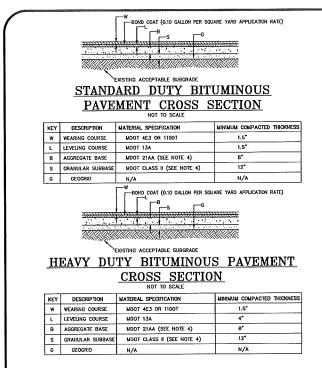
4" SQ. STEEL LIGHT POLE 'STANDARD' 5" SQ. STEEL LIGHT POLE (100 M.P.H.+ WIND LOAD AREAS) -2" HAND HOLE ~5/8" CHAMFEF

(4) 3/4\*,30° x4\* ANCHOR BOLTS WTH 2 3/4\* MIN. BOLT PROJECTION (STD.) (4) 1\*358\*,4\* ANCHOR BOLTS W/ 3 1/2\* MIN. BOLT PROJECTION (100 M.P.H.+ WINDLOADS) 3 43 TES 0 2 0.C.

TOP OF PAVEMENT/FINISH GRADE 10'-0" HIGH RIGID FIBER FORM (SONOTUBE) FORM FILLED WITH 4000 PSI CONCRETE (8) #6 BARS W/ #3 TIES O 12 LOCATE ALTERNATE LOCATIONS FROM ANCHOR BOLTS "3/4" HEAVY WALL CONDUIT EXTEND TO BUILDING ELECTRICAL SERVICE

## LIGHT POLE BASE STANDARD DETAIL





BITUMINOUS PAVEMENT CROSS SECTION NOTES:

The construction specifications of the Local Municipality are a part of this work. Refer to the General Notes and the Bituminou Pavement Cross Section Details on the Project Plans for additional requirements.

2. The Geotechnical Evaluation Report for the project site is a part of this work. The General Contractor, Earthwork Subcontractor, and Pavement Subcontractor shall obtain, review, and become familiar with the Geotechnical

3. The bituminous pavement cross section specifications are based on typical weather conditions during the June through September Construction Season. If the bituminous parking area and/or bituminous driveways are to be constructed during any other time of the year and/or if weather conditions are unseasonably wet, then modifications to the bituminous pavement cross section specifications are seen and the section of the e necessary. If either of these conditions exists, then contact the Material Testing Engineer and/or the Project Engineer for additional

4. The existing granular subbase and aggregate base materials are to be left in place and salvaged for reuse to the greatest extent feasible. The existing aggregate base material shall be finish graded and compacted to a minimum of 95% of the maximum mit weight, Modified Prottor, prior to placement of the bituminous leveling course. In areas of new pavement and in locations where the existing granular subbase and/or aggregate base materials cannot be salvaged, provide the appropriate subbase-materials in accordance with the Bituminous Pavement of additional aggregate base material aggregate base materials examets of any emerical aggregate base materials in accordance with the Bituminous Pavement of additional aggregate base material, provide the appropriate subbase-have aggregate base material in accordance with the Bituminous Pavement Cross Section Details and Specifications provided on the Project Plans.

5. Any existing subgrade soils that are exposed during construction procedures shall be prepared in accordance with the Geotechnical Evaluation Report prior to placement of the granular subbase material, including fine grading and compaction to a minimum of 95% of the maximum unit weight, Modified Protor. Unsuitable soils found within the 1 on 1 influence zone of the propored payment areas, such as muck, peet, topooil, maximi, soil or outer unstable materials shall be excerted and replaced with structural fill shall be been structured. The Material Statistical Evaluation Report. The bitminum of 95% of the maximum unit weight, Modified Protor. Unsuitable soils found within the 1 on 1 influence zone of the propored payment areas, such as muck, peet, topooil, maximilar material placed in accordance with the General Notes on the Project Plans and the Geotechnical Evaluation Report. The Material Testing Engineer and/or the Project Engineer shall be profored in accordance with the Generatical Evaluation Report. The Material Testing Engineer and/or the Project Engineer shall be subgrade proforof. Lareas of subgrade tabilization may be considered when recommended by the Material Testing Engineer. Alternative means of subgrade stabilization may be Considered when recommended by the Material Testing Engineer. Alternative methods shall not be maximum and the Subgrade Data of Data Stabilization may be considered when recommended by the Material Testing Engineer. Alternative methods shall not be maximum and the Subgrade Data of Data Stabilization may be considered when recommended by the Material Testing Engineer. Alternative methods shall not be maximum and the Subgrade Data Stabilization may be considered when recommended by the Material Testing Engineer. Alternative methods shall not be maximum and the subgrade Data Stabilization may be considered when recommended by the Material Testing Engineer. Alternative methods shall not be maximum and the subgrade Data Stabilization may be considered when recommended by the Mat ned without receipt of the Owner's Authorization.

6. The bituminous pavement granular subbase material shall be MDOT Class II sand. No granular subbase material substitutions shall 6. How of manufacture determines a second of the Project Engineer and receipt of the Owner's Authorization. The granular subbase shall be compacted to a minimum of 95% of the maximum unit weight, Modified Proctor.

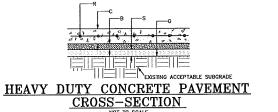
7. The bituminous pavement aggregate base material shall be MDOT 21AA crushed angular limestone or crushed angular natural stone aggregate material. Crushed concrete shall NOT be utilized for the standard or heavy duty bituminous pavement aggregate base. No aggregate hase material substitutions shall be permitted without prior written approval of the Project Engineer and receipt of the Owner's Authorization. The aggregate base shall be compacted to a minimum of 95% of the maximum unit weight, Modified Proctor.

8. The bituminous pavement leveling course material shall be MDOT 13A bituminous material placed in 1 lift. The bituminous pavement warring course material shall be MDOT 453 or MDOT 1100T bituminous material placed in 1 lift. The bituminous pavement leveling and wearing courses shall NOT be combined into a single course. No bituminous material substitutions shall be permitted without prior written approval of the Project Engineer and receipt of the Owner's Authonization. Compaction of the leveling course shall be removed prior to placement of the wearing course. The bond coat shall be sprayed on the leveling course shall be removed prior to placement of the wearing course. The bond coat shall be sprayed on the leveling course of placement of the wearing course. The bituminous pavement material shall be compacted to a minimum of 95% of the 50-blow Marshall Density.

9. Placement of the bituminous pavement leveling course and bituminous pavement wearing course shall be performed in two separatum obilizations. Placement of the bituminous pavement wearing course shall be postponed as directed by the General Contractor and/or the Owner until the majority of the construction and/or the construction and/or the bituminous very be necessary due to construction traffic and/or any delay in placement of the bituminous very derived by the bituminous very derived by the derived by the bituminous very derived by the bitumino epaired as directed by Material Testing Engineer and/or Owner prior to placement of the bituminous wearing cou

10 The existing hituminous pavement to remain shall be resurfaced where specified on the project plans. In areas where the proposed 10. The existing bituminous pavement to remain shall be resurfaced where specified on the project plans. In areas where the proposed top of pavement elevation is existing bituminous pavement shall be milled to a depth of 1.5<sup>5</sup> below the PROPOSED top of pavement elevation. In areas where the proposed top of pavement elevation is 1.5<sup>5</sup> or greater above the existing top of pavement elevation. In areas where the proposed top of pavement elevation is pavement elevation is 1.5<sup>5</sup> or greater above the existing top of pavement elevation. In areas where the proposed top of pavement elevation is pavement elevation is 1.5<sup>5</sup> or greater above the existing top of pavement elevation. In areas where the proposed top of pavement elevation is pavement elevation is 1.5<sup>5</sup> or greater above the existing top of pavement elevation. In accordance with bituminous pavement leveling ecourse material shall be placed over the existing bituminous pavement in accordance with bituminous pavement eross section note 8 prior to placement of the fmall bituminous pavement werange course. In locations where the ornspaced thickness of the leveling wedge will exceed 2.5<sup>s</sup>, then the leveling wedge shall be placed in multiple courses not to exceed 2.5<sup>s</sup> compacted thickness per course.

11. Bituminous mix designs shall be developed in accordance with the MDOT HMA Production Manual. The Contractor shall submit the bituminous pavement mix designs to the Material Testing Engineer for review and approval a minimum of 3 business days prior to use. Bituminous pavement work shall not commence without receipt of the Material Testing Engineer's approval of the bituminous mix design. The bituminous pavement mix design shall be a virgin mix. RAP mixtures shall not be utilized without prior written approval of the Material Testing Engineer's approval of the bituminous mix of the Material Testing Engineer's approval of the bituminous mix of the Material Testing Engineer's approval of the Material Testing Engineer's approare and receipt of the Owner's authorization. RAP mixtures, if authorized, shall be designed and produces in accordance with MDOT Tire I or Tire II RAP Mixture Specifications. In no instance shall MDOT Tire II or non-MDOT RAP aixtures be permitted or utilized



KEY	DESCRIPTION	MATERIAL SPEC.	MIN. THICKNESS
R	REINFORCEMENT	SEE NOTE 8	SEE NOTE 8
С	CONCRETE	MDOT P1-1A - 6 SACK	8"
θ	AGGREGATE BASE	N/A	N/A
s	GRANULAR SUBBASE	MDOT CLASS II	12"
G	GEOGRID	N/A	N/A

CONCRETE PAVEMENT CROSS SECTION NOTES:

The construction specifications of the Local Municipality are a part of this work. Refer to the General Notes and the Heavy Duty Concrete Pavement Cross Section Detail on the Project Plans for were set of the Section Detail on the Project Plans for the Section Detail on the Section Detail on the Sect

2. The Geotechnical Evaluation Report for the project site, is a part of this work. The Genera actor. Farthwork Subcontractor and Concrete Pavement Subcontractor shall obtain, review and become familiar with the Geotechnical Evaluation Report.

3. The concrete pavement cross section specifications are based on typical weather conditions during 6. The concrete payement ross section spectrations to asset on typical weather constructed he June through Spectromer Construction Season. If the concrete payement areas are to be constructed furing any other time of the year and/or if weather conditions are unseasonably wet, then modifications to the concrete payement cross section specifications may be necessary. If either of these conditions exists, then contact the Material Testing Engineer and/or the Project Engineer for additional exists, then c

4. The existing subgrade soils shall be prepared in accordance with the Geotechnical Evaluation Report. Unsuitable soils found within the 1 on 1 influence zone of the proposed pavement areas, such as muck, peat, topsoil, marl, silt or other unstable materials shall be excavated and replaced with structural fill. Structural fill shall be MDOT Class II granulum material placed in accordance with the General Notes on the Project Plans and the Geotechnical Evaluation Report.

5. The concrete pavement subgrade shall be prepared and proof rolled in accordance with the Geotechnical Evaluation Report. The Material Testing Engineer and/or the Project Engineer shall observe the subgrade proof roll. Areas of subgrade that do not pass a proof roll inspection shall be undercut in accordance with the Subgrade Undercut Notes and Details on the Project Plans. Alternative means of subgrade stabilization may be considered when recommended by the Material Testing Engineer. Alternative methods shall not be performed without receipt of the Owner's Authorization.

6. The concrete pavement compacted subbase material shall be MDOT Class II granular material. No subbase material substitutions shall be permitted without prior written approval of the Project Engineer and receipt of the Owner's Authorization. The subbase shall be compacted to a minimum of 95% of the maximum unit weight, Modified Proctor.

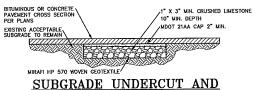
7. Concrete material shall be MDOT P1 (I-A) 6.0 sack concrete pavement mixture with a minimum 28-day design compressive strength of 4,000 PSI and 6.5% (+/1.5%) entrained air. The Contractor shall submit concrete mix design and aggregate mechanical analysis report to the Material Testing Engineer for review and approval prior to use.

eavy Duty Concrete Pavement placed within the Truck Well shall be Reinforced with epoxy coated med #5 bars at 12" on center each way placed at mid-depth of the concrete, unless noted otherwise 8. Heavy Duty Conon the Project Structural Plan

9. Install transverse contraction joints and longitudinal contraction joints at the locations specified on the Project Structural Plans. Joints shall be 2° deep, unless noted otherwise on the Project Structural Plans. Tool joints in fresh concrete or saw cut within 4 hours after placement with soft cut saws.

10. Provide 1" asphalt fiber control joint between concrete pavement and all other concrete structures such as concrete building foundations, concrete curb and concrete sidewalks.

11. The Concrete Pavement shall not be exposed to vehicular traffic until the concrete has reached at least 75% of the design flexural strength



REPLACEMENT CROSS-SECTION

I A VEMENTI SUPUKADE UNDERCUT NOFES: 1. Areas of payment subgrade that do not pass a proof roll inspection shall be undercut when directed by the Material Testing Engineer and/or Project Engineer. All undercut work shall be witnessed and field measured by the Material Testing Engineer and/or Project Engineer. Copies of the field notes depicting the field measurements of the undercut areas shall be provided to the General Contractor and/or Earthwork Subcontractor and ALDI Inc. PAVEMENT SUBGRADE UNDERCUT NOTES:

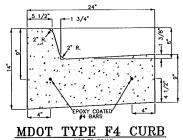
2. Undercut areas shall be excavated to a depth of 12" below the proposed subgrade elevation using an Excavator or Backhoe with a Smooth Edged Ditching Bucket so as not to scarify the underlying soils. Undercut areas shall remain free of all construction traffic and equipment to avoid ruting and/or tracking of the underlying soils

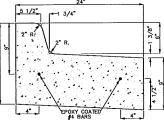
3. Mirafi HP 570 Woven Geotextile Fabric (or approved equal) shall be placed over all undercut areas per the Manufacturer's specifications. Overlap all seams a minimum of 12" unless specified otherwise by the

4. Backfill the undercut areas with 1" x 3" minimum size crushed angular limestone up to the proposed subgrade elevation. Crushed concrete material shall NOT be substituted for crushed limestone material subgrade elevation. Crushed concrete material shall NOT be substituted for crushed limestone materi The backfill material shall be spread with a Wide Track Dozer to minimize loading on the underlying soils. Static of the backfill material with a large smooth drum roller.

. Construct the appropriate Bituminous or Concrete Pavement Cross Section over the undercut areas pe

6. The General Contractor and/or Earthwork Subcontractor shall provide ALDI Inc with unit pricing perform subgrade undercut work per square yard (SY) of undercut area. Undercut Unit Pricing SH/ include excavation, loading, hauling and offisite disposal of excess spoils, placement of geotextile fa and backfil including all labor, equipment and materials necessary to complete pavement subgrade undercut work as specified on the Project Plans.





## MDOT TYPE F4 CURB **REVERSE PITCH**

CONCRETE CURB NOTES: 1. Refer to the project plans for the proposed locations of the specific curt types

The construction specifications of the appropriate Local Municipality are a part of this work. Refer to the General Notes and Curb Cross Section Details on the project plans for additional requirements.

3. Extend the base and/or subbase material of the appropriate adjacent pavement cross-section horizontally to 1 foot behind the back of curb. Concrete curb shall be constructed on no less than 6" of combined depth of unnacted base/subbase material.

4 Concrete material shall be MDOT P1 (I-A) 6.0 sack concrete pavement •. Concrete instead and set MAAT FT (VA) ON SEC CONCRETE payement, instrume with a minimum 28 day design compressive strength of 4,000 PSI and 6.5% (+/-1.5%) entrained air. Contractor shall submit concrete mix design and aggregate mechanical analysis report to the Local Municipality and Engineer for review and approval prior to use.

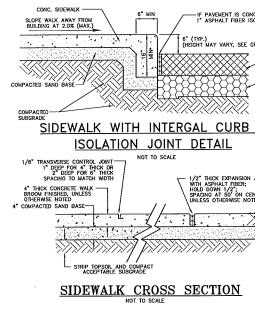
5. Install transverse contraction control joints in concrete curb with 1' num depth at 10' on center. Tool joints in fresh concrete or saw cut within 8 hour

6. Install transverse expansion control joints in concrete curb as follows 6. install narrows expansion control points in control curve strongs. 400° maximum on center, at spring points of intersecting streets and within 10° on each side of catch basins. Transverse expansion control joints shall be 1° thick asphalt fiber joint filler matching entire curb cross section.

7 Provide 1" asphalt fiber control joint between back of curb and all other es, such as concrete sidewalks and concrete drive

Curb Contractor shall provide final adjustment of catch basin castings in curb line. Castings shall be tuck pointed to structure water tight with concrete or mortar inside and outside of casting.

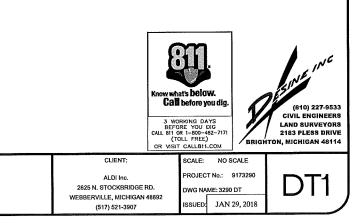
9. Install curb cuts for all existing and proposed sidewalks and pedestrian ramps in accordance with the American Disabilities Act and the Barrier Tree Design requirements of the appropriate Local, County and/or State Agency. Refer to MDOT Standard Plan R-28, latest revision. Install curb suts for all existing and proceeding the state of the sta sting and proposed vehicular ramps and drives as noted on the project plans

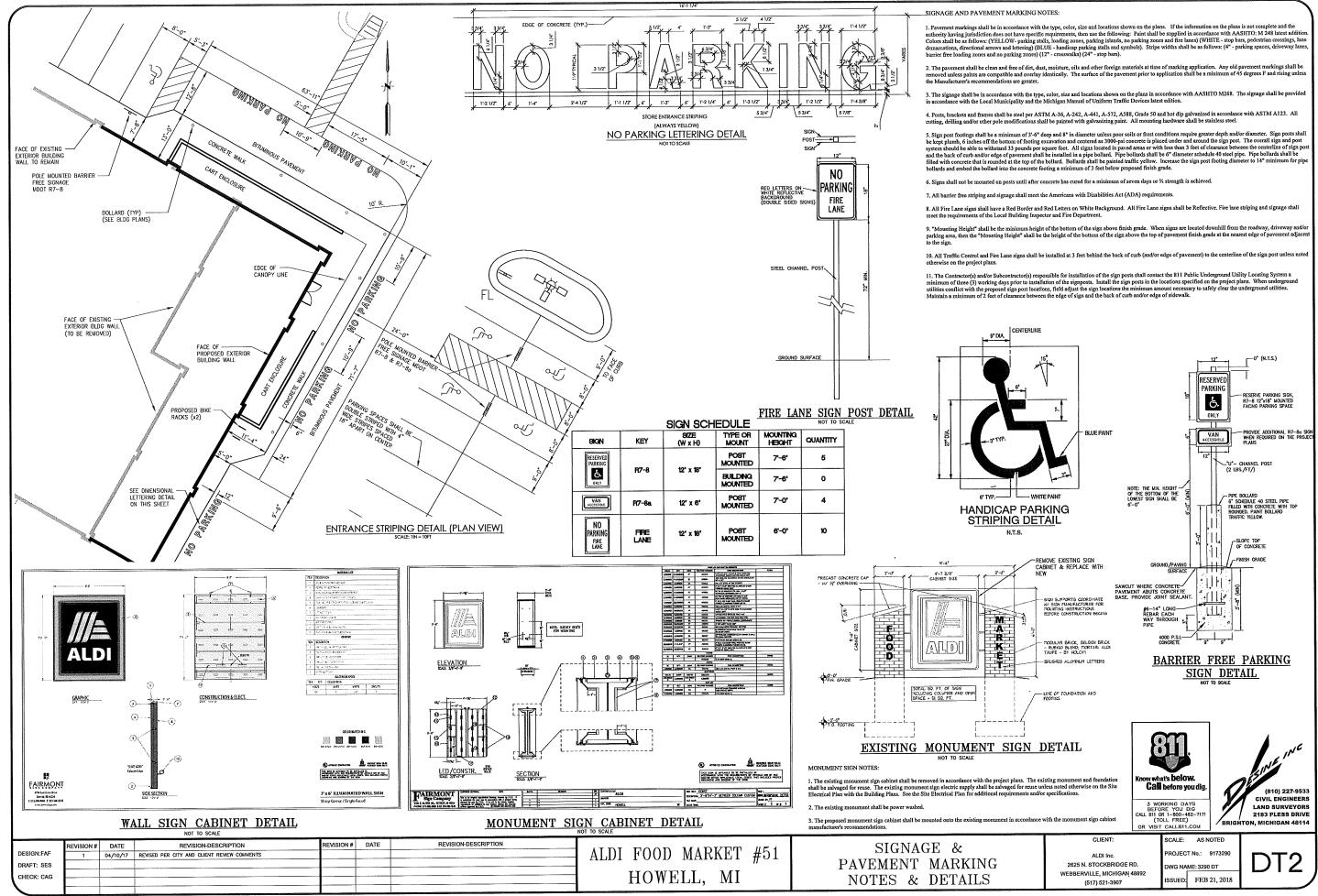


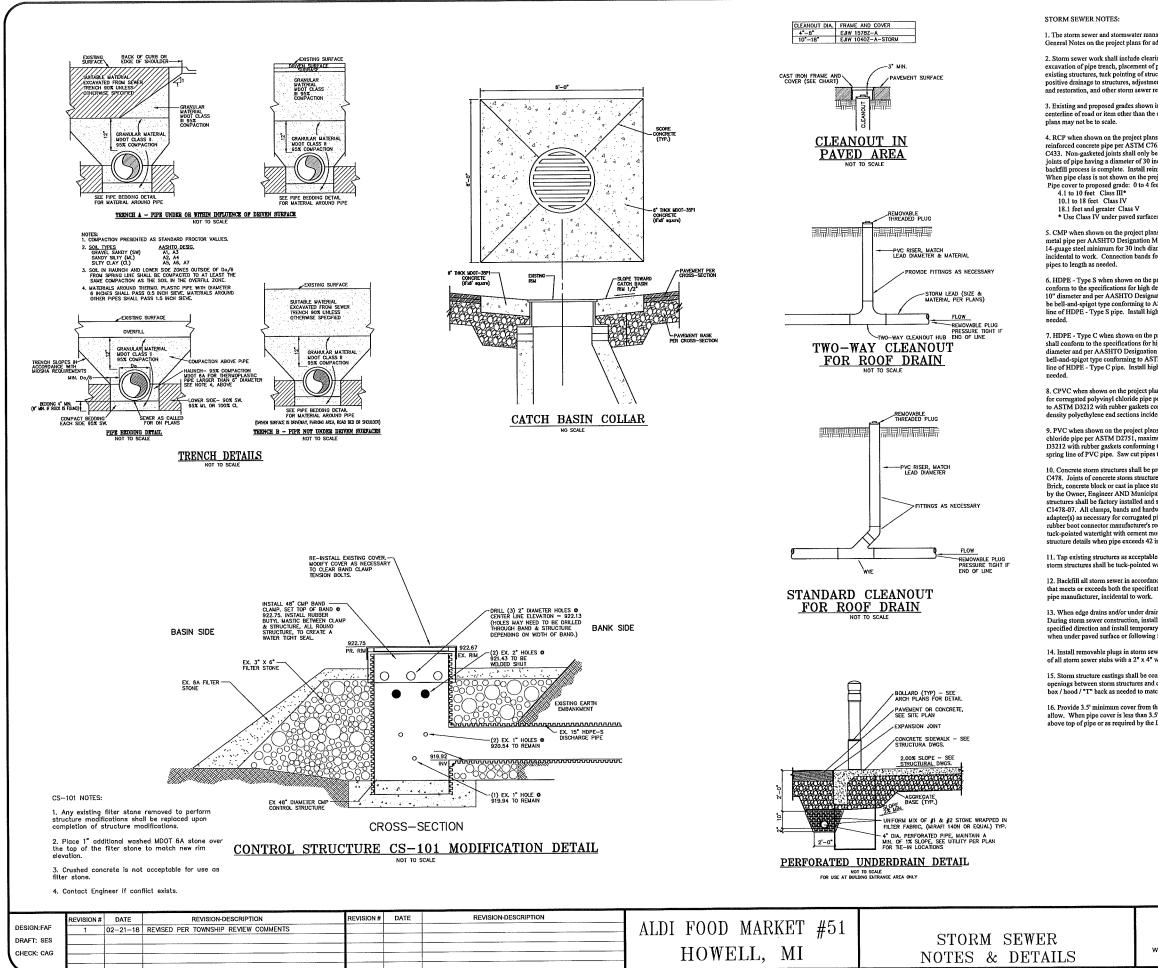
GENERAL NOTES GENERAL NOTES: 1. Contractor shall perform the work in accordance with the requirements of the appropriate Local, County and State Agencies and all other Government and Regulatory Agencies with jurisdiction ov the project. Contractor shall notify the appropriate Agencies in advance of each stage of work in accordance with each Agency's requirements. IF PAVEMENT IS CONCRETE; PROVIDE 1" ASPHALT FIBER ISOLATION JOINT " (TYP.) HEIGHT MAY VARY, SEE GRADING PLAN) 2. Contractor shall comply with all permit, insurance, licensing and inspection requirements associated with the work. Prior to construction, Contractor and Owner/Developer shall determine who is responsible for obtaining each required permit. Contractor shall verify that the each required permit has been obtained prior to commencement of the stage of work associated with the required permit(s). PAVEMEN 3. Contractor shall furnish liability insurance and property damage insurance to save harmless the Owner, Developer, Architect, Engineer, Surveyor and Government Agencies for any accident occurring during the construction period. Refer to the appropriate Local, County and State Agencies for additional requirements. Copies of insurance certifications shall be made available to the AGG. BASE SIDEWALK WITH INTERGAL CURB & 4. Contractor shall conduct and perform work in a safe and competent manner. Contractor shall perform all necessary measures to provide for traffic and pedestrian safety from the start of work and through substantial completion. Contractor shall determine procedures and provide safety equipment such as traffic controls, warning devices, temporty pavement markings and signs as needed. Contractor shall comply with the safety standards of the State Department of Labor, the occupational health standards of the State Department of Flash and a starty regulations of the sappropriate Local, Country, State and Federal Agencies. Refer to the safety specifications of the appropriate Local, Agencies. The Contractor shall designate a quilified employee with complete jobs site autority over the work and safety precautions; said designated employee shall be on site at all times during the work. 1/2" THICK EXPANSION JOINT WITH ASPHALT FIBER; HOLD DOWN 1/2"; SPACING AT 50" ON CENTER, UNLESS OTHERWISE NOTED 5. Contractor shall coordinate scheduling of all work in the proper sequence, including work by Subcontractors. Additional costs due to improper planning by Contractor or work done out of sequence as determined by standard acceptable construction practices, shall be Contractor's and the sequence as determined by standard acceptable construction practices, shall be Contractor's 6. Contractor shall contact the 811 Public Underground Utility Locating System or other appropriate local underground utility locating Agency, a minimum of three (3) working days prior to construction. Existing utility information on the project plans may be from information disclosed to this firm by the In Mixing unity intofination on the project pairs any or horizon atomation assesses. No guarantee is given as to the completeness or accuracy thereof. Prior to construction, locations and depths of all existing utilities (in possible conflict with the proposed improvements) shall be verified in the field. 7. Contractor shall coordinate scheduling a Pre-Construction Meeting with Engineer prior to 8. The Local Municipality, County and/or State in which the project is located may require an Engineer's Certification of construction of the proposed site improvements. Contractor shall verify the certification requirements with Engineer prior to commencement of work. Contractor shall coordinate construction staking, testing, documentation submittal and observation with the appropriate Agency, Surveyor and/or Engineer as required for Engineer's Certification and Government Agency. Acceptance, All materials used and work done shall meter or exceed the requirements of certification and acceptance, the contract documents and the material specifications noted on the project plans. Any materials used or work done that does not meet staid requirements, contract documents and/or specifications shall be replaced and/or Agency reviews prior to accepting work. SIDEWALK CROSS SECTION NOTES: The construction specifications of the Local Municipality are a part of this work. Refer to the General Notes and the Sidewalk Cross Section Details on the Project Plans for additional requirements. 2. Sidewalk widths may vary. See the Project Plans for the proposed sidewalk width at each locatio Increase sidewalks to 6" minimum thickness at driveways and other areas exposed to vehicular traffic 9. Engineer may provide subsurface soil evaluation results, if available, to Contra-9. Engineer may provide subsurace soil evaluation results, it available, to Contacted topol request. Subsurace soil evaluation results, soils maps and/or any other documentation does NOT guarance existing soil conditions or that sufficient, acceptable on-site granular material is available for use as structural fill, pipe bedding, pipe backfill, road ubbase or use as any other granular material specified on the project plans. On-site granular material that meets or exceeds the material specifications noted on the project plans may be used as structural fill, pipe bedding, pipe backfill and/or road subbase material. On-site granular material shall be stockpiled and tested as acceptable to the appropriate Agency and/or Engineer prior to use. 4. The sidewalk compacted subbase material shall be MDOT CL II sand. No subbase material substitution 10. During the performance of their work, Contractor shall be solely responsible for determining soil conditions and appropriate construction methods based on the actual field conditions. Contractor shall furnish, install and maintain sheeting, shoring, braving and/or other tools and equipment and/or construction techniques as needed for the safety and protection of the workers, pedestrians and vehicular traffic and for protection of adjacent structures and site improvements. 11. Contractor shall install temporary and permanent soil erosion and sedimentation control devices at the appropriate stages of construction in accordance with the appropriate regulatory Agencies. Refer to Soil Erosion and Sedimentation Control Plans and Notes on the project plans. concrete or taw cut within 8 hours 7. Install transverse expansion control joints in accordance with the Sidewalk Cross Section Detail. Space 7. Intellin transverse expansion control joints in accordance with the bia expansion control joints at 50 feet on center maximum. Transverse exp thick asphalt fiber joint filler matching entire sidewalk cross section. 12. Structural fill shall be placed as specified on the project plans and within the 1 on 1 influence zone of all structures, paved areas and other areas subject to vehicular traffic. Structural fill shall be placed using the controlled density method (12' maximum lifts, compared to 95% maximum unit weight, modified proctor). Fill material shall meet or exceed the specifications noted on the project plans or as on control joints shall be 1/2" 8. Provide 1" asphalt fiber control joint between concrete sidewalks and all other concrete structures, such as concrete building foundations, concrete curb and concrete driveways directed by Engineer when not specified on the project plans. 13. All existing monuments, property corners, ground control and benchmarks shall be protected and preserved; and if disturbed by Contractor, shall be restored at Contractor's expense. Contractor shall notify Surveyor 6 any conflicts between existing monuments, property comers, ground control and/or 9. Construct all Barrier Free Sidewalk Ramps in accordance with the American Disabilities Act and the chmarks and the proposed site improvement 10. The Concrete Pavement shall not be exposed to vehicular traffic until the concrete has reached at least 75 14. Contractor shall notify Owner/Developer and Engineer immediately upon encountering any field conditions, which are inconsistent with the project plans and/or specifications. % of the design flexural strength. 15. When noted on the project plans for demolition and/or removal, Contractor shall remove existing structures, building and debris and recycle and/or dispose of in accordance with Local, County, State 16. Contractor shall remove excess construction materials and debris from site and perform restoration in accordance with the project plans and specifications. Disposing of excess materials and debris shall be performed in accordance with Local, County, State and Pederal regulations. 17. Construction access to the site shall be located as acceptable to the Owner/Developer and to the appropriate Local, County and/or State Agency with jurisdiction over the total(s) providing access to the site. Construction access shall be maintained and cleaned in accordance with the appropriate Local, County and/or State Agencies and as directed by Owner/Developer and/or Engineer. 18. Contractor shall take necessary precautions to protect all site improvements from heavy and construction procedures. Damage resulting from Contractor actions shall be repaired a its from heavy equipmen Contractor's expense.

3. The existing subgrade soils shall be prepared prior to placement of the granular subbase. Unsuitable soils found within the 1 on 1 influence zone of the proposed sidewalk areas, such as muck, peat, topsoil, marl, silt or other unstable materials shall be excavated and replaced with structural fill. Structural fill shall be MDOT Class II granular material placed in accordance with the General Notes on the Project Plans. shall be permitted without prior written approval of the Project Engineer and receipt of the Owner's Authorization. The subbase shall be compacted to a minimum of 95% of the maximum unit weight, modified 5. Concrete material shall be MDOT P1 (I-A) 6.0 sack concrete pavement mixture with a minimum 28 day design compressive strength of 4,000 PS1 and 6.5% (4/-1.5%) entrained air. The Contractor shall submit the concrete mix design and aggregate mechanical analysis report to the Material Testing Engineer and/or Project Engineer for review and approval prior to use. 6. Install transverse contraction control joints in accordance with the Sidewalk Cross Section Detail. Space contraction control joints to match sidewalk width, but no greater than 10' on center. Tool joints in fresh , cummons an pairser reconservance stamps in accordance with the American Disabilities Act and the Barrier Free Design Requirements of the appropriate Local, County or State Agency with jurisdiction over the project. Refer to MDOT Standard Plan R-28, latest revision.

	<b>REVISION #</b>	DATE	REVISION-DESCRIPTION	REVISION #	DATE	REVISION-DESCRIPTION	ALDI BOOD MADVET 151
DESIGN:FAF							ALDI FOOD MARKET #51
DRAFT: SES							11
CHECK: CAG							HOWELL, MI







The storm sewer and stormwater management specifications of the Local Municipality are a part of this work. Refer to the General Notes on the project plans for additional information and requirements.

2. Storm sewer work shall include clearing of vegetation and tree stumps, stripping and stockpiling of topsoil for reuse, excavation of pipe trench, placement of pipe bedding, placement of pipe and structures including castings, connection to existing structures, tuck pointing of structures, backfill of pipe trench, compaction of backfill, finish grading to provide positive drainage to structures, adjustment of castings to match finish grade, topsoil placement, seed & mulch, site cleanup and restoration, and other storm sewer related work as shown on the project plans and specifications.

3. Existing and proposed grades shown in profile view, when provided on the project plans, may be in relation to the centerline of road or item other than the centerline of pipe. The pipe lengths and grades shown in profile view on the project

4. RCP when shown on the project plans shall be reinforced concrete pipe and shall conform to the specifications for reinforced concrete pipe per ASTM C76. RCP pipe joints shall be bell-and-spigot with rubber gaskets conforming to ASTM C433. Non-gasketed joints shall only be utilized when authorized by the Owner, Engineer AND Municipality. Non-gasketed joints of pipe having a diameter of 30 inches or greater shall be tuck-pointed on the inside with cement montar after the backfill process is complete. Install reinforced concrete end sections incidental to work. Saw cut pipes to length as needed. When pipe class is not shown on the project plans, provide the following: Pipe cover to proposed grade: 0 to 4 feet Class V 4.1 to 10 feet Class III\*

5. CMP when shown on the project plans shall be corrugated metal pipe and shall conform to the specifications for corrugated metal pipe per AASHTO Designation M36. CMP shall be 16-guage steel minimum for 24 inch diameter or smaller and 14-guage steel minimum for 30 inch diameter or greater. Install galvanized steel end sections and connection bands, incidental to work. Connection bands for CMP pipe joints located under paved surfaces shall be gasketed couplers. Saw cut

6. HDPE - Type S when shown on the project plans shall be high density polyethylene pipe with a smooth interior and shall conform to the specifications for high density polyethylene pipe per AASHTO Designation M252 Type S for pipes of 3" to 10" diameter and per AASHTO Designation M294 Type S for pipes of 12" to 60" diameter. HDPE - Type S pipe joints shall be bell-and-spigot type conforming to ASTM D3212 with tubber gaskets conforming to ASTM F477. Tamp backfill at spring line of HDPE - Type S pipe. Install high density polyethylene end sections incidental to work. Saw cut pipes to length as

7. HDPE - Type C when shown on the project plans shall be high density polyethylene pipe with a corrugated interior and shall conform to the specifications for high density polyethylene pipe per AASHTO Designation M252 for pipes of 3" to 10" diameter and per AASHTO Designation M294 for pipes of 12" to 60" diameter. HDPE - Type C pipe joints shall be bell-and-spipe type corporations to ASTM 477. Tamp backfill at spring line of HDPE - Type C pipe. Install high density polyethylene end sections incidental to work. Saw cut pipes to length as a contract.

8. CPVC when shown on the project plans shall be corrugated polyvinyl chloride pipe and shall conform to the specifications for corrugated polyvinyl chloride pipe per ASTM F794 and F949, CPVC pipe joints shall be bell-and-spigot type conforming to ASTM D3212 with rubber gaskets conforming to ASTM F477. Tamb backfill at spring line of CPVC pipe. Install high density polychylene end sections incidental to work. Saw cut pipes to length as needed.

9. PVC when shown on the project plans shall be polyvinyl chloride pipe and shall conform to the specifications for polyvinyl chloride pipe per ASTM D2751, maximum SDR of 26. PVC pipe joints shall be bell-and-spigot type conforming to ASTM D2212 with rubber gaskets conforming to ASTM P477 or solvent welded type conforming to ASTM D2564. Tamp backfill at the specification of the spe spring line of PVC pipe. Saw cut pipes to length as needed.

10. Concrete storm structures shall be pre-cast and shall conform to the specification of pre-cast concrete structures per ASTM C478. Joints of concrete storm structures sections shall be bell-and-spigot with rubber gaskets conforming to ASTM C433. Brick, concrete block or cast in place storm structures may be substituted for pre-cast storm structures ONL7 When authorized by the Owner, Engineer AND Manicipality; refer to MDOT standard plan R-1, latest revision. All pipe openings in pre-cast structures shall be factory installed and shall include a rubber boot resilient pipe to manhole connector conforming to ASTM C1478-07. All clamps, bhands and hardware shall be statuses statel or stirm concorrosive material. Provide the appropriate adapter(s) as necessary for corrugated pipe. Pipe to storm structure connections shall be performed in accordance with the rubber boot connector manufacturer's recommendations. All temporary openings and seams in storm structures shall be fuck-pointed watertight with cernent mortar. Refer to MDOT standard plan R-2, latest revision, for alternate on-line storm structure details when pipe exceeds 42 inch diameter.

11. Tap existing structures as acceptable to the Engineer and Municipality, incidental to work. All temporary openings in storm structures shall be tuck-pointed watertight with cement mortar.

12. Backfill all storm sewer in accordance with the Pipe Trench details provided on the project plans. Provide pipe bedding that meets or exceeds both the specifications of the Pipe Trench details on the project plans and the recommendation of the pipe manufacturer, incidental to work.

13. When edge drains and/or under drains are shown on the project plans, connection to storm structures is incidental to work During storm sewer construction, install first 10 linear feet of edge drain and/or under drain from the storm structures in each Dung shows own we want the provide the subgrade when not under paved surface.

14. Install removable plugs in storm sewer stubs as acceptable to Engineer and Municipality, incidental to work. Mark the end of all storm sewer stubs with a 2" x 4" wooden stake extending a minimum of 12" above finish grade, incidental to work.

15. Storm structure castings shall be coated with water based asphaltic paint by the manufacturer. Seams and temporary openings between storm structures and castings shall be tuck-pointed water tight with cement mortar. Coordinate correct curb box / hood / "T" back as needed to match curb profile. See easting schedule on project plans for additional requirements.

16. Provide 3.5' minimum cover from the top of pipe of all roof drain pipes to the proposed finish grade when site conditions allow. When pipe cover is less than 3.5', install 2" thick by 24" wide Styrofoam insulation centered over the top of pipe at 12" above top of pipe or as required by the Local Municipality.

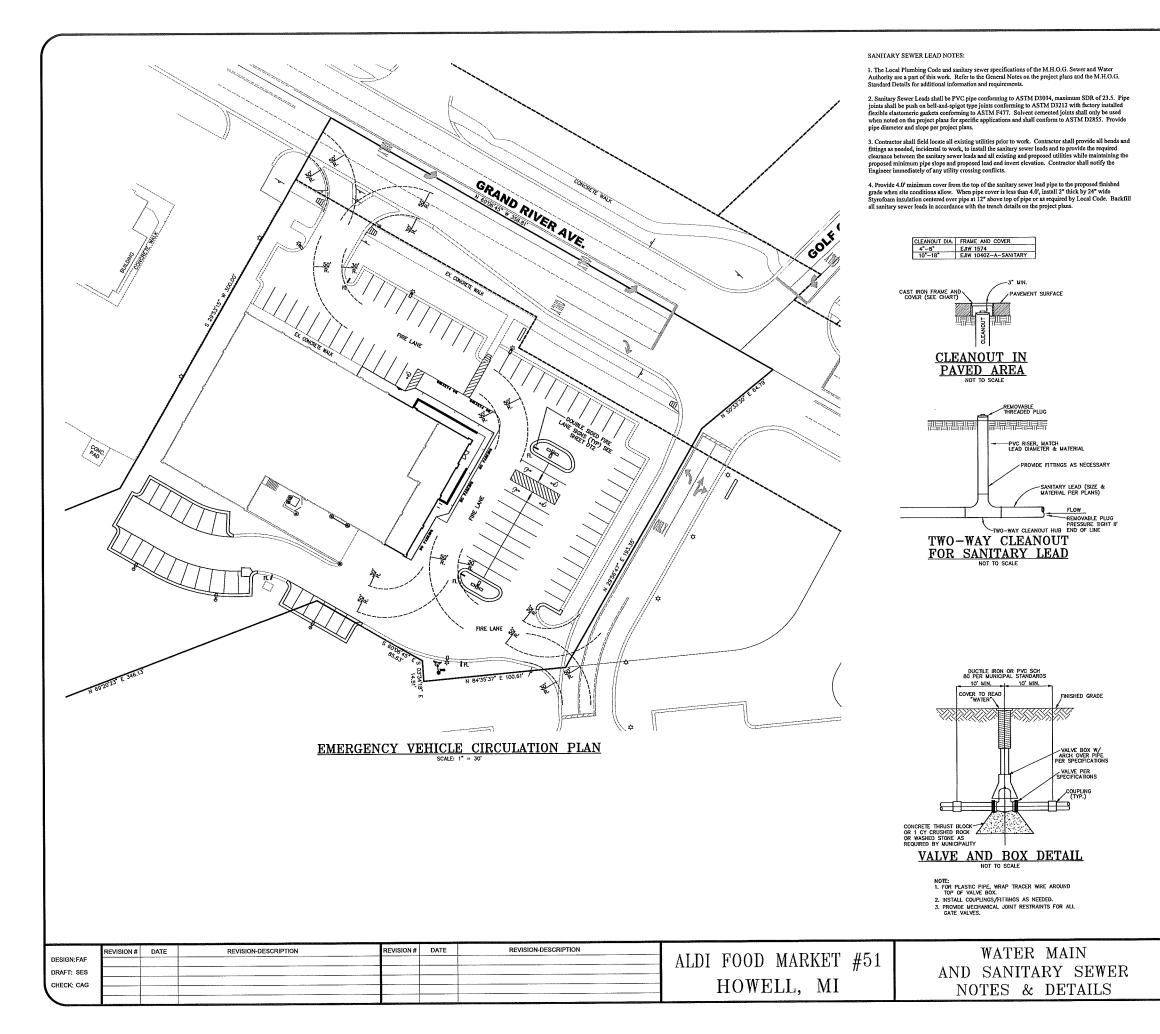


(810) 227-9533

CIVIL ENGINEERS

LAND SURVEYORS 2183 PLESS DRIVE

CLIENT:	SCALE: NO SCALE	
ALDI, Inc.	PROJECT No.: 9173290	DT3
2625 N. STOCKBRIDGE RD.	DWG NAME: 3290 DT	
WEBBERVILLE, MICHIGAN 48892 (517) 521-3907	ISSUED: FEB 21, 2018	



WATER MAIN NOTES:

 The water main specifications of the M.H.O.G. Sewer and Water Authority are a part of this work. Refer to the General Notes on the project plans and the M.H.O.G. Standard Details for additional information and requirement.

2. DIP shall be ductile iron pipe conforming to ANSI/AWWA C151/A21.51-02. DIP shall have a cement lining conforming to ANSI/AWWA C104/A21.4-95. DIP shall be class 52 unless noted otherwise on the project plans. Provide polyethylene wrap is accordance with M.H.O.G. standards and specifications.

3. Fittings shall be cast ductile iron Class 52, or Pressure Class 350 and shall conform to ANSI/AWWWA C153/A21.53-00 and shall have a cement lining conforming to ANSI/AWWA C104/A21.4-95.

4. Joints shall push-on type conforming to ANSU/AWWA C111/A21.11-00. Sealing gaskets, retainer glands and lubricants shall be in accordance with the pipe manufacturer's specifications.

5. Provide approved mechanical restraint systems at all bends of 11.25 degrees or greater, tees, crosses and hydrari shoes in accordance with the M.H.O.G. Standard Details. Thrust blocks shall only be utilized when authorized by M.H.O.G.

6. Water gate main valves shall be nomising stem, resilient-scated gate valves with box, conforming to AWWA C515-01, 250 psig aninimum working pressure rating with interior costing conforming to AWWA C550. All gate valves shall open counter clockwise [cfb].

7. Fire Hydrant Assemblies shall include all necessary piping and fittings for a complete assembly. Fire hydrants shall conform to AWWA C502. Fire hydrants shall be East Jordan Iron Works SBR-250 Watermaste with an integrated 5<sup>st</sup> Storz coupling or as otherwise specified by M.H.O.G. and/or the Brighton Area Fire Authority. Provide 5<sup>st</sup>.<sup>st</sup> minimum cover.

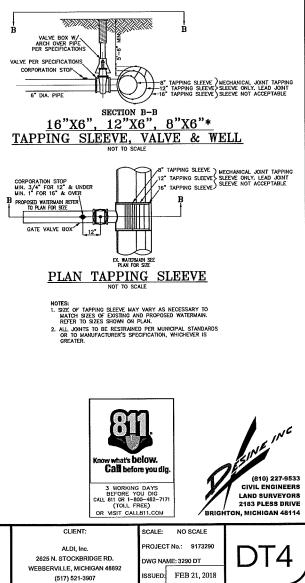
 Tapping Sleeve and Valve shall be meet the requirements of the M.H.O.G Sewer and Water Authority. See Tapping Sleeve and Valve detail for additional requirements.

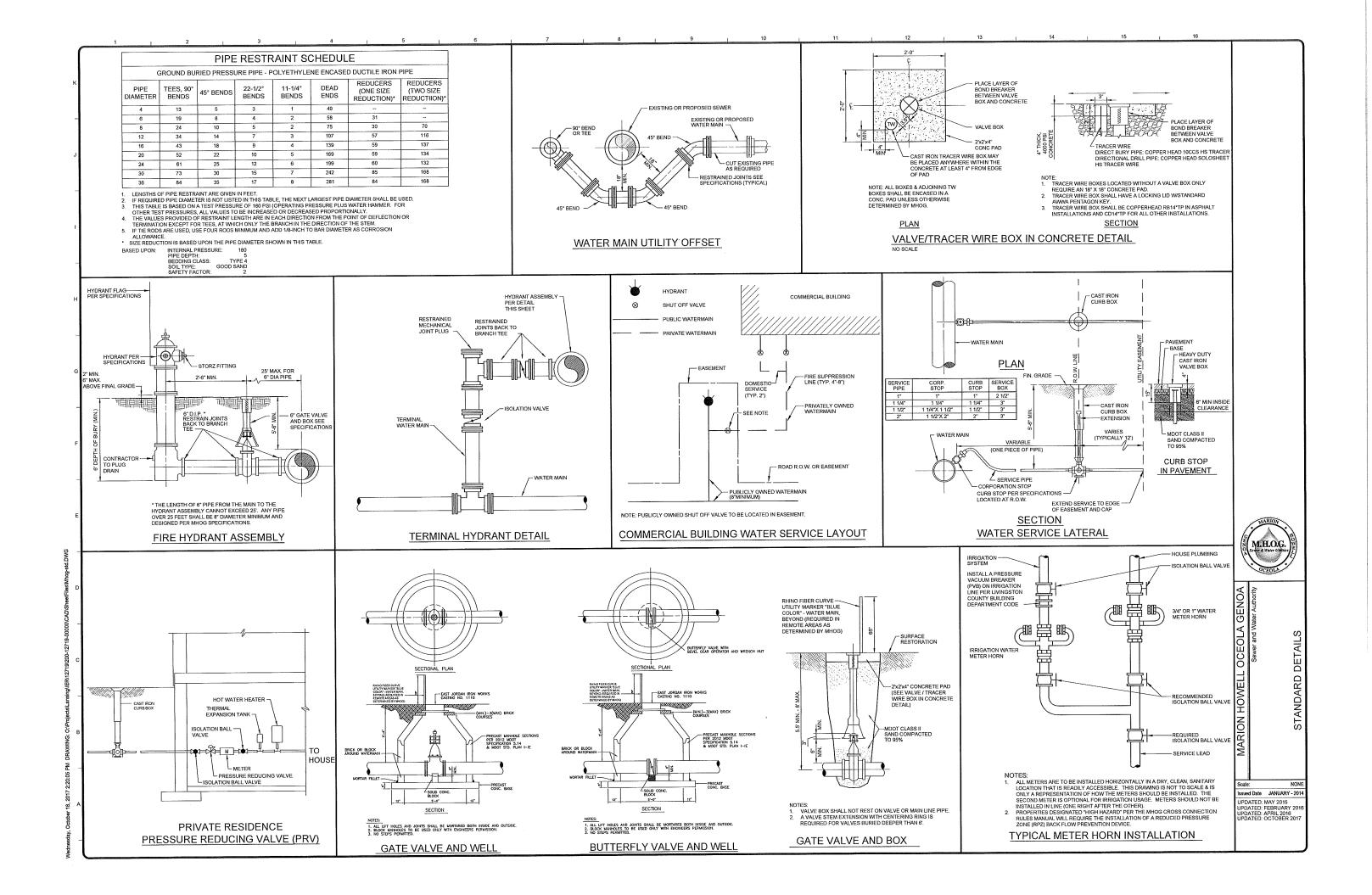
 Connect to existing water mains in accordance with the project plans and M.H.O.G. standards and specifications. Provide all materials and labor required for a complete watertight connection, incidental to work. Taps to existing water main shall be performed under pressure and without interruption of service.

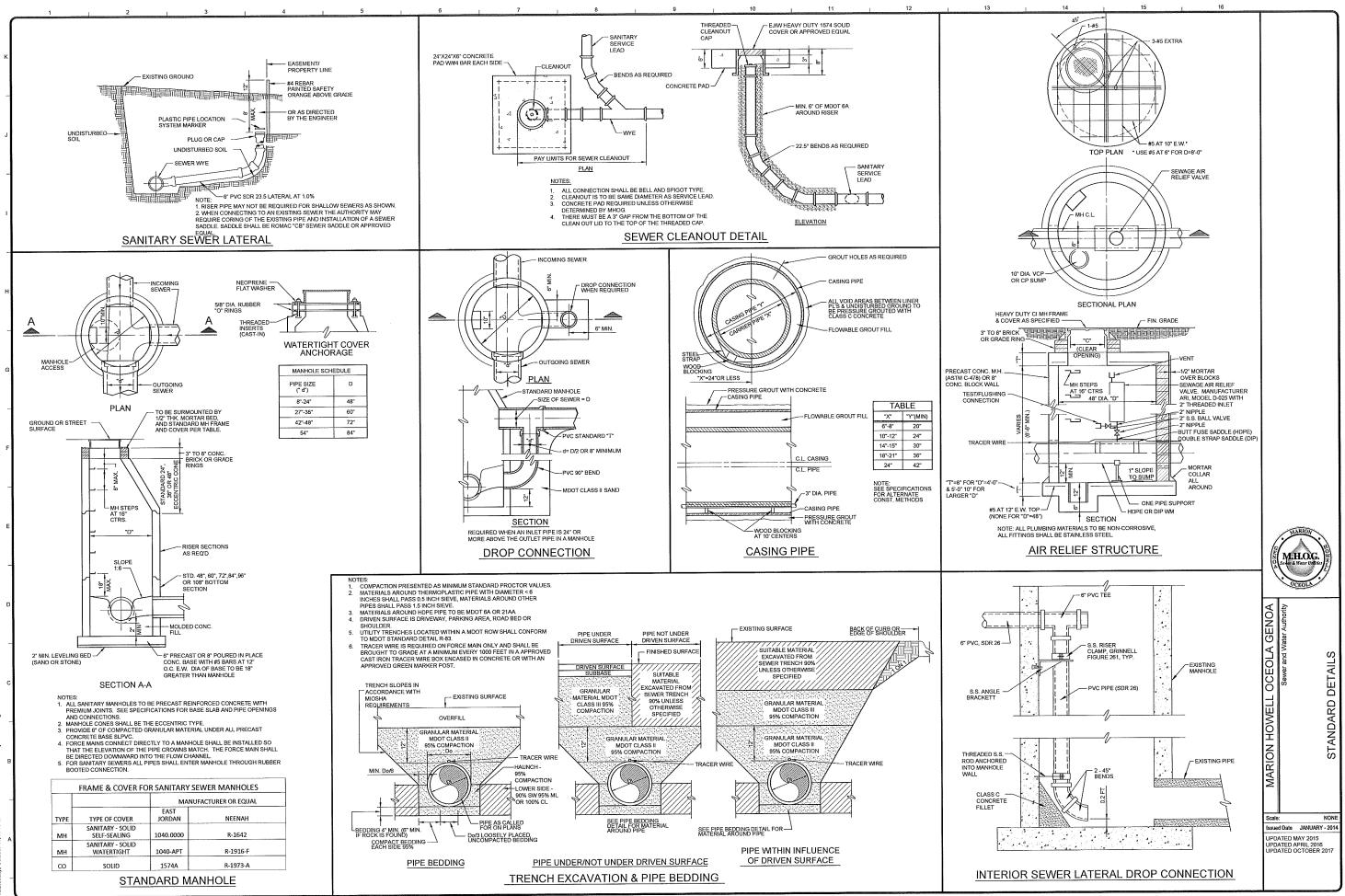
10. Backfill all water main in accordance with the Pipe Trench details provided on the project plans. Provide pipe bedding that meets or exceeds both the specifications of the Pipe Trench details on the project plans and the recommendation of the pipe manufacturer, incidental to work. Provide 5'-6" minimum cover for all water main.

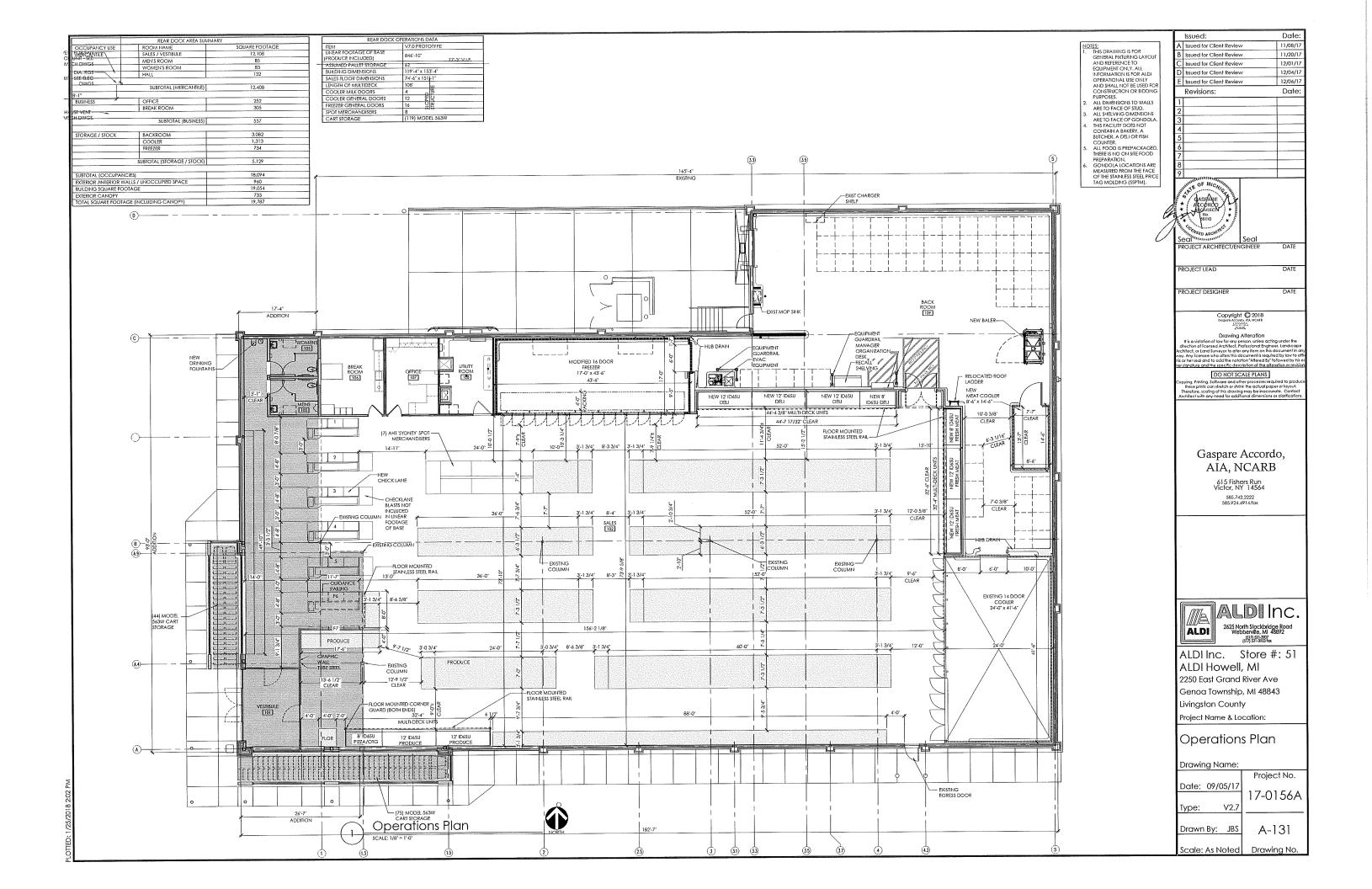
11. Provide 10' minimum horizontal separation and 1.5' minimum vertical separation between water main and both sanitary sewer and storm sewer.

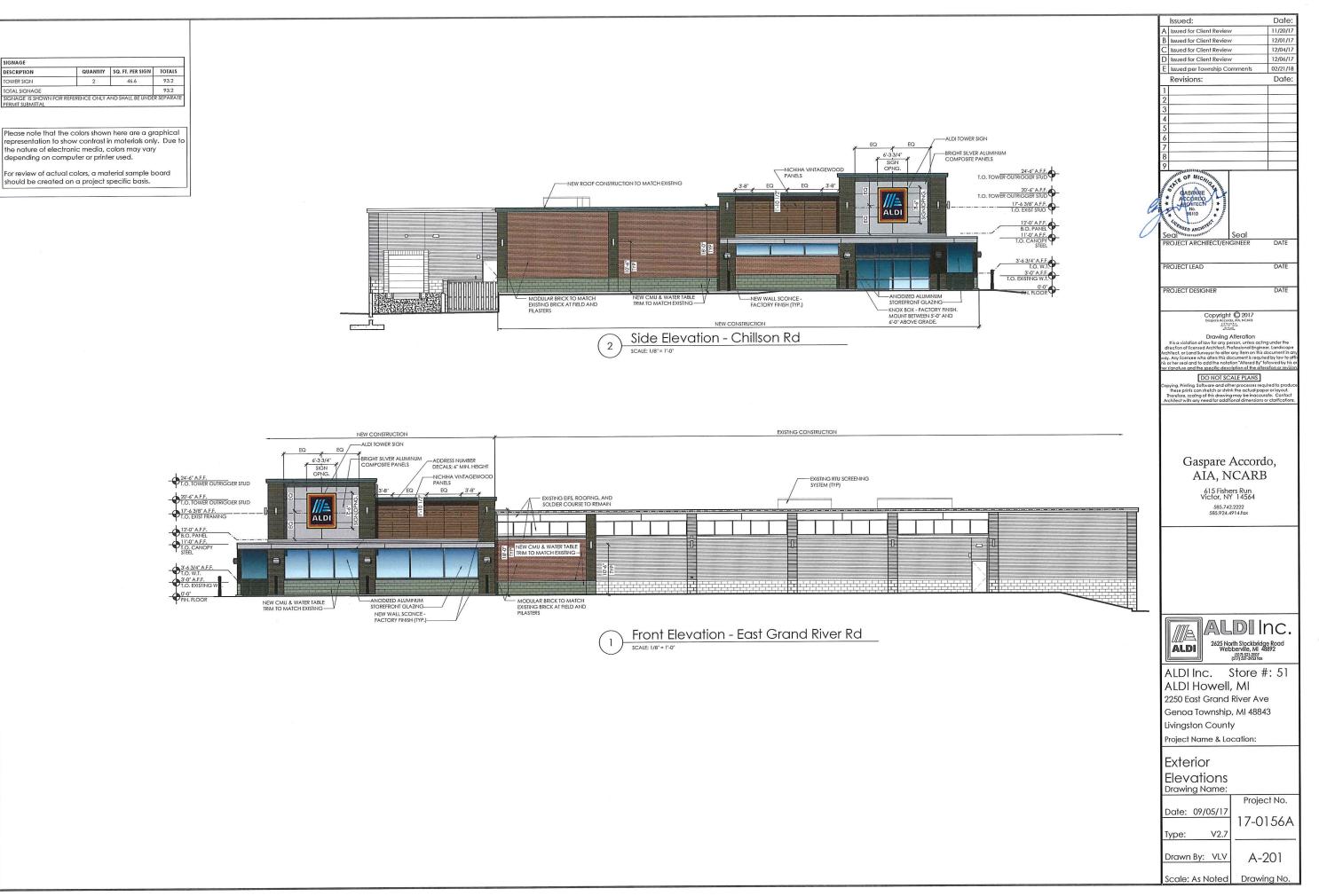
12. Contractor shall flush, test and chlorinate the water main in accordance with M.H.O.G. standards and specifications,











**GENOA TOWNSHIP** 

FEB 2 2 2018

RECEIVED



GENOA CHARTER TOWNSHIP

Application for Site Plan Review - LAKE SHORE

VILLAGE PH 3 Amendment

TO THE GENOA TOWNSHIP PLANNING COMMISSION AND TOWNSHIP BOARD: WALTER COPONEN ENT APPLICANT NAME & ADDRESS: COPONEN ARCHITECTS 8002 W. GRAND RIVER If applicant is not the owner, a letter of Authorization from Property Owner is needed. BRIGHTON, MI. 4BI/4 LAKESHORE VILLAGE L.T.D HOUSINGASS OCIATION OWNER'S NAME & ADDRESS: 27777 FRANKLIN RD. SUITE 1410 SOUTHFIELD, MI. 48034 LAKESHORE VILLAGE APARTMENTS **M**]. SITE ADDRESS: 2812 ONTARIO CT. HOWELL, PARCEL #(s): PHASE ILL. APPLICANT PHONE: (8/0) 225- 4/4 OWNER PHONE: ( ENT OWNER EMAIL: <u>coponenarchitects@sbcalobal.net</u> LOCATION AND BRIEF DESCRIPTION OF SITE: LAKESHARE VILLAGE APARTMENTS PHASE THREE CONSISTS OF 144 APARTMENTS ALONG WITH A BUSINESS/FITNESS CENTER. THE PROJECT WAS APPRIVED LAST OCTOBER AND IS CURRENTLY UNDER CONSTRUCTION. BRIEF STATEMENT OF PROPOSED USE: LAKECHORE VILLAGE APARTMENTS IS A RESIDENTIAL RENTAL COMMUNITY CONSISTING OF ONE. TWO. AND THREE BEDROOM APARTMENTS WITH CLUB HOUSE AND SWIMMING POOL THE FOLLOWING BUILDINGS ARE PROPOSED: THE PROPOSED AMENDED SITE PLAN WILL ADD (4) FOUR ONE BEDROOM APARTMEN UNITS AND MOVE THE BUSINESS FITNESS FENTER 30 FEFT TO ALLOW FOR FUTURE EXPANSION OF THE BUILDING, SEE LETTER I HEREBY CERTIFY THAT ALL INFORMATION AND DATA ATTACHED TO AND PART OF THIS APPLICATION IS TRUE AND ACCURATE TO THE BEST OF MY KNOWLEDGE AND BELIEF. Donen Architects, K.C. BY: ADDRESS:

<u>Contact Information</u> - Review Letters and Correspondence shall be forwarded to the following:

۰,

Name Business Affiliation coponen orchitects@sbcglobal.uet	1.) WALTER	COPONEN	of	COPONEN APCHITECTS	at
coponen prchitects@sbc.alobal.uet	Name			Business Affiliation	E-mail Address
			<u>co</u>	ponen architects@sbcg	lobal. net

i.

FEE EXCEEDANCE AGREEMENT				
As stated on the site plan review fee schedule, all site plans are allocated two (2) consultant reviews and one (1) Planning Commission meeting. If additional reviews or meetings are necessary, the applicant will be required to pay the actual incurred costs for the additional reviews. If applicable, additional review fee payment will be required concurrent with submittal to the Township Board. By signing below, applicant indicates agreement and full understanding of this policy.				
SIGNATURE: Watter F. Cippinen DATE: 2.26.18				
PRINT NAME: WALTER F. COPONEN PHONE: 810.225.4141				
PRINT NAME: WALTER F. COPONEN PHONE: 8/0.225.4/4/ ADDRESS: 8002. W. Grandriver Suite A. Brighton, Mi. 48114				



GENOA TOWNSHIP

FEB 2 2 2018 RECEIVED

February 15, 2018

Genoa Township 2911 Dorr Road Brighton, MI 48116

**Re: Lakeshore III Site Plan Approval** 

To Whom It May Concern:

Lakeshore Village Limited Dividend Housing Association Limited Partners is the owner for the construction of Lakeshore Village Apartments Phase III. This 144 unit project will be located on the east side Chilson Rd, south of Grand River in Genoa Twp, MI.

The purpose of this letter is to authorize Walt Coponen of Coponen Architects, P.C. to act as our agent to coordinate site plan approval with Genoa Township for changes made to the original site plan, which was approved in October 2016.

Should you need to contact me, I can be reached by telephone at 248-703-0145.

Sincerely,

ML

Mark Lockwood President and CEO

27777 Franklin Rd. Suite 1410 Southfield, MI 48034-2337 248.203.0991



March 7, 2018

Planning Commission Genoa Township 2911 Dorr Road Brighton, Michigan 48116

Attention:	Kelly Van Marter, AICP
	Planning Director and Assistant Township Manager
Subject:	Lakeshore Village Apartments – Site Plan Review #1
Location:	Chilson Road - east side of Chilson, between Grand River and C&O Railroad
Zoning:	HDR High Density Residential District

Dear Commissioners:

At the Township's request, we have reviewed the site plan (cover sheet dated 2/22/18) proposing an amendment to the previously approved plans for Phase 3 of the Lakeshore Village Apartments.

We have reviewed the proposal in accordance with the applicable provisions of the Genoa Township Zoning Ordinance.

#### A. Summary

- 1. We request the applicant confirm that no changes from the originally approved plan are proposed to:
  - building design and materials;
  - carport structures;
  - vehicular circulation;
  - waste receptacle enclosures;
  - site lighting; and
  - entry signage.
- 2. The Commission may wish to consider the appropriateness of adding a sidewalk along the south side of the main drive connection to Chilson.
- 3. The Commission may wish to require that the applicant submit the missing landscape sheet for a complete review.

#### B. Proposal

The applicant proposes an amendment to Phase 3 of their development with the inclusion of 4 additional units (each 1-bedroom) and relocation of the business/fitness center.

Multiple family dwellings are permitted by right in the HDR; however, since the proposed amendment does not constitute a "minor" change (Section 18.10), site plan review/approval by the Planning Commission is required.

Genoa Township Planning Commission Lakeshore Village Apartments Site Plan Review #1 Page 2



Aerial view of site and surroundings (looking north)

#### C. Site Plan Review

1. General Comments. Based on the cover letter and application form included with the submittal, the proposal entails only an expansion of Building 22 (to accommodate 4 additional one-bedroom units) and relocation of the business/fitness center building by approximately 30 feet.

Accordingly, the amended site plan includes only a limited amount of detail. In order to ensure nothing is missed, we request the applicant confirm that no changes are proposed to:

- building design and materials;
- carport structures;
- vehicular circulation;
- waste receptacle enclosures;
- site lighting; and
- entry signage.
- 2. Dimensional Requirements. The only dimensional requirements impacted by the request are a slight increase in density and lot coverage.

More specifically, the inclusion of 4 additional units results in a density to 5.3 units/acre, which is well within that allowed (8 units/acre).

Additionally, the resulting building coverage (7.5%) and impervious coverage (23.3%) are well within the maximum allowed (35% and 50%, respectively).

**3. Parking.** Per Section 14.04, a total of 295 spaces are required (286 for the dwelling units and 9 for the business/fitness center). The proposed plan provides a total of 334 spaces, including 20 barrier free spaces.

Proposed parking spaces and drive aisles meet or exceed the minimum standards of Section 14.06 and the use of looped striping is proposed.

**4. Pedestrian Circulation.** The plan includes 5 to 7-foot wide sidewalks throughout the development with pedestrian connections between buildings, parking areas, the mailbox cluster and waste receptacles. Crosswalk striping is also provided at appropriate locations.

The Commission may wish to consider the appropriateness of adding a sidewalk along the south side of the main drive connection to Chilson. This would provide better pedestrian access for those units in the southerly portion of the site to the business/fitness center.

**5.** Landscaping. We are unable to conduct a complete review of the landscape plan due to the fact that Sheet LS-103 is not provided.

However, based on the plant list (Sheet LS-101), the type, quantity and size of plantings proposed meets Ordinance requirements for greenbelt, detention pond, parking lot and buffer zone landscaping.

If deemed necessary by the Commission, the applicant must provide the missing landscape sheet for review.

Should you have any questions concerning this matter, please do not hesitate to contact our office. I can be reached by phone at (248) 586-0505, or via e-mail at <u>borden@lslplanning.com</u>.

Respectfully, LSL PLANNING, A SAFEBUILT COMPANY

Brian V. Borden, AICP Planning Manager



March 7, 2018

Ms. Kelly Van Marter Genoa Township 2911 Dorr Road Brighton, MI 48116

#### Re: Lakeshore Village Apartments Phase 3 Amendment Plan Review #1

Dear Ms. Van Marter:

Tetra Tech conducted a site plan review of the Lakeshore Village Apartments Phase 3 amended Boss Engineering construction plans submitted by Walter Coponen of Coponen Architects. The developer has added 4 single-unit apartments to one of their buildings to address greater than expected demand for that type of unit, in addition to moving the physical location of one of the buildings on site approximately 30 feet east. Included in the submission were the site plan review application, a letter from the architect explaining the changes, and a revised impact assessment. We offer the following comments:

• The increase in the overall number of units will have an impact on the property's utility usage estimates used to compute the connection fees. The Township should review if the connection fee should be revised.

The petitioner should contact the Township to confirm their usage and connection fees are correct prior to approval. We have no further engineering related concerns. Please call or email if you have any questions.

Sincerely,

Gary J. Markstrom, P.E. Unit Vice President

copy: Walter Coponen, Coponen Architects

Joseph C Siwek Project Engineer

**BRIGHTON AREA FIRE AUTHORITY** 



615 W. Grand River Ave. Brighton, MI 48116 o: 810-229-6640 f: 810-229-1619

March 6, 2018

Kelly VanMarter Genoa Township 2911 Dorr Road Brighton, MI 48116

RE: Lakeshore Village Phase 3 2812 Ontario Court Howell (Genoa Twp.), MI 48843

Dear Kelly:

The Brighton Area Fire Department has reviewed the above mentioned site plan. The plans were received for review on February 27, 2018 and the drawings are dated February 3, 2016 with latest revisions dated February 22, 2018. The submittal is for the proposed relocation of the business center and Building 22 to accommodate additional 1-bedroom units. The project is already under construction and this is a request to modify the previous approved drawings.

The fire authority has no objection to the proposed relocation or additions as there is little to no effect on the site fire protection features and access.

If you have any questions about the comments on this plan review please contact me at 810-229-6640.

Cordially,

Rick Boisvert, CFPS Fire Marshal

# LEGAL DESCRIPTIONS

LAND IN THE TOWNSHIP OF GENOA, LIVINGSTON COUNTY, MICHIGAN, DESCRIBED AS FOLLOWS:

PARCEL 2-B:

PART OF THE SOUTHEAST ¼ OF SECTION 6. TOWN 2 NORTH. RANGE 5 EAST, GENOA TOWNSHIP, LIVINGSTON COUNTY, MICHIGAN, MOR PARTICULARLY DESCRIBED AS FOLLOWS: COMMENCING AT THE EAST 1/2 CORNER OF SAID SECTION 6: OAKS WEST INDUSTRIAL PARK " A SUBDIVISION AS RECORDED IN LIBER 30 OF PLATS ON PAGES 1-5 OF LIVINGSTON COUNTY RECORDS 330.00 FEET; THENCE ALONG THE WEST ORTHERLY RIGHT-OF-WAY LINE OF THE C&O RAILROAD, NORTHWESTERLY ON AN ARC LEFT, HAVING A 28.34.50 FFFT. A CENTRAL ANGLE OF 18 DEGREES 0.3 MINUTES 31 SECONDS AND A LONG CHORD WHICH BEARS NORTH CONDS WEST. 889.69 FEFT: THENCE ALONG THE CENTERLINE OF CHILSON ROAD ( NORTH 18 DEGREES 44 MINUTES 40 SECONDS WEST, 809.27 FEET; THENCE NORTH 89 DEGREES 00 MINUTES 29 SECONDS EAST, 487.21 FEE THENCE NORTH 01 DEGREE 06 MINUTES 44 SECONDS WEST, 408.86 FEET; THENCE SOUTH 82 DEGREES 31 MINUTES 05 SECONDS EAST, 797.4 FEET, TO THE POINT OF BEGINNING. CONTAINING 27.80 ACRES

TAX ITEM NO,: 11-06-400-015-201-47070

## CONSTRUCTION NOTES

WITH THE FOLLOWING NOTES AND ANY WORK INVOLVED SHALL BE CONSIDERED INCIDENTAL TO THE CONTRACT

- 1. THE CONTRACTOR SHALL HOLD HARMLESS THE DESIGN PROFESSIONAL, MUNICIPALITY, COUNTY, STATE AND ALL OF ITS SUB CONSULTANTS, PUBLIC AND PRIVATE UTILITY COMPANIES AND LANDOWNERS FOR DAMAGES TO INDIVIDUALS AND PROPERTY REAL OR OTHERWISE, DUE TO THE OPERATIONS OF THE CONTRACTOR AND/OF THEIR SUBCONTRACTOR
- 2. DO NOT SCALE THESE DRAWINGS AS IT IS A REPRODUCTION AND SUBJECT TO DISTORTION
- 3. A GRADING PERMIT FOR SOIL EROSION-SEDIMENTATION CONTROL SHALL BE OBTAINED FROM THE GOVERNING AGENCY PRIOR TO THE START OF CONSTRUCTION
- IF DUST PROBLEM OCCURS DURING CONSTRUCTION, CONTROL WILL BE PROVIDED BY AN APPLICATION OF WATER, EITHER BY SPRINKLER OR TANK TRUCK.
- 5. ALL CONSTRUCTION AND MATERIALS SHALL BE IN ACCORDANCE WITH LOCAL MUNICIPAL STANDARDS AND SPECIFICATIONS.
- 6. THE CONTRACTOR IS RESPONSIBLE FOR OBTAINING ALL REQUIRED TOWNSHIP, COUNTY, AND STATE OF MICHIGAN PERMITS. 7. PAVED SURFACES, WALKWAYS, SIGNS, LIGHTING AND OTHER STRUCTURES SHALL BE MAINTAINED IN A SAFE, ATTRACTIVE CONDITION AS ORIGINALLY DESIGNED AND CONSTRUCTED.
- 8. ALL BARRIER-FREE FEATURES SHALL BE CONSTRUCTED TO MEET ALL LOCAL, STATE AND A.D.A. REQUIREMENTS.
- 9. ANY DISCREPANCY IN THIS PLAN AND ACTUAL FIELD CONDITIONS SHALL BE REPORTED TO THE DESIGN ENGINEER PRIOR TO THE START OF CONSTRUCTION. CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFICATION OF ALL SETBACKS, EASEMENTS AND DIMENSIONS SHOWN HEREON BEFORE BEGINNING CONSTRUCTION
- 10. THE CONTRACTOR SHALL CONTACT ALL OWNERS OF EASEMENTS, UTILITIES AND RIGHTS-OF-WAY, PUBLIC OR PRIVATE, PRIOR TO THE START OF CONSTRUCTION 11. THE CONTRACTOR SHALL COORDINATE WITH ALL OWNERS TO DETERMINE THE LOCATION OF EXISTING LANDSCAPING, IRRIGATION LINES & PRIVATE UTILITY LINES. THE CONTRACTOR IS RESPONSIBLE FOR ANY DAMAGE TO EXISTING LANDSCAPING, IRRIGATION LINES, AND PRIVATE UTILITY LINES.
- 12. THE CONTRACTOR SHALL REMOVE ALL TRASH AND DEBRIS FROM THE SITE UPON COMPLETION OF THE PROJECT.
- 13. THE CONTRACTOR SHALL MAINTAIN THE SITE IN A MANNER SO THAT WORKMEN AND PUBLIC SHALL BE PROTECTED FROM INJURY, AND ADJOINING PROPERTY PROTECTED FROM DAMAGE
- 14. THE CONTRACTOR SHALL KEEP THE AREA OUTSIDE THE "CONSTRUCTION LIMITS" BROOM CLEAN AT ALL TIMES.
- 15. THE CONTRACTOR SHALL CALL MISS DIG A MINIMUM OF 72 HOURS PRIOR TO THE START OF CONSTRUCTION.
- 16. ALL EXCAVATION UNDER OR WITHIN 3 FEET OF PUBLIC PAVEMENT, EXISTING OR PROPOSED SHALL BE BACKFILLED AND COMPACTED WITH SAND (MDOT CLASS II). 17. ALL PAVEMENT REPLACEMENT AND OTHER WORKS COVERED BY THESE PLANS SHALL BE DONE IN ACCORDANCE WITH THE REQUIREMENTS OF THE TOWNSHIP,

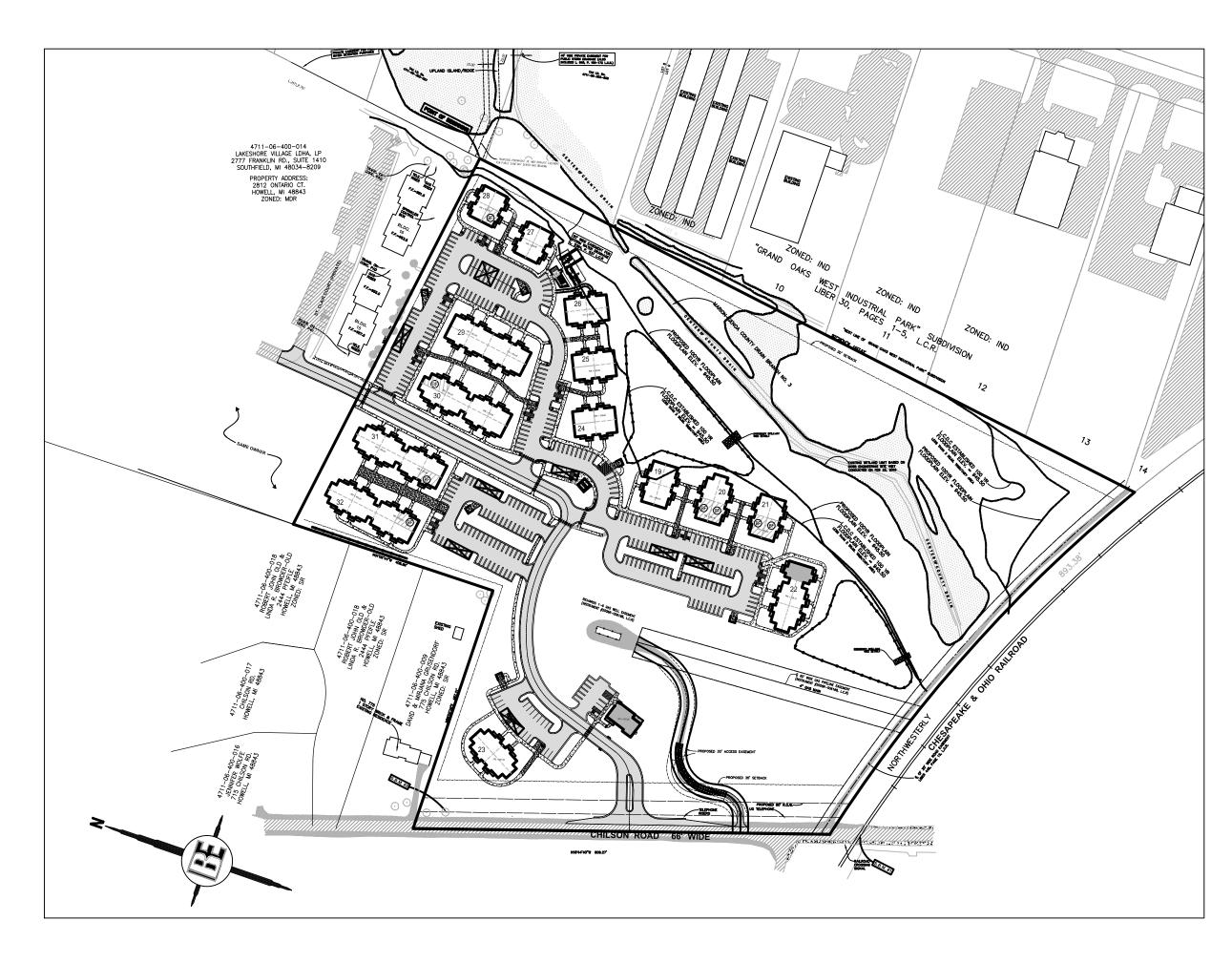
INCLUDING THE LATEST MICHIGAN DEPARTMENT OF TRANSPORTATION (MDOT) SPECIFICATIONS FOR HIGHWAY CONSTRUCTION.

18. THE CONTRACTOR IS RESPONSIBLE FOR ALL DAMAGE TO EXISTING UTILITIES.

- 19. NO ADDITIONAL COMPENSATION WILL BE PAID TO THE CONTRACTOR FOR ANY DELAY OR INCONVENIENCE DUE TO THE MATERIAL SHORTAGES OR RESPONSIBLE DELAYS DUE TO THE OPERATIONS OF SUCH OTHER PARTIES DOING WORK INDICATED OR SHOWN ON THE PLANS OR IN THE SPECIFICATION OR FOR ANY REASONABLE DELAYS IN CONSTRUCTION DUE TO THE ENCOUNTERING OR EXISTING UTILITIES THAT MAY OR MAY NOT BE SHOWN ON THE PLANS.
- 20. DURING THE CONSTRUCTION OPERATIONS, THE CONTRACTOR SHALL NOT PERFORM WORK BY PRIVATE AGREEMENT WITH PROPERTY OWNERS ADJACENT TO THE PROJECT.
- 21. IF WORK EXTENDS BEYOND NOVEMBER 15, NO COMPENSATION WILL BE DUE TO THE CONTRACTOR FOR ANY WINTER PROTECTION MEASURES THAT MAY BE REQUIRED BY THE ENGINEER.
- 22. NO TREES ARE TO BE REMOVED UNTIL MARKED IN THE FIELD BY THE ENGINEER.
- 23. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DAMAGE TO THE PROPERTY BEYOND THE CONSTRUCTION LIMITS INCLUDING BUT NOT LIMITED TO EXISTING FENCE, LAWN, TREES AND SHRUBBERY
- 24. ALL AREAS DISTURBED BY THE CONTRACTOR BEYOND THE NORMAL CONSTRUCTION LIMITS OF THE PROJECT SHALL BE SODDED OR SEEDED AS SPECIFIED OR DIRECTED BY THE ENGINEER.
- 25. ALL ROOTS, STUMPS AND OTHER OBJECTIONABLE MATERIALS SHALL BE REMOVED AND THE HOLE BACKFILLED WITH SUITABLE MATERIAL. WHERE GRADE CORRECTION I S REQUIRED, THE SUBGRADE SHALL BE CUT TO CONFORM TO THE CROSS-SECTION AS SHOWN IN THE PLANS.
- 26. TRAFFIC SHALL BE MAINTAINED DURING CONSTRUCTION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING ALL SIGNS AND TRAFFIC CONTROL DEVICES. FLAG PERSONS SHALL BE PROVIDED BY THE CONTRACTOR IF DETERMINED NECESSARY BY THE ENGINEER. ALL SIGNS SHALL CONFORM TO THE MICHIGAN MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES AT NO COST TO THE TOWNSHIP. NO WORK SHALL BE DONE UNLESS THE APPROPRIATE TRAFFIC CONTROL DEVICES ARE IN PLACE.
- 27. ALL DEMOLISHED MATERIALS AND SOIL SPOILS SHALL BE REMOVED FROM THE SITE AT NO ADDITIONAL COST, AND DISPOSED OF IN ACCORDANCE WITH LOCAL, STATE AND FEDERAL REGULATIONS.
- 28. AFTER REMOVAL OF TOPSOIL, THE SUBGRADE SHALL BE COMPACTED TO 95% OF ITS UNIT WEIGHT. 29. ALL GRADING IN THE PLANS SHALL BE DONE AS PART OF THIS CONTRACT. ALL DELETERIOUS MATERIAL SHALL BE REMOVED FROM THE SUBGRADE PRIOR TO COMPACTING.
- 30. NO SEEDING SHALL BE DONE AFTER OCTOBER 15 WITHOUT APPROVAL OF THE ENGINEER.
- 31. ANY EXISTING APPURTENANCES SUCH AS MANHOLES, GATE VALVES, ETC. SHALL BE ADJUSTED TO THE PROPOSED GRADE AND SHALL BE CONSIDERED INCIDENTAL TO THE CONTRACT.
- 32. SOIL EROSION MEASURES SHALL BE MAINTAINED BY THE CONTRACTOR UNTIL VEGETATION HAS BEEN RE-ESTABLISHED.
- 33. ALL PERMANENT SIGNS AND PAVEMENT MARKINGS SHALL BE INSTALLED IN ACCORDANCE WITH THE LATEST REVISION OF THE MICHIGAN MUTCD MANUAL AND SHALL BE INCIDENTAL TO THE CONTRACT.
- 34. THE EXISTING AND PROPOSED ONSITE DRAINAGE SYSTEMS ARE TO BE OWNED AND PROPERLY MAINTAINED BY THE PROPERTY OWNER.

# AMENDED SITE PLAN FOR LAKESHORE VILLAGE APARTMENTS PHASE 3

# PART OF SE 1/4, SECTION 8, T2N R5E GENOA TOWNSHIP, LIVINGSTON COUNTY, MI



**OVERALL SITE MAP** NO SCALE

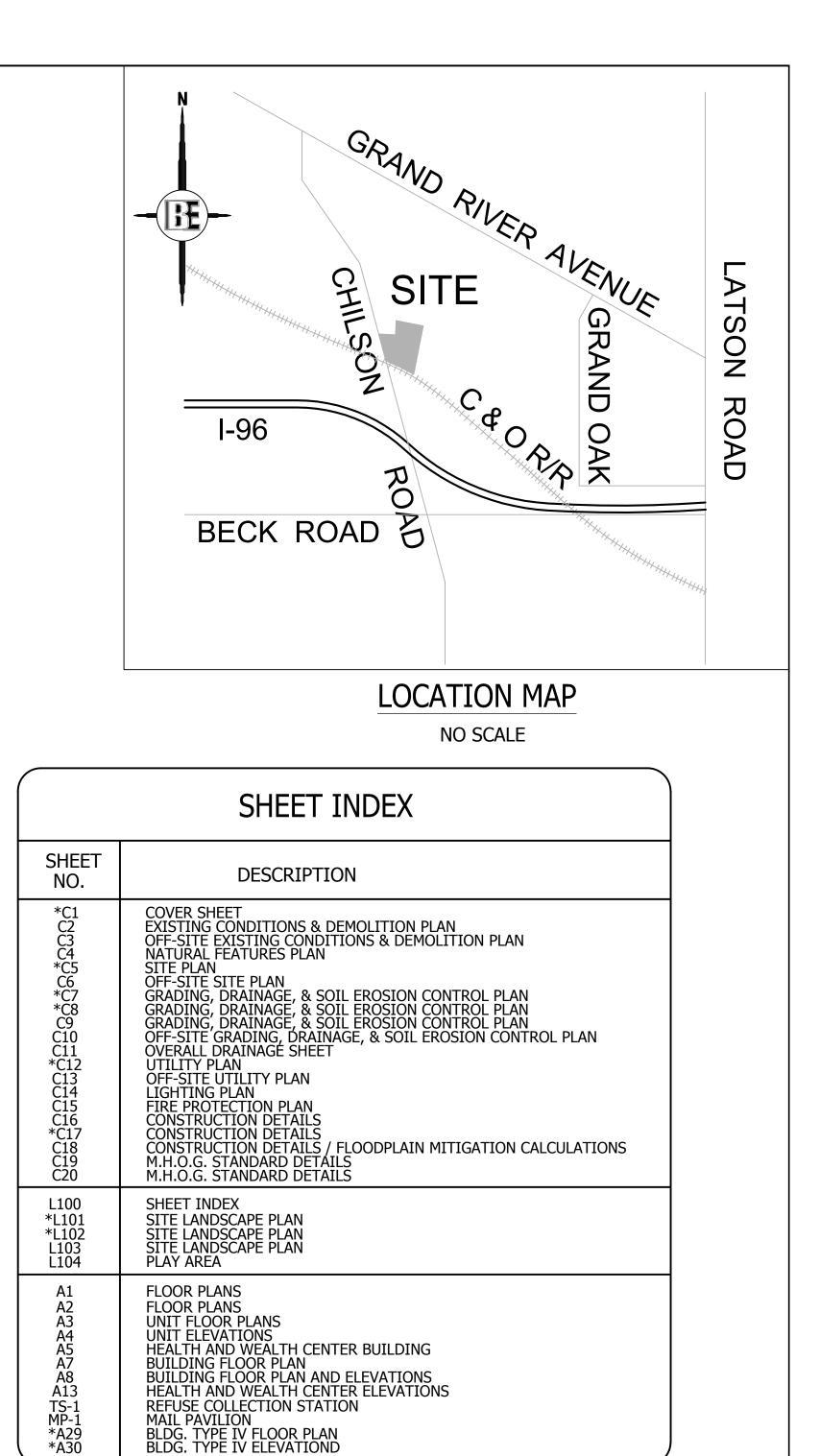
# **PROJECT TEAM:**

COPONEN ARCHITECTS 8002 W. GRAND RIVER, SUITE A BRIGHTON, MI 48114 **CONTACT: WALTER COPONEN** PHONE: 810.225.4141

KENNETH WEIKAL LANDSCAPE ARHITECTURE 33203 BIDDESTONE LANE FARMINGTON HILLS, MI 48334 CONTACT: KEN WEIKAL PHONE: 248.477.3600

# INDEMNIFICATION STATEMENT

THE CONTRACTOR SHALL HOLD HARMLESS THE DESIGN PROFESSIONAL, MUNICIPALITY, COUNTY, STATE AND ALL OF ITS SUB CONSULTANTS, PUBLIC AND PRIVATE UTILITY COMPANIES, AND LANDOWNERS FOR DAMAGES TO INDIVIDUALS AND PROPERTY, REAL OR OTHERWISE, DUE TO THE OPERATIONS OF THE CONTRACTOR AND/OR THEIR SUBCONTRACTORS.

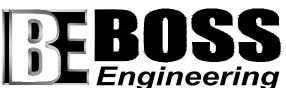


# LAKESHORE VILLAGE APARTMENTS PHASE 3

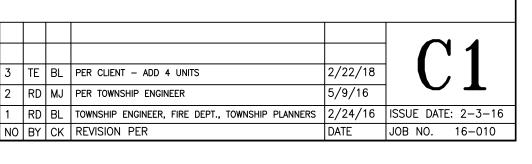
**PREPARED FOR:** 

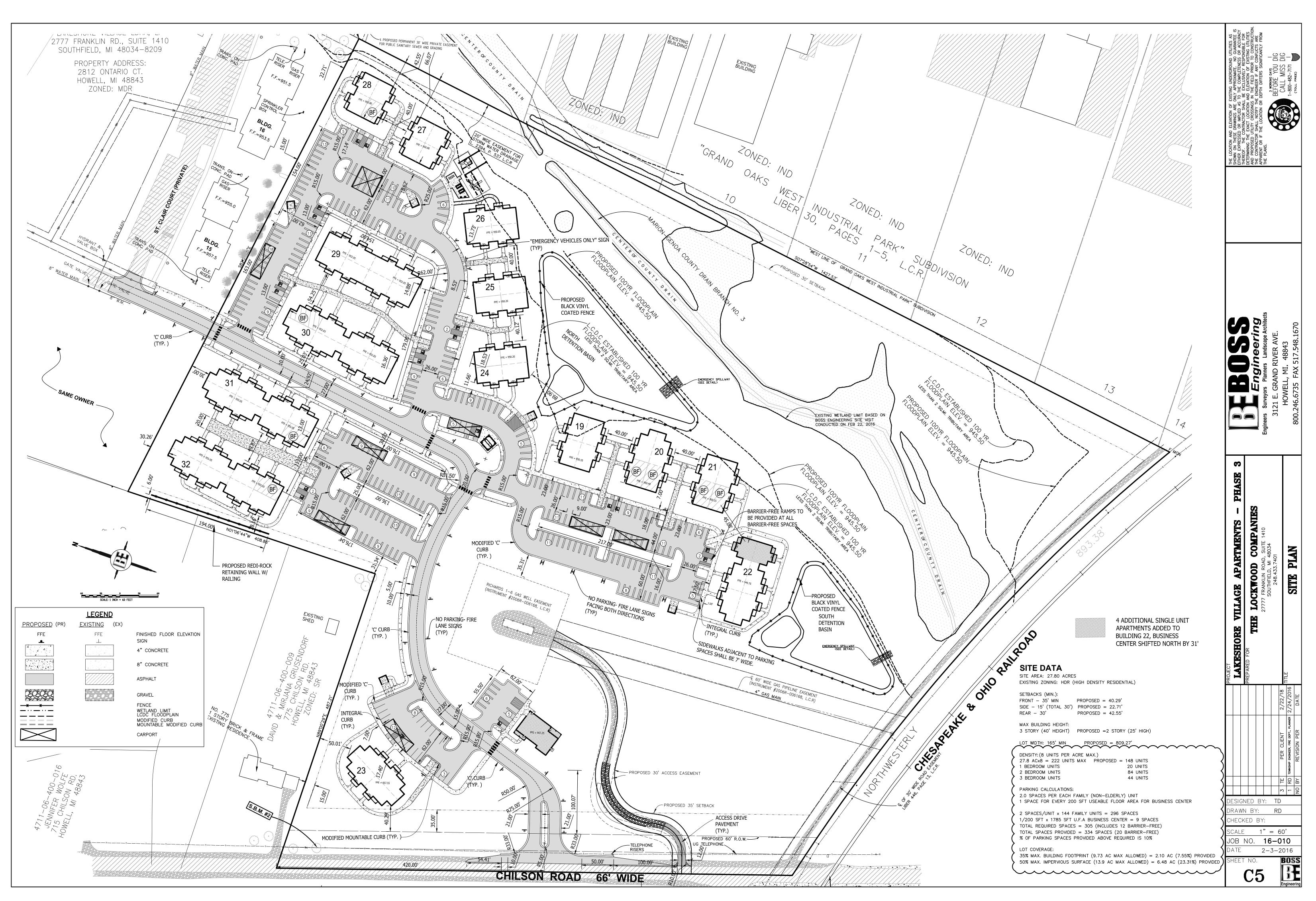
BLDG. TYPE IV FLOOR PLAN BLDG. TYPE IV ELEVATIOND

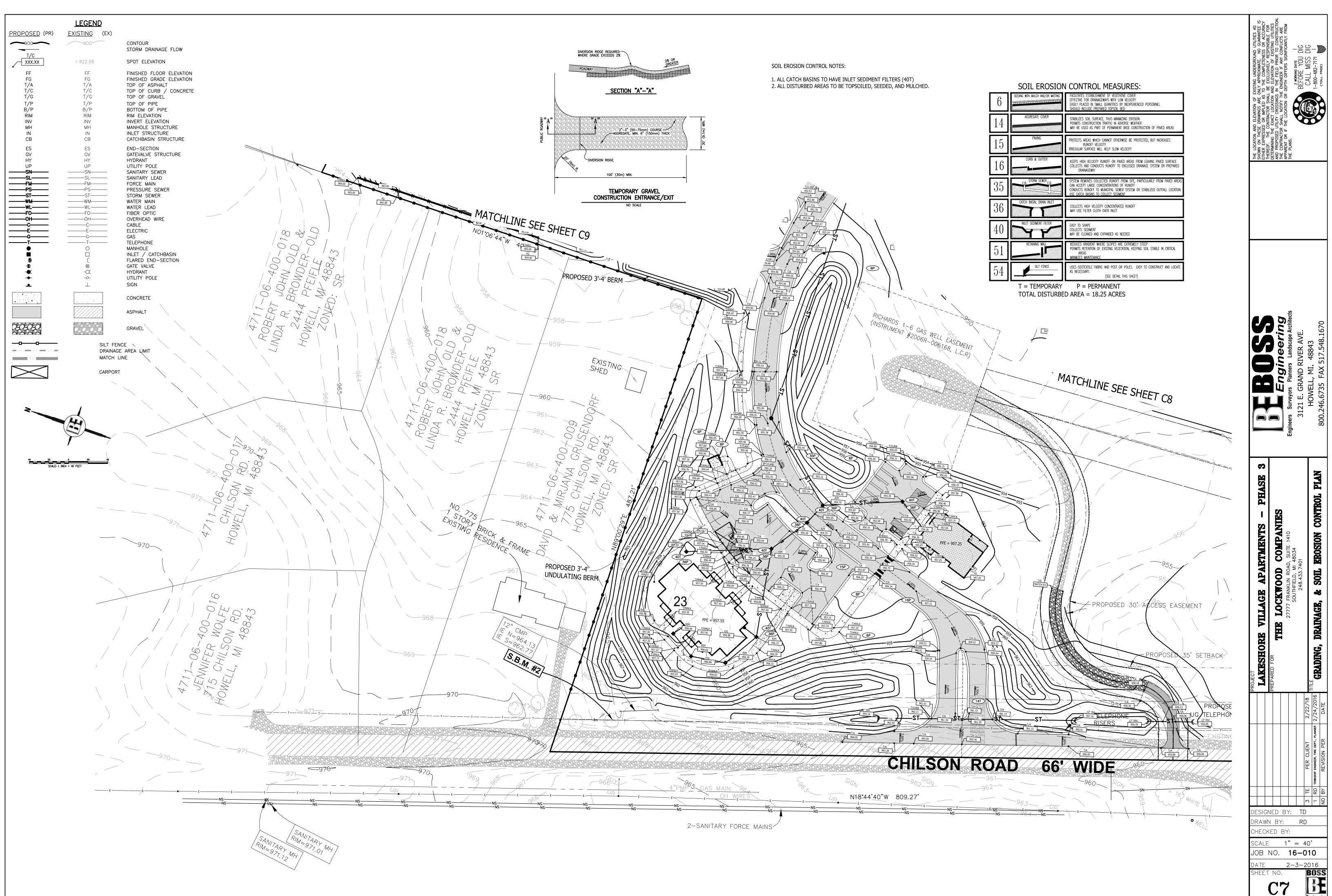
THE LOCKWOOD COMPANIES 27777 FRANKLIN ROAD, SUITE 1410 SOUTHFIELD, MI 48034 CONTACT: MARK LOCKWOOD PHONE: 248.433.7401 PREPARED BY:

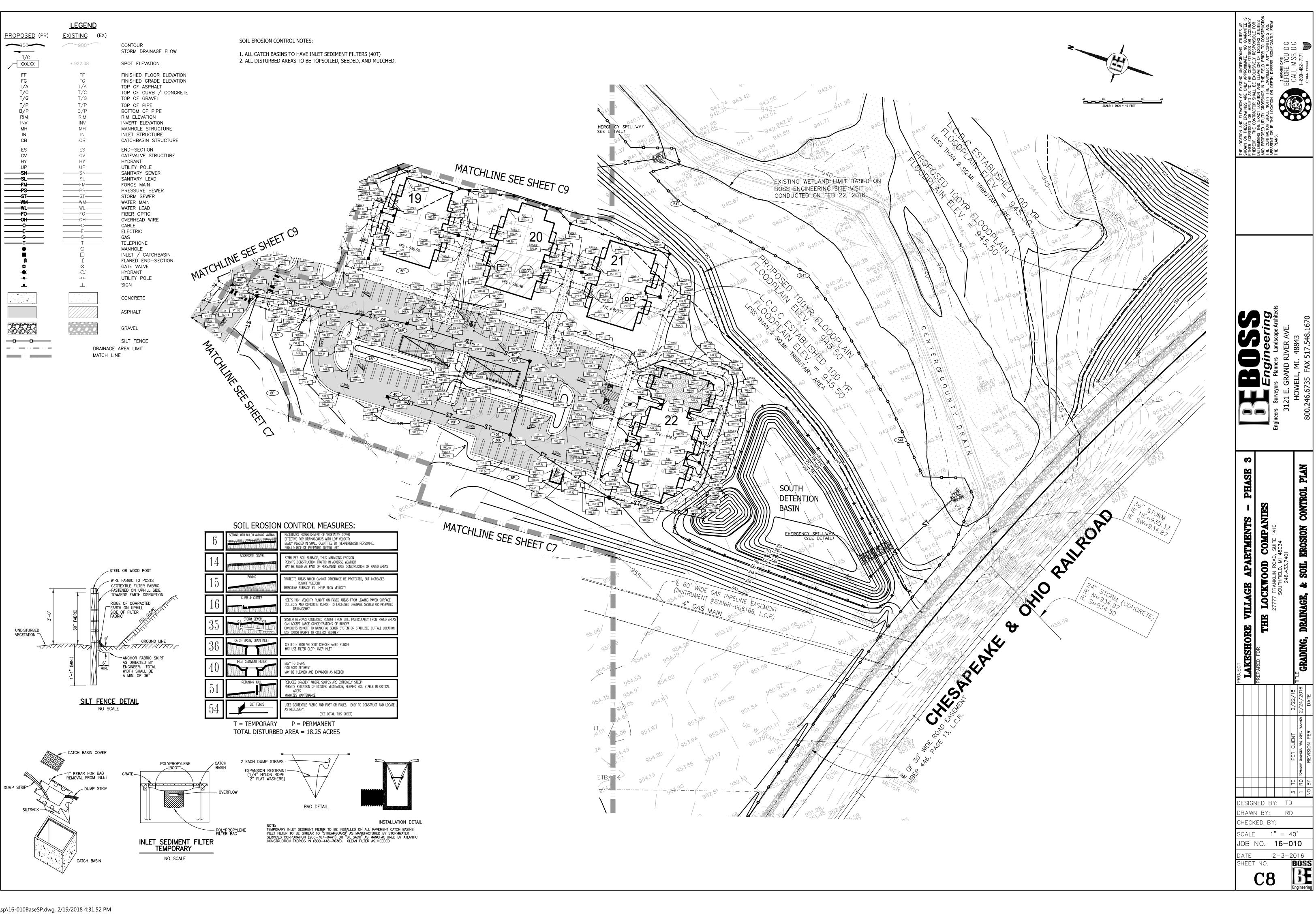


Engineers Surveyors Planners Landscape Architects 3121 E. GRAND RIVER AVE. HOWELL, MI. 48843 800.246.6735 FAX 517.548.1670

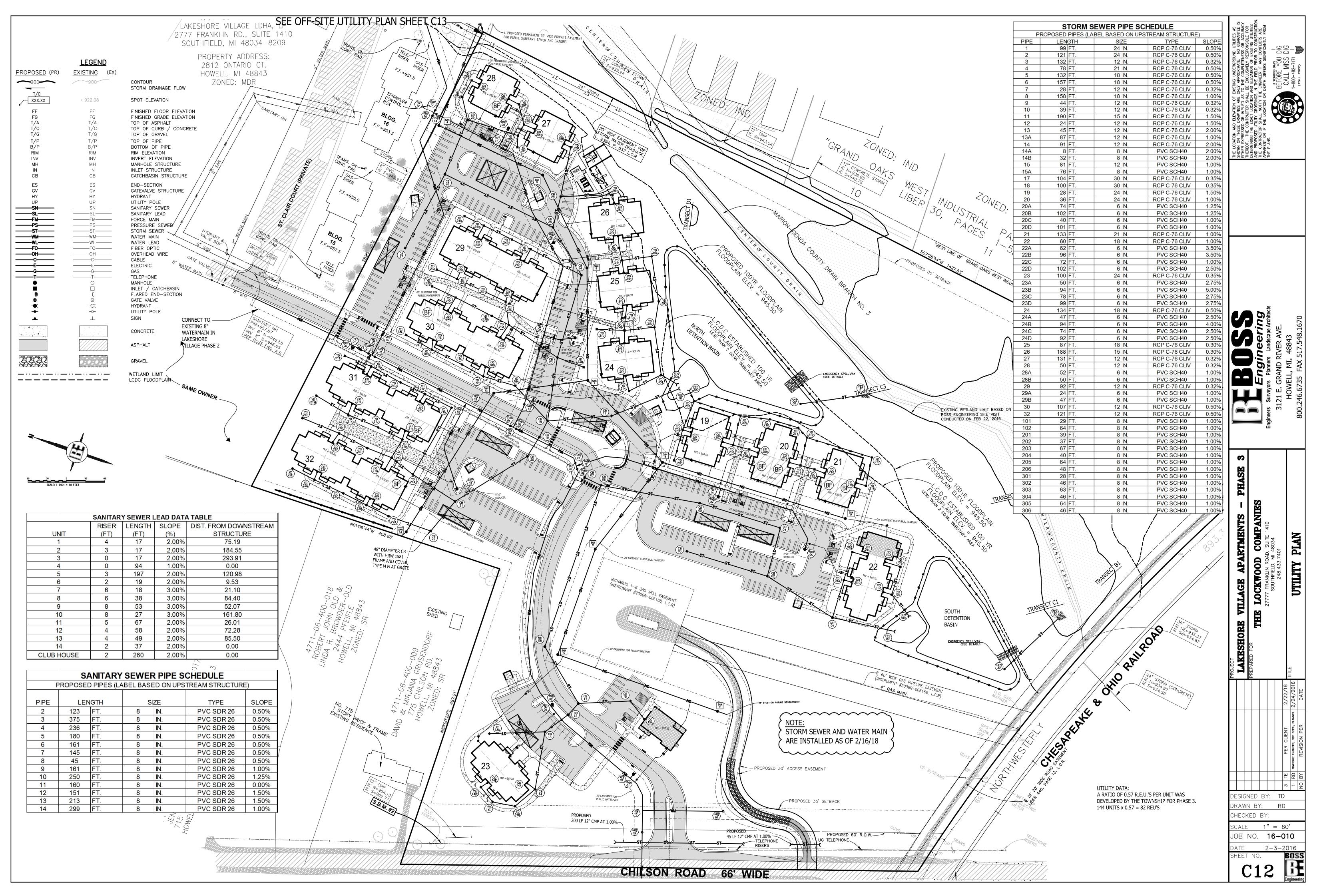








G:\16-010\dwg\sp\16-010BaseSP.dwg, 2/19/2018 4:31:52 PM



		LIVINGSTON COUNTY DETENTION BASIN CALCULATIONS
AREA AREA RUNOFF EQUIV. INTEN- TIME OF ADD'L PIPE FROM TO DRAIN ACRES IMPERV PERV COEFF AREA SITY CONC. RUNOFF		LIVINGSTON COUNTY DETENTION BASIN CALCULATIONS SOUTH BASIN IMPERVIOUS ACRE
AREA A 0.9 0.2 C A*C I T <sub>C</sub> Q	Q (LF) (IN) FULL (FPS) SLOPE % USED CAPACITY (FT/SEC) (MIN) END END END END END (FT) >1 >2.667 COVER	AREA (ACRES) FACTOR IMPERVIOUS 3.00 0.9 2.70
14 CB14 CB13 14 0.14 0.01 0.13 0.26 0.0377 4.37 15.06 0.43 13 CB13 CB12 13 0.23 0.20 0.04 0.79 0.1838 4.34 15.30 0.14 12 CB12 CB11 12 0.84 0.16 0.68 0.33 0.28 4.33 15.42	0.84       91       12       1.07       2.00%       2.00%       5.05       6.43       0.24       953.30       951.47       956.15       952.50       950.67       3.65       2.85       2.65       0.16         1.78       45       12       2.27       2.00%       2.00%       5.05       6.43       0.12       951.47       950.57       950.67       949.77       4.83       4.03       3.83       0.80         3.00       24       12       3.82       1.50%       1.50%       4.38       5.57       0.07       950.57       950.21       949.77       949.41       4.93       4.13       3.93       1.21	0.00 0.7 0.00 8.03 0.2 1.61
11         CB11         MH08         11         0.40         0.21         0.20         0.56         0.2246         4.32         15.49         0.81           8         MH08         CB06         8         -         -         -         4.28         15.86	4.77       103       15       3.89       0.54%       0.75%       5.61       4.57       0.38       950.21       949.44       954.70       949.21       948.44       5.49       4.49       4.24       0.97         4.77       267       15       3.89       0.54%       0.75%       5.61       4.57       0.97       949.44       956.50       948.44       1.65       8.06       7.06       6.81       0.00	COMPOUND C: 0.39 TOTAL DRAINAGE AREA: 11.03 ACRES
6 CB06 CB05 6 0.29 0.08 0.22 0.38 0.1128 4.18 16.84 0.51 5 CB05 CB04 5 0.39 0.27 0.12 0.69 0.2681 4.12 17.46 4 CB04 CB02 4 0.49 0.32 0.17 0.65 0.3212 4.07 17.98	5.76       157       18       3.26       0.50%       0.50%       7.45       4.21       0.62       945.79       945.00       951.70       944.59       943.80       7.11       5.91       4.01       0.47         6.86       132       18       3.88       0.42%       0.50%       7.45       4.21       0.52       945.00       944.34       947.95       943.80       7.11       5.91       4.01       0.47         6.86       132       18       3.88       0.42%       0.50%       7.45       4.21       0.52       945.00       944.34       947.95       943.80       943.14       4.15       2.95       2.65       1.11         8.17       78       24       2.40       0.50%       14.23       0.67       0.28       044.34       043.05       043.04       043.05       043.04       043.05       2.05       2.65       1.21	K1 = AxC (Design Constant) 4.3017
4 CB04 CB02 4 0.49 0.32 0.17 0.65 0.3212 4.07 17.98 2 CB02 MH01 2 1.54 0.55 1.00 0.45 0.6897 4.05 18.26 1.58 1 MH01 FES00 1 4.01 18.65 0.34	8.17       78       21       3.40       0.50%       0.50%       11.23       4.67       0.28       944.34       943.95       942.94       942.55       4.36       2.96       2.61       1.31         12.54       121       24       3.99       0.50%       0.50%       16.04       5.11       0.39       943.95       943.35       947.35       942.35       941.75       5.00       3.40       3.00       2.79         12.89       99       24       4.10       0.50%       0.50%       16.04       5.11       0.32       943.35       942.85       949.00       941.75       941.25       7.25       5.65       5.25       0.00	Qa=MAX ALLOW OUTFLOW (0.083 CFS / ACRE 0.915 CFS DURATION DURATION INTENSITY INFLOW VOLUME OUTFLOW STORAGE VOLUME
3 CB03 CB02 3 1.11 0.20 0.91 0.33 0.3622 4.38 15.00	1.58       132       12       2.02       0.50%       0.50%       2.53       3.22       0.68       945.00       944.34       948.00       944.20       943.54       3.80       3.00       2.80       1.58	DORATION         INTENSITY         INFLOW VOLUME         OUTFLOW         STORAGE VOLUME           MINUTES         SECONDS         (IN/HR)         INCHES         IN. RUNOFF xAxC         DURATION x qo         INFLOW         OUTFLOW           5         300         9.17         2750         11830         275         11555
7 CB07 CB06 7 0.18 0.12 0.06 0.66 0.1164 4.38 15.00	0.51 28 12 0.65 1.00% 1.00% 3.57 4.55 0.10 946.07 945.79 951.70 945.27 944.99 6.43 5.63 5.43 0.51	106007.8647142027954919730159006.8861882661782425793
32 OCS32 FES33	0.92 115 12 1.17 0.50% 0.50% 2.53 3.22 0.59 940.80 940.23 947.00 940.00 939.43 7.00 6.20 6.00 0.00	2012006.117333315461098304483018005.009000387151647370686036003.241164750102329446808
		90         5400         2.39         12913         55548         4941         50607           120         7200         1.90         13655         58740         6588         52152
27         CB27         CB26         27         0.45         0.21         0.23         0.54         0.2394         4.38         15.00           26         CB26         CB25         26         0.37         0.21         0.17         0.59         0.218         4.28         15.85         1.69           25         CB25         CB24         25         0.25         0.23         0.02         0.85         0.2108         4.16         17.04         1.09	1.05       131       12       1.33       0.32%       0.32%       2.02       2.57       0.85       946.05       945.63       951.50       945.01       944.59       6.49       5.45       5.49       1.05         3.67       188       15       2.99       0.32%       0.25%       3.24       2.64       1.19       945.63       950.30       944.39       943.92       5.91       4.67       4.66       0.93         5.65       87       18       3.19       0.29%       0.25%       5.27       2.98       0.49       945.03       944.78       948.75       943.72       943.50       5.03       3.72       3.53       0.88	180 10800 1.34 14488 62322 9882 52440
24 CB24 CB23 24 0.39 0.22 0.17 0.60 0.2348 4.12 17.52 0.51 23 CB23 CB18 23 0.32 0.21 0.11 0.66 0.2101 4.06 18.12 1.49	7.12       134       18       4.03       0.46%       0.40%       6.66       3.77       0.59       944.78       947.80       943.50       942.97       4.30       3.02       2.80       0.97         9.46       100       24       3.01       0.35%       0.35%       13.42       4.27       0.39       944.17       943.82       948.55       942.97       4.30       3.02       2.80       0.97	REQUIRED 100 YEAR DETENTION VOLUME =     52440 CF       FOREBAY VOLUME
18 CB18 CB17 18 0.22 0.20 0.02 0.84 0.1879 4.02 18.51 12.93 17 CB17 FES16 17 0.35 0.28 0.08 0.75 0.2641 3.99 18.85	23.15 100 30 4.72 0.32% 0.35% 24.33 4.96 0.34 943.72 943.39 949.20 941.72 941.36 7.48 5.48 4.98 0.76 24.20 114 30 4.93 0.35% 0.32% 23.27 4.74 0.40 943.39 943.00 948.40 941.36 941.00 7.04 5.01 4.54 1.05 941.36	V(F) = 5% OF THE 100-YEAR STORM VOLUME BASED ON THE AREA TRIBUTARY TO THE INLET
22 CB22 CB21 22 5.59 0.09 5.50 0.21 1.1805 4.38 15.00 0.68 21 CB21 CB20 21 0.56 0.38 0.19 0.67 0.377 4.36 15.17 2.09	5.85       60       18       3.31       1.00%       10.0%       10.53       5.96       0.17       946.52       945.92       951.50       945.32       944.72       6.18       4.98       4.68       5.16         9.58       133       18       5.42       0.83%       1.00%       10.53       5.96       0.37       945.92       951.50       944.72       943.39       6.78       5.58       5.28       1.64	V(F)= (.05)(V100) V(F)= 2622 CF
20 CB20 CB19 20 0.38 0.29 0.09 0.73 0.2786 4.32 15.54 19 CB19 CB18 19 0.85 0.24 0.61 0.40 0.3411 4.31 15.63	10.78       36       18       6.10       1.05%       1.25%       11.78       6.66       0.09       944.59       944.14       950.80       943.39       942.94       7.41       6.21       5.91       1.20         12.25       28       18       6.93       1.35%       1.50%       12.90       7.30       0.06       944.14       943.72       949.20       942.94       942.52       6.26       5.06       4.76       1.47	FOREBAY STORAGE VOLUME REQUIRED:     2622 CF       FOREBAY STORAGE VOLUME PROVIDED:
29 CB29 CB26 29 0.32 0.25 0.07 0.75 0.2398 4.38 15.00 0.64	1.69       92       12       2.15       0.32%       0.32%       2.02       2.57       0.60       945.69       948.85       944.89       944.59       3.96       3.16       2.96       1.05	CUMMULATIVE ELEV AREA VOLUME VOLUME
28 CB28 CB25 28 0.29 0.14 0.15 0.53 0.1524 4.38 15.00 0.43	1.09 50 12 1.39 0.32% 0.32% 2.02 2.57 0.32 945.08 944.92 948.45 944.28 944.12 4.17 3.37 3.17 0.67	945         1587         1368         3407         DESIGN HIGHWATER ELEVATION           944         1149         959         2039           943         769         607         1080
30 OCS30 FES31	1.27 96 12 1.61 0.50% 0.50% 2.53 3.22 0.50 940.80 940.32 946.00 940.00 939.52 6.00 5.20 5.00 0.00	942 445 473 <b>473</b> 941 501 0 <b>0</b> BOTTOM OF FOREBAY
STORM SEWER INVENTORY		940         251         0         0           939         76         0         0
FES 00 CB11	CB22	BANKFULL FLOOD VOLUME
FLARED END SECTION W/ ANIMAL GRATE     4' DIA CATCH BASIN, COVER 'K'       RIM     N/A       IN/CEDT     24       O41 25	4'CB, COVER 'EJIW 1581 TYPE M FLAT GRATE' RIM 951.50	V <sub>BF</sub> = 5160 x A x C= 22197 CF
INVERT 24 941.25 INV. E 15 949.21 INV. N 12 949.41 INV. S 12 949.41	INV. SE 18 945.32 INV. N 6 946.12 2' SLIMP	FIRST FLUSH VOLUME           VFF = 1815 x A x C=         7808 CF
MH01 2' SUMP	2' SUMP	BASIN STORAGE PROVIDED ELEV. AREA DEPTH VOLUME TOTAL
4' DIA MANHOLE, COVER 'A'           RIM         949.00         CB12           INV. S         24         941.75         4' DIA CATCH BASIN, COVER 'K'	CB23 4' DIA CATCH BASIN, COVER 'D'	(FT <sup>2</sup> ) (FT) (FT <sup>3</sup> ) VOLUME (FT <sup>3</sup> )
INV. N 24 941.75 RIM 954.70	RIM 948.55 INV. E 24 942.57	946         21075.79         1         18,673         80,126         FREEBOARD ELEVATION           945         16270         1         15,437 <b>61,453 DESIGN HIGHWATER ELEVATION</b>
INV. E 6 942.95 INV. E 12 949.77 INV. W 12 949.77	INV.NW 18 942.97 INV.N 6 943.77	94414604113,80446,01694313003112,23332,21294211464110,71619,979
	INV. S 6 943.77 2' SUMP	941996719,2639,263940855900
4' DIA CATCH BASIN, COVER 'D'     CB13       RIM     947.35     4' DIA CATCH BASIN, COVER 'D'	CB24	BOTTOM OF BASIN = 940.00
INV. S         24         942.35         RIM         955.50           INV. E         21         942.55         INV. SW         12         950.67           INV. NUME         10         942.54         INV. SW         12         950.67	4' DIA CATCH BASIN, COVER 'D' RIM 947.80	FIRST FLUSH X <sub>FF</sub> = 940.84
INV. N         12         943.54         INV. NW         6         951.07           2' SUMP         INV. W         12         951.07	INV. E 18 943.50 INV.NW 18 943.50	<u>BANKFULL</u> $X_{BF} = 942.17$
CB03	INV. N 6 944.30 2' SUMP	<u>100 YEAR</u> X <sub>100</sub> = 944.42
2' DIA CATCH BASIN, COVER 'D'     CB14       RIM     948.00     4' DIA CATCH BASIN, COVER 'E'	CB25	OUTLET CONTROL STRUCTURE
INV. S         12         944.20         RIM         956.15           2' SUMP         INV. E         12         952.50	4' DIA CATCH BASIN, COVER 'D'         RIM       948.75	<u>FIRST FLUSH OF RUNOFF</u> THE AVERAGE ALLOWABLE RELEASE RATE FOR RUNOFF IS 0.5" OVER AREA OF SITE IN 24 HRS.
INV. NW         6         952.90           CB04         INV. N         6         952.90	INV. SW 18 943.72 INV.N 15 943.92	Q <sub>FF</sub> = V <sub>FF</sub> x (1/24HRS) x (1HR/3600SEC)= 0.090 CFS
4' DIA CATCH BASIN, COVER 'D'         2' SUMP           RIM         947.30	INV. E 12 944.12 2' SUMP	PLACE OPENINGS IN STANDPIPE AT BOTTOM OF BASIN = 940.00
INV. W         21         942.94         CB15           INV. N         18         943.14         4' DIA CATCH BASIN, COVER 'K'	CB26	$HEAD = h_{FF} = X_{FF} - BOTTOM BASIN ELEV = 0.84 FT$
2' SUMP INV. N 12 950.23	4' DIA CATCH BASIN, COVER 'D' RIM 950.30	$A = Q_{FF} / (0.62 \text{ x} (2 \text{ x} 32.2 \text{ x} h_{FF})^{0.5}) = 0.020 \text{ FT}^2$
CB05         INV. W         8         950.49           4' DIA CATCH BASIN, COVER 'D'         2' SUMP         2' SUMP	INV.S 15 944.39 INV.W 12 944.59	A         1         INCH DIAMETER ORIFICE HAS AN AREA OF         0.0055         SF           A/ 0.0055         =         3.63
RIM 947.95 INV. S 18 943.80 <b>FES 16</b>	INV. E 12 944.59 2' SUMP	THEREFORE, USE THE FOLLOWING NUMBER OF 1 INCH DIAMETER HOLES
INV. N         18         943.80         FLARED END SECTION W/ ANIMAL GRATE           2' SUMP         RIM         N/A	CB27	3.00 HOLES, AT ELEV. 940.00 Q <sub>FF</sub> MAX = 0.171 CFS
INVERT 30 941.00	2' DIA CATCH BASIN, COVER 'D' RIM 951.50	<u>BANKFULL FLOOD</u> FOR THE ALLOWABLE RELEASE RATE OF 24-40 HOURS, CHECK THE DISCHARGE THROUGH THE
4' DIA CATCH BASIN, COVER 'K'           RIM         951.70           CB17	INV. E 12 945.01 2' SUMP	FIRST FLUSH ORIFICE TO SEE IF ADDITIONAL HOLES ARE NECESSARY.
INV. SE         18         944.59         5' DIA CATCH BASIN, COVER 'D'           INV. SW         15         946.44         RIM         948.40	CB28	HEAD = h = $X_{BF}$ - BOTTOM OF BASIN = 2.17 FT
INV. N         12         944.99         INV. SE         30         941.36           2' SUMP         INV. NW         30         941.36	4' DIA CATCH BASIN, COVER 'D' RIM 948.45	$Q_{90.0} = 0.62x \text{ #HOLES x (AREA EACH HOLE_{FF}) x (2 x 32.2 x h)^{0.5} =$ 0.120 CFS $T_{90.0} = (1SEC / Q_{90.0}) x V_{BF} x (1HR / 3600SEC) =$ 51.41 HRS
2' SUMP	INV. W 12 944.28 INV. NE 6 944.68	BECAUSE THE HOLDING TIME EXCEEDS 40 HRS, ADDITIONAL ORIFICES IN THE STANDPIPE ARE REQUIRED.
2' DIA CATCH BASIN, COVER 'K'         CB18           RIM         951.70         5' DIA CATCH BASIN, COVER 'K'	INV. SE 6 944.68 2' SUMP	VOLUME THROUGH31INCH DIAMETER HOLES IN 24 HOURS:V=Q90.0x24HRSx3600SEC/HR =10362 CFDEMAINING VOL-
INV. S         12         945.27         RIM         949.20           2' SUMP         INV. SE         30         941.72	CB29	REMAINING VOL. =         11834 CF           QBF = REMAINING VOLUME x (1 / 24HRS) x (1 / 3600SEC) =         0.137 CFS
INV. E         24         942.22           MH08         INV. W         18         942.52	4' DIA CATCH BASIN, COVER 'D' RIM 948.85	PLACE OPENINGS AT FIRST FLUSH ELEVATION =940.84HEAD = hBF = XBF - XFF =1.33 FT
4' DIA. MANHOLE, COVER 'A'         INV. N         6         942.92           RIM         956.50         2' SUMP	INV. W 12 944.89 INV. E 6 945.29	A = QBF / ( 0.62 * (2*32.2*hBF)^0.5) = 0.024 SF A 1 INCH DIAMETER ORIFICE HAS AN AREA OF 0.0055 SF
INV. E 15 948.44 INV.W 15 948.44 <b>CB19</b>	INV. S 6 945.29 2' SUMP	A/ 0.0055 = 4.38 THEREFORE, USE 4 1 INCH DIAMETER HOLES AT ELEV. = 940.84
4' DIA CATCH BASIN, COVER 'K' RIM 949.20	OCS30	$Q_{BF}MAX = 0.205$ CFS
*NOTE: STRUCTURES 9 AND 10 INV. E 18 942.94 INV. W 18 942.94	OUTLET CONTROL STUCTURE       RIM     945.00	<u>100 YEAR FLOOD</u> Q <sub>a</sub> = ALLOWABLE RELEASE RATE x AREA SITE IN ACRES= 0.915 CFS
INTENTIONALLY OMMITTED 2' SUMP	INVERT 12 940.00	$Q_a = ALLOWABLE RELEASE RATE x AREA SITE IN ACRES = 0.915 CFS$ $Q_a$ IS A PEAK OR MAXIMUM FLOW. CALCULATE THE MAXIMUM FLOW PASSING THROUGH FIRST
CB20 4' DIA CATCH BASIN, COVER 'D'	FES31	$Q_a$ is a peak or maximum plow. Calculate the maximum plow passing through project from $Q_a$ to determine the orifice size to release the 100 year storm volume:
RIM 950.80 INV. E 18 943.39	FLARED END SECTION       RIM     N/A	$Q_{FF}MAX+Q_{BF}MAX = 0.38 CFS$
INV.NW 18 943.39 2' SUMP	INVERT 12 939.52	$Q_a - (Q_{FF}MAX + Q_{BF}MAX) = 0.54 CFS$
CB21	OCS32	$A = Q_a / (0.62 * (2 * 32.2 * (X_{100}-X_{BF}))^{0.5}) = 0.072 \text{ SF}$
4' DIA CATCH BASIN, COVER 'D' RIM 951.50	OUTLET CONTROL STUCTURE RIM 945.00	A 1.5 INCH DIAMETER ORIFICE HAS AN AREA OF 0.012 SF
INV. SE 18 944.72 INV. NW 18 944.72	INVERT 12 940.00	A/ 0.012 = 5.89
2'SUMP	FES33	THEREFORE, USE THE FOLLOWING NUMBER OF1.5INCH DIAMETER HOLES:5 HOLES AT ELEV. =942.17
	FLARED END SECTION W/ ANIMAL GRATE RIM N/A	$Q_{100} = 0.458$ CFS $Q_0 = Q_{100} + Q_{BF}MAX + Q_{FF}MAX$ 0.834 CFS
	INVERT 12 939.43	Q <sub>0</sub> = Q <sub>100</sub> + Q <sub>BF</sub> MAX + Q <sub>FF</sub> MAX 0.834 CFS SUMMARY OF REQUIRED STANDPIPE HOLES:
		ELEVATION# OF HOLESDIAMETER OF HOLES942.1751.5
		940.84         4         1         INCHES           940.00         3         1         INCHES

	IMPERVIOUS	ACRE	CULATIONS				
AREA (ACRES) 4.71	IMPERVIOUS FACTOR 0.9	ACRE IMPERVIOUS 4.24					
0.00	0.7	0.00					
COMPOUND C:		0.42					
		14.99	ACRES				
K1 = AxC (Desig Qa=MAX ALLOV		083 CFS / ACRE	6.2958 1.244	CFS			
DURATION MINUTES	DURATION SECONDS	INTENSITY (IN/HR)	INCHES	INFLOW VOLUME	OUTFLOW DURATION X Qo	STORAGE VOLUME INFLOW - OUTFLOW	
5	300 600	9.17 7.86	2750 4714	17313 29680	373 746	16940 28934	
15 20	900 1200	6.88 6.11	6188 7333	38955 46169	1120 1493	37836 44676	
30 60	1800 3600	5.00 3.24	9000 11647	56662 73328	2239 4478	54423 68849	
90 120	5400 7200	2.39 1.90	12913 13655	81298 85970	6718 8957	74580 77013	
180	10800	1.34	14488	91212	13435	77777	
REQUIRED 100	YEAR DETENTION	ON VOLUME =	77777	CF			
<b>FOREBAY VOLU</b> V(F) = 5% OF Th		TORM VOLUME	BASED ON TH	E AREA TRIBUTA	RY TO THE INLET		
V(F)=	(.05)(V100)						
V(F)= FOREBAY STOP	3889 RAGE VOLUME		3889	CF			
FOREBAY STOP	RAGEVOLUME						
ELEV	AREA	VOLUME	CUMMULATIVE VOLUME				
<b>945</b> 944	2339 1788	2064 1541	5352 3289	DESIGN HIGHWA	ATER ELEVATION		
943 942	1293 852	1073 676	1748 676				
941 940	499 239	369 0	369 0				
BANKFULL FLC							
V <sub>BF</sub> = 5160 x A x	: C=	32486	CF				
FIRST FLUSH V V <sub>FF</sub> = 1815 x A >		11427	CF				
BASIN STORAG		1142/	<b>9</b> 1				
ELEV.	AREA (FT <sup>2</sup> )	DEPTH (FT)	VOLUME (FT <sup>3</sup> )	TOTAL			
046		(FT)		(FT <sup>3</sup> )	FREEDOADD ELE		
946 <b>945</b> 944	29815 <b>24205</b> 21941	1 1 1	27,010 23,073 20,829	113,823 86,813 63,740	FREEBOARD ELEV DESIGN HIGHWAT		
944 943 942	19717 17572	1 1 1	20,829 18,645 16,526	42,911 24,266			
942 941 940	15480 13520	1	7,740	7,740 0			
		=	940.00				
FIRST FLUSH		X <sub>FF</sub> =	941.22				
BANKFULL		X <sub>BF</sub> =	942.42				
100 Y EAR		X <sub>100</sub> =	944.61				
OUTLET CONTR		<u>E</u>					
FIRST FLUSH O	F RUNOFF		For Runoff I	S 0.5" OVER ARE	A OF SITE IN 24 HF	RS.	
FIRST FLUSH O	<u>F RUNOFF</u> ALLOWABLE R	ELEASE RATE F	FOR RUNOFF I	S 0.5" OVER ARE 0.132		RS.	
<u>FIRST FLUSH O</u> THE AVERAGE Q <sub>FF</sub> = V <sub>FF</sub> x (1/2	<u>F RUNOFF</u> ALLOWABLE R 4HRS) x (1HR/3	ELEASE RATE F				RS. 940.00	
<u>FIRST FLUSH O</u> THE AVERAGE Q <sub>FF</sub> = V <sub>FF</sub> x (1/2	<u>F RUNOFF</u> ALLOWABLE R 4HRS) x (1HR/3 GS IN STANDPI	ELEASE RATE F 600SEC)= PE AT BOTTOM			CFS		
<u>FIRST FLUSH O</u> THE AVERAGE Q <sub>FF</sub> = V <sub>FF</sub> x (1/2 PLACE OPENIN	<u>F RUNOFF</u> ALLOWABLE R 4HRS) x (1HR/3 GS IN STANDPI FF - BOTTOM BA	ELEASE RATE F 600SEC)= PE AT BOTTOM ASIN ELEV =		0.132	CFS FT		
FIRST FLUSH O THE AVERAGE $Q_{FF} = V_{FF} x (1/2)$ PLACE OPENIN HEAD = $h_{FF} = X_{FF}$	<u>F RUNOFF</u> ALLOWABLE R 4HRS) x (1HR/3 GS IN STANDPI FF - BOTTOM BA	ELEASE RATE F 600SEC)= PE AT BOTTOM ASIN ELEV =	OF BASIN =	0.132	CFS FT		
$\frac{FIRST FLUSH O}{THE AVERAGE}$ $Q_{FF} = V_{FF} \times (1/2)$ $PLACE OPENIN$ $HEAD = h_{FF} = X_{FF}$ $A = Q_{FF} / (0.62 \times A)$	<u>F RUNOFF</u> ALLOWABLE R 4HRS) x (1HR/30 GS IN STANDPI FF - BOTTOM BA (2 x 32.2 x h <sub>FF</sub> )	ELEASE RATE F 600SEC)= PE AT BOTTOM ASIN ELEV =	OF BASIN =	0.132	CFS FT FT <sup>2</sup>	940.00	
FIRST FLUSH O THE AVERAGE $Q_{FF} = V_{FF} \times (1/2)$ PLACE OPENIN HEAD = $h_{FF} = X_F$ A = $Q_{FF} / (0.62 \times A)$ A	<u>F RUNOFF</u> ALLOWABLE R 4HRS) x (1HR/30 GS IN STANDPI 5F - BOTTOM BA 5 (2 x 32.2 x h <sub>FF</sub> ) 1 0.0055 <b>SE THE FOLLO</b>	ELEASE RATE F 600SEC)= PE AT BOTTOM ASIN ELEV = <sup>0.5</sup> ) = INCH DIAMETEF	OF BASIN = R ORIFICE HAS 4.41	0.132	CFS FT FT <sup>2</sup>	940.00 SF	
$\frac{FIRST FLUSH O}{THE AVERAGE}$ $Q_{FF} = V_{FF} \times (1/2)$ $PLACE OPENIN$ $HEAD = h_{FF} = X_{F}$ $A = Q_{FF} / (0.62 \times A)$ $A$ $A = A$	<u>F RUNOFF</u> ALLOWABLE R 4HRS) x (1HR/3 GS IN STANDPI GS IN STANDPI (2 x 32.2 x h <sub>FF</sub> ) 1 0.0055 SE THE FOLLO	ELEASE RATE F 600SEC)= PE AT BOTTOM ASIN ELEV = <sup>0.5</sup> ) = INCH DIAMETEF = WING NUMBER AT ELEV.	OF BASIN = R ORIFICE HAS 4.41	0.132 1.22 0.024 S AN AREA OF	CFS FT FT <sup>2</sup> 0.0055	940.00 SF	
FIRST FLUSH O         THE AVERAGE $Q_{FF} = V_{FF} \times (1/2)$ PLACE OPENIN         HEAD = $h_{FF} = X_{F}$ A = $Q_{FF} / (0.62 \times A)$ BANKFULL FLO	<u>F RUNOFF</u> ALLOWABLE R 4HRS) x (1HR/3) GS IN STANDPI (2 x 32.2 x h <sub>FF</sub> ) 1 0.0055 SE THE FOLLON HOLES, 0.233 OD	ELEASE RATE F 600SEC)= PE AT BOTTOM ASIN ELEV = <sup>0.5</sup> ) = INCH DIAMETEF = WING NUMBER AT ELEV. CFS	OF BASIN = R ORIFICE HAS 4.41 OF 940.00	0.132 1.22 0.024 S AN AREA OF 1	CFS FT FT <sup>2</sup> 0.0055 INCH DIAMETER H	940.00 SF OLES	
FIRST FLUSH O         THE AVERAGE $Q_{FF} = V_{FF} \times (1/2)$ PLACE OPENIN         HEAD = $h_{FF} = X_F$ A = $Q_{FF} / (0.62 \times A)$ BANKEFORE, US         BANKFULL FLO         FOR THE ALLON	<u>F RUNOFF</u> ALLOWABLE R 4HRS) x (1HR/3) GS IN STANDPI (2 x 32.2 x h <sub>FF</sub> ) 1 0.0055 <b>SE THE FOLLON</b> HOLES, 0.233 <u>OD</u> WABLE RELEAS	ELEASE RATE F 600SEC)= PE AT BOTTOM ASIN ELEV = <sup>0.5</sup> ) = INCH DIAMETEF = WING NUMBER AT ELEV. CFS	OF BASIN = R ORIFICE HAS 4.41 <b>OF</b> <b>940.00</b> 40 HOURS, CH	0.132 1.22 0.024 5 AN AREA OF 1 ECK THE DISCHA	CFS FT FT <sup>2</sup> 0.0055	940.00 SF OLES	
FIRST FLUSH O         THE AVERAGE $Q_{FF} = V_{FF} \times (1/2)$ PLACE OPENIN         HEAD = $h_{FF} = X_F$ A = $Q_{FF} / (0.62 \times A)$ BANKEFULL FLO         FOR THE ALLON	<u>F RUNOFF</u> ALLOWABLE R 4HRS) x (1HR/3) GS IN STANDPI (3 (2 x 32.2 x h <sub>FF</sub> ) 1 0.0055 <b>SE THE FOLLO</b> <b>HOLES</b> , 0.233 <u>OD</u> WABLE RELEAS RIFICE TO SEE	ELEASE RATE F 600SEC)= PE AT BOTTOM ASIN ELEV = <sup>0.5</sup> ) = INCH DIAMETEF = WING NUMBER AT ELEV. CFS SE RATE OF 24- IF ADDITIONAL	OF BASIN = R ORIFICE HAS 4.41 <b>OF</b> <b>940.00</b> 40 HOURS, CH	0.132 1.22 0.024 5 AN AREA OF 1 ECK THE DISCHA	CFS FT FT <sup>2</sup> 0.0055 INCH DIAMETER H	940.00 SF OLES	
FIRST FLUSH O         THE AVERAGE $Q_{FF} = V_{FF} \times (1/2)$ PLACE OPENIN         HEAD = $h_{FF} = X_{F}$ A = $Q_{FF} / (0.62 \times A)$ BANKFULL FLO         FIRST FLUSH O         HEAD = $h = X_{BF}$	<u>F RUNOFF</u> ALLOWABLE R 4HRS) x (1HR/30 GS IN STANDPI GS IN STANDPI (2 x 32.2 x h <sub>FF</sub> ) 1 0.0055 <b>SE THE FOLLOY</b> HOLES, 0.233 <u>OD</u> NABLE RELEAS RIFICE TO SEE - BOTTOM OF E	ELEASE RATE F 600SEC)= PE AT BOTTOM ASIN ELEV = <sup>0.5</sup> ) = INCH DIAMETEF = WING NUMBER AT ELEV. CFS SE RATE OF 24- IF ADDITIONAL	OF BASIN = R ORIFICE HAS 4.41 OF 940.00 40 HOURS, CH HOLES ARE N	0.132 1.22 0.024 S AN AREA OF 1 ECK THE DISCHA ECESSARY. 2.42	CFS FT FT <sup>2</sup> 0.0055 INCH DIAMETER H	940.00 SF OLES	CFS
FIRST FLUSH O         THE AVERAGE $Q_{FF} = V_{FF} x (1/2)$ PLACE OPENIN         HEAD = $h_{FF} = X_{F}$ A = $Q_{FF} / (0.62 \times A)$ BANKFULL FLO         FOR THE ALLOW         FIRST FLUSH O         HEAD = $h = X_{BF}$ $Q_{90.0} = 0.62 \times H$	F RUNOFF ALLOWABLE R 4HRS) x (1HR/30 GS IN STANDPI GS IN STANDPI (2 x 32.2 x h <sub>FF</sub> ) 1 0.0055 SE THE FOLLON HOLES, 0.233 OD WABLE RELEAS RIFICE TO SEE - BOTTOM OF E OLES x (AREA	ELEASE RATE F 600SEC)= PE AT BOTTOM ASIN ELEV = <sup>0.5</sup> ) = INCH DIAMETEF = <b>WING NUMBER</b> AT ELEV. CFS SE RATE OF 24- IF ADDITIONAL BASIN =	OF BASIN = R ORIFICE HAS 4.41 <b>OF</b> 940.00 40 HOURS, CH HOLES ARE N	0.132 1.22 0.024 S AN AREA OF 1 ECK THE DISCHA ECESSARY. 2.42	CFS FT FT <sup>2</sup> 0.0055 INCH DIAMETER H	940.00 SF OLES E 0.169	CFS
$\frac{\text{FIRST FLUSH O}}{\text{THE AVERAGE}}$ $Q_{FF} = V_{FF} \times (1/2)$ $PLACE OPENIN$ $HEAD = h_{FF} = X_{F}$ $A = Q_{FF} / (0.62 \times A)$	F RUNOFF ALLOWABLE R 4HRS) x (1HR/30 GS IN STANDPI (2 x 32.2 x h <sub>FF</sub> ) 1 0.0055 SE THE FOLLON HOLES, 0.233 OD WABLE RELEAS RIFICE TO SEE - BOTTOM OF E OLES x (AREA Q <sub>90.0</sub> ) x V <sub>BF</sub> x (	ELEASE RATE F 600SEC)= PE AT BOTTOM ASIN ELEV = 0 <sup>0.5</sup> ) = INCH DIAMETEF = WING NUMBER AT ELEV. CFS SE RATE OF 24- IF ADDITIONAL BASIN = EACH HOLE <sub>FF</sub> ) > 1HR / 3600SEC )	OF BASIN = R ORIFICE HAS 4.41 OF 940.00 40 HOURS, CH HOLES ARE N ( (2 x 32.2 x h) <sup>0</sup> =	0.132 1.22 0.024 S AN AREA OF 1 ECEK THE DISCHA ECESSARY. 2.42	CFS FT FT <sup>2</sup> 0.0055 INCH DIAMETER H	940.00 SF OLES E 0.169 HRS	CFS
$\frac{\text{FIRST FLUSH O}}{\text{THE AVERAGE}}$ $Q_{FF} = V_{FF} \times (1/2)$ $PLACE OPENIN$ $HEAD = h_{FF} = X_{F}$ $A = Q_{FF} / (0.62 \times A)$	<u>F RUNOFF</u> ALLOWABLE R 4HRS) x (1HR/30 GS IN STANDPI (2 x 32.2 x h <sub>FF</sub> ) 1 0.0055 <b>SE THE FOLLON</b> HOLES, 0.233 <u>OD</u> WABLE RELEAS RIFICE TO SEE - BOTTOM OF E OLES x (AREA Q <sub>90.0</sub> ) x V <sub>BF</sub> x ( HOLDING TIME JGH	ELEASE RATE F 600SEC)= PE AT BOTTOM ASIN ELEV = 0 <sup>0.5</sup> ) = INCH DIAMETEF = WING NUMBER AT ELEV. CFS SE RATE OF 24- IF ADDITIONAL BASIN = EACH HOLE <sub>FF</sub> )> 1HR / 3600SEC ) EXCEEDS 40 HF 4	OF BASIN = R ORIFICE HAS 4.41 OF 940.00 40 HOURS, CH HOLES ARE N ( (2 x 32.2 x h) <sup>0</sup> =	0.132 1.22 0.024 S AN AREA OF 1 ECK THE DISCHA ECESSARY. 2.42 <sup>0.5</sup> = L ORIFICES IN THINCH DIAMETER	CFS FT FT <sup>2</sup> 0.0055 INCH DIAMETER H NRGE THROUGH TH FT 53.48	940.00 SF OLES 0.169 HRS REQUIRED.	CFS
FIRST FLUSH O         THE AVERAGE $Q_{FF} = V_{FF} \times (1/2)$ PLACE OPENIN         HEAD = $h_{FF} = X_{FF}$ A = $Q_{FF} / (0.62 \times A)$ BANKFULL FLO         FOR THE ALLOV         FIRST FLUSH O         HEAD = h = $X_{BF}$ $Q_{90.0} = (0.62 \times \#H)$ $T_{90.0} = (1SEC / H)$ BECAUSE THE         VOLUME THROV         VOLUME THROV         REMAINING VO	F RUNOFF           ALLOWABLE R           4HRS) x (1HR/30           GS IN STANDPI           I           0.0055           SE THE FOLLON           HOLES,           0.233           OD           WABLE RELEAS           RIFICE TO SEE           - BOTTOM OF E           OLES X (AREA           Q90.0 ) X VBF X (           HOLDING TIME           JGH           SX3600SEC/HR =	ELEASE RATE F 600SEC)= PE AT BOTTOM ASIN ELEV = 0 <sup>0.5</sup> ) = INCH DIAMETEF = WING NUMBER AT ELEV. CFS SE RATE OF 24- IF ADDITIONAL BASIN = EACH HOLE <sub>FF</sub> )> 1HR / 3600SEC ) EXCEEDS 40 HF 4	OF BASIN = R ORIFICE HAS 4.41 OF 940.00 40 HOURS, CH HOLES ARE N ( (2 x 32.2 x h)) = RS, ADDITIONA 1 14579 CF	0.132 1.22 0.024 S AN AREA OF 1 ECK THE DISCHA ECESSARY. 2.42 <sup>0.5</sup> = L ORIFICES IN THINCH DIAMETER	CFS FT FT <sup>2</sup> 0.0055 INCH DIAMETER H ARGE THROUGH TH FT 53.48	940.00 SF OLES 0.169 HRS REQUIRED.	
FIRST FLUSH O         THE AVERAGE $Q_{FF} = V_{FF} \times (1/2)$ PLACE OPENIN         HEAD = $h_{FF} = X_{FF}$ A = $Q_{FF} / (0.62 \times A)$ A = $Q_{FF} / (0.62 \times A)$ A = $Q_{FF} / (0.62 \times A)$ Marrier (0.62 \times A)         Q_{FF}MAX =         BANKFULL FLO         FOR THE ALLOV         FIRST FLUSH O         HEAD = $h = X_{BF}$ Q_{90.0} = 0.62x #H         T_{90.0} = (1SEC / 1)         BECAUSE THE         VOLUME THROV         QBF = REMAINING VO         QBF = REMAINI         PLACE OPENIN	F RUNOFF           ALLOWABLE R           4HRS) x (1HR/30           GS IN STANDPI           GS AT FIRST FI	ELEASE RATE F 600SEC)= PE AT BOTTOM ASIN ELEV = 0 <sup>0.5</sup> ) = INCH DIAMETEF = WING NUMBER AT ELEV. CFS SE RATE OF 24- IF ADDITIONAL BASIN = EACH HOLE <sub>FF</sub> )> 1HR / 3600SEC ) EXCEEDS 40 HF 4 = 17907	OF BASIN = R ORIFICE HAS 4.41 OF 940.00 40 HOURS, CH HOLES ARE N ( (2 x 32.2 x h)) = RS, ADDITIONA 1 14579 CF / 3600SEC) = N =	0.132 1.22 0.024 S AN AREA OF 1 ECK THE DISCHA ECESSARY. 2.42 <sup>0.5</sup> = L ORIFICES IN THINCH DIAMETER CF	CFS FT FT <sup>2</sup> 0.0055 INCH DIAMETER H ARGE THROUGH TH FT 53.48	940.00 SF OLES 0.169 HRS REQUIRED. RS: 0.207	
FIRST FLUSH O         THE AVERAGE $Q_{FF} = V_{FF} \times (1/2)$ PLACE OPENIN         HEAD = $h_{FF} = X_{F}$ A = $Q_{FF} / (0.62 \times A)$ BANKFULL FLO         FIRST FLUSH O         HEAD = $h = X_{BF}$ $Q_{90.0} = (0.62 \times H)$ $T_{90.0} = (1SEC / 1)$ BECAUSE THE         VOLUME THRON	F RUNOFF         ALLOWABLE R         4HRS) x (1HR/30         GS IN STANDPI         GS IN STANDPI         (2 x 32.2 x hFF)         1         0.0055         SE THE FOLLON         HOLES,         0.233         OD         WABLE RELEAS         RIFICE TO SEE         - BOTTOM OF E         OLES x (AREA         Q90.0 ) x VBF x (         HOLDING TIME         JGH         SX 3600SEC/HR =         NG VOLUME x H         GS AT FIRST FI         (BF -XFF =	ELEASE RATE F 600SEC)= PE AT BOTTOM ASIN ELEV = <sup>0.5</sup> ) = INCH DIAMETEF = WING NUMBER AT ELEV. CFS SE RATE OF 24- IF ADDITIONAL BASIN = EACH HOLE <sub>FF</sub> )> 1HR / 3600SEC ) EXCEEDS 40 HF 4 = 17907 (1 / 24HRS) x (1 / LUSH ELEVATIO	OF BASIN = R ORIFICE HAS 4.41 OF 940.00 40 HOURS, CH HOLES ARE N (2 x 32.2 x h) () = RS, ADDITIONA 1 14579 CF / 3600SEC) =	0.132 1.22 0.024 S AN AREA OF 1 ECK THE DISCHA ECESSARY. 2.42 <sup>0.5</sup> = L ORIFICES IN THINCH DIAMETER CF	CFS FT FT <sup>2</sup> 0.0055 INCH DIAMETER H NRGE THROUGH TH FT 53.48 HE STANDPIPE ARE HOLES IN 24 HOUF 941.22	940.00 SF OLES 0.169 HRS REQUIRED. RS: 0.207	
FIRST FLUSH O         THE AVERAGE $Q_{FF} = V_{FF} \times (1/2)$ PLACE OPENIN         HEAD = $h_{FF} = X_{FF}$ A = $Q_{FF} / (0.62 \times A)$ BANKFULL FLO         FOR THE ALLOV         FIRST FLUSH O         HEAD = $h = X_{BF}$ $Q_{90.0} = 0.62 \times \#H$ $T_{90.0} = (1SEC / P)$ BECAUSE THE         VOLUME THROV         QBF = REMAINING VO         A = QBF / ( 0.62 \times A)	F RUNOFF ALLOWABLE R ALLOWABLE R 4HRS) x (1HR/3) GS IN STANDPI (GS IN STANDPI (C2 x 32.2 x h <sub>FF</sub> ) (C2 x 2 x h <sub>FF</sub> )	ELEASE RATE F 600SEC)= PE AT BOTTOM ASIN ELEV = <sup>0.5</sup> ) = INCH DIAMETEF = WING NUMBER AT ELEV. CFS SE RATE OF 24- IF ADDITIONAL BASIN = EACH HOLE <sub>FF</sub> )> 1HR / 3600SEC ) EXCEEDS 40 HF 4 = 17907 (1 / 24HRS) x (1, - USH ELEVATIO '0.5) = INCH DIAMETEF	OF BASIN = R ORIFICE HAS 4.41 OF 940.00 40 HOURS, CH HOLES ARE N ( (2 x 32.2 x h)) = RS, ADDITIONA 1 14579 CF / 3600SEC) = N = 1.19 R ORIFICE HAS	0.132 1.22 0.024 3 AN AREA OF 1 ECK THE DISCHA ECESSARY. 2.42 <sup>0.5</sup> = L ORIFICES IN THINCH DIAMETER CF FT 0.038	CFS FT FT <sup>2</sup> 0.0055 INCH DIAMETER H NRGE THROUGH TH FT 53.48 HE STANDPIPE ARE HOLES IN 24 HOUF 941.22	940.00 SF OLES 0.169 HRS REQUIRED. RS: 0.207	
FIRST FLUSH O         THE AVERAGE $Q_{FF} = V_{FF} \times (1/2)$ PLACE OPENIN         HEAD = $h_{FF} = X_{FF}$ A = $Q_{FF} / (0.62 \times A)$ BANKFULL FLO         FOR THE ALLOV         FIRST FLUSH O         HEAD = $h = X_{BF}$ $Q_{90.0} = 0.62 \times \# H$ $T_{90.0} = (1SEC / P)$ BECAUSE THE         VOLUME THROV         QBF = REMAINING VO         A = QBF / ( 0.62 \times A)         A         A = QBF / ( 0.62 \times A)	F RUNOFF ALLOWABLE R 4HRS) x (1HR/3) GS IN STANDPI GS IN STANDPI (2 x 32.2 x h <sub>FF</sub> ) (2 x 32.2 x h <sub>FF</sub> ) (3 (2 x 32.2 x h <sub>FF</sub> ) (4 (2 x 32.2 x h <sub>FF</sub> )) (5 (2 x 32.2 x h <sub>FF</sub> )) (6 (2 x 32.2 x h <sub>FF</sub> )) (7 (1 (2 x 32.2 x h <sub>FF</sub> )) (7 (1 (2 x 32.2 t h <sub>FF</sub> )) (7 (1 (2 x 32.2 t h <sub>FF</sub> )) (7 (1 (2 x 32.2 t h <sub>FF</sub> )) (7 (1 (2 x 32.2 t h <sub>FF</sub> )) (7 (1 (2 x 32.2 t h <sub>FF</sub> ))) (7 (1 (2 x 32.2 t h <sub>FF</sub> ))) (7 (1 (2 x 32.2 t h <sub>FF</sub> ))) (7 (1 (2 x 32.2 t h <sub>FF</sub> ))) (7 (1 (2 x 32.2 t h <sub>FF</sub> ))) (7 (1 (2 x 32.2 t h <sub>FF</sub> ))) (7 (1 (2 x 32.2 t h <sub>FF</sub> ))) (7 (1 (2 x 32.2 t h <sub>FF</sub> ))) (7 (1 (2 x 32.2 t h <sub>FF</sub> ))) (7 (1 (2 x 32.2 t h <sub>FF</sub> ))) (7 (1 (2 x 32.2 t h <sub>FF</sub> ))) (7 (1 (2 x 32.2 t h <sub>FF</sub> ))) (7 (1 (2 x 32.2 t h <sub>FF</sub> ))))	ELEASE RATE F 600SEC)= PE AT BOTTOM ASIN ELEV = $0^{0.5}$ ) = INCH DIAMETEF = WING NUMBER AT ELEV. CFS SE RATE OF 24- IF ADDITIONAL BASIN = EACH HOLE <sub>FF</sub> )> 1HR / 3600SEC ) EXCEEDS 40 HF 4 = 17907 (1 / 24HRS) × (1. USH ELEVATIO 0.5) = INCH DIAMETEF = 6	OF BASIN = R ORIFICE HAS 4.41 OF 940.00 40 HOURS, CH HOLES ARE N (2 x 32.2 x h) () = RS, ADDITIONA 1 14579 CF / 3600SEC) = N = 1.19	0.132 1.22 0.024 S AN AREA OF 1 ECK THE DISCHA ECESSARY. 2.42 <sup>0.5</sup> = L ORIFICES IN THINCH DIAMETER CF FT 0.038 S AN AREA OF	CFS FT FT <sup>2</sup> 0.0055 INCH DIAMETER H ARGE THROUGH TH FT 53.48 HE STANDPIPE ARE HOLES IN 24 HOUF 941.22 SF	940.00 SF OLES 0.169 HRS REQUIRED. RS: 0.207 SF	
FIRST FLUSH O         THE AVERAGE $Q_{FF} = V_{FF} \times (1/2)$ PLACE OPENIN         HEAD = $h_{FF} = X_{FF}$ A = $Q_{FF} / (0.62 \times A)$ BANKFULL FLO         FOR THE ALLOV         FOR THE ALLOV         FIRST FLUSH O         HEAD = $h = X_{BF}$ $Q_{90.0} = 0.62x \# H$ $T_{90.0} = (1SEC / T)$ BECAUSE THE         VOLUME THROV         V=Q90.0x24HRS         REMAINING VO         QBF = REMAINI         PLACE OPENIN         HEAD = hBF = $X$ A = QBF / (0.62)         A         QBF MAX =	F RUNOFF ALLOWABLE R ALLOWABLE R 4HRS) x (1HR/3) GS IN STANDPI GS IN STANDPI (2 x 32.2 x h <sub>FF</sub> ) 1 0.0055 SE THE FOLLOM HOLES, 0.233 OD NABLE RELEAS RIFICE TO SEE - BOTTOM OF E OLES x (AREA Q90.0) X V <sub>BF</sub> x ( HOLDING TIME UGH Sx3600SEC/HR = L = NG VOLUME x 1 GS AT FIRST FI CBS AT FIRST F	ELEASE RATE F 600SEC)= PE AT BOTTOM ASIN ELEV = <sup>0.5</sup> ) = INCH DIAMETEF = <b>WING NUMBER</b> <b>AT ELEV.</b> CFS SE RATE OF 24- IF ADDITIONAL BASIN = EACH HOLE <sub>FF</sub> )> 1HR / 3600SEC ) EXCEEDS 40 HF 4 = 17907 (1 / 24HRS) x (1, - USH ELEVATIO '0.5) = INCH DIAMETEF =	OF BASIN = R ORIFICE HAS 4.41 OF 940.00 40 HOURS, CH HOLES ARE N ( (2 x 32.2 x h)) = RS, ADDITIONA 1 14579 CF / 3600SEC) = N = 1.19 R ORIFICE HAS 6.99	0.132 1.22 0.024 S AN AREA OF 1 ECK THE DISCHA ECESSARY. 2.42 <sup>0.5</sup> = L ORIFICES IN THINCH DIAMETER CF FT 0.038 S AN AREA OF	CFS FT FT <sup>2</sup> 0.0055 INCH DIAMETER H ARGE THROUGH TH FT 53.48 HE STANDPIPE ARE HOLES IN 24 HOUF 941.22 SF 0.0055	940.00 SF OLES 0.169 HRS REQUIRED. RS: 0.207 SF	CFS
FIRST FLUSH O         THE AVERAGE $Q_{FF} = V_{FF} \times (1/2)$ PLACE OPENIN         HEAD = $h_{FF} = X_{F}$ A = $Q_{FF} / (0.62 \times A)$ A = $Q_{FF} / (0.62 \times A)$ A = $Q_{FF} / (0.62 \times A)$ BANKFULL FLO         FOR THE ALLON         FIRST FLUSH O         HEAD = $h = X_{BF}$ $Q_{90.0} = 0.62x \# H$ $T_{90.0} = (1SEC / B)$ BECAUSE THE         VOLUME THRON         V=Q90.0x24HRS         REMAINING VO         QBF = REMAINI         PLACE OPENIN         HEAD = hBF = $\lambda$ A = QBF / (0.62)         A         QBF MAX =         100 YEAR FLOCE	F RUNOFF ALLOWABLE R 4HRS) x (1HR/3) GS IN STANDPI GS IN STANDPI (2 x 32.2 x h <sub>FF</sub> ) (2 x 32.2 x h <sub>FF</sub> ) (2 x 32.2 x h <sub>FF</sub> ) (3 0.0055 (4 C x 32.2 x h <sub>FF</sub> ) (5 C x 32.2 x h <sub>FF</sub> ) (6 C x 32.2 x h <sub>FF</sub> ) (7 C x 4 x 4 x 4 x 4 x 4 x 4 x 4 x 4 x 4 x	ELEASE RATE F 600SEC)= PE AT BOTTOM ASIN ELEV = <sup>0.5</sup> ) = INCH DIAMETEF = WING NUMBER AT ELEV. CFS SE RATE OF 24 IF ADDITIONAL BASIN = EACH HOLE <sub>FF</sub> ) > 1HR / 3600SEC ) EXCEEDS 40 HF 4 = 17907 (1 / 24HRS) x (1 / USH ELEVATIO 0.5) = INCH DIAMETEF = 6 CFS	OF BASIN = R ORIFICE HAS 4.41 OF 940.00 40 HOURS, CH HOLES ARE N (2 x 32.2 x h) (2 x 32.2 x h) (2 x 32.2 x h) (2 x 32.2 x h) (3 CF (3 600SEC) = N = 1.19 R ORIFICE HAS 6.99 1	0.132 1.22 0.024 S AN AREA OF 1 ECK THE DISCHA ECESSARY. 2.42 <sup>0.5</sup> = L ORIFICES IN THINCH DIAMETER CF FT 0.038 S AN AREA OF	CFS FT FT <sup>2</sup> 0.0055 INCH DIAMETER H ARGE THROUGH TH FT 53.48 HE STANDPIPE ARE HOLES IN 24 HOUF 941.22 SF 0.0055	940.00 SF OLES 0.169 HRS E REQUIRED. RS: 0.207 SF	CFS 941.22
FIRST FLUSH O         THE AVERAGE $Q_{FF} = V_{FF} \times (1/2)$ PLACE OPENIN         HEAD = $h_{FF} = X_i$ A = $Q_{FF} / (0.62 \times A)$ BANKFULL FLO         FOR THE ALLON         FIRST FLUSH O         HEAD = h = $X_{BF}$ $Q_{90.0} = (0.62x \# H)$ $T_{90.0} = (1SEC / 0)$ BECAUSE THE         VOLUME THRON         V= Q90.0 × 24HRS         REMAINING VO         QBF = REMAINI         PLACE OPENIN         HEAD = hBF = $X$ A = QBF / (0.62)         A $A/T$ THEREFORE, US $Q_{BF}MAX =$ 100 YEAR FLOC $Q_a = ALLOWAB$	$\frac{F \text{ RUNOFF}}{ALLOWABLE R}$ $\frac{ALLOWABLE R}{ALLOWABLE R}$ $\frac{4HRS) \times (1HR/3)}{GS IN STANDPI}$ $\frac{GS IN STANDPI}{GS IN STANDPI}$ $\frac{GS IN STANDPI}{GS IN STANDPI}$ $\frac{1}{1}$	ELEASE RATE F 600SEC)= PE AT BOTTOM ASIN ELEV = $0^{0.5}$ ) = INCH DIAMETEF = WING NUMBER AT ELEV. CFS SE RATE OF 24- IF ADDITIONAL BASIN = EACH HOLE <sub>FF</sub> ) > 1HR / 3600SEC ) EXCEEDS 40 HF 4 = 17907 (1 / 24HRS) × (1 / USH ELEVATIO '0.5) = INCH DIAMETEF = 6 CFS ATE × AREA SIT	OF BASIN = R ORIFICE HAS 4.41 OF 940.00 40 HOURS, CH HOLES ARE N (2 x 32.2 x h) (2 x 32.2 x h) (3 (2 x 32.2 x h) (4 (2 x 32.2 x h)) (4 (2 x 32.2 x h)) (5 (2 x 32.2 x h)) (5 (2 x 32.2 x h)) (6 (2 x 32.2 x h)) (7 (2 x 32.2 x h)) (7 (2 x 32.2 x h)) (8 (2 x 32.2 x h)) (9 (2 x 32.2 x h)) (1	0.132 1.22 0.024 S AN AREA OF 1 ECK THE DISCHA ECESSARY. 2.42 <sup>0.5</sup> = L ORIFICES IN TH INCH DIAMETER CF FT 0.038 S AN AREA OF INCH DIAMETER	CFS FT FT <sup>2</sup> 0.0055 INCH DIAMETER H ARGE THROUGH TH FT 53.48 IE STANDPIPE ARE HOLES IN 24 HOUF 941.22 SF 0.0055 HOLES AT ELEV. =	940.00 SF OLES 0.169 HRS E REQUIRED. RS: 0.207 SF	CFS 941.22
FIRST FLUSH O         THE AVERAGE $Q_{FF} = V_{FF} \times (1/2)$ PLACE OPENIN         HEAD = $h_{FF} = X_{FF}$ A = $Q_{FF} / (0.62 \times A)$ BANKFULL FLO         FOR THE ALLOV         FIRST FLUSH O         HEAD = $h = X_{BF}$ $Q_{90.0} = 0.62 \times \# H$ $T_{90.0} = (1SEC / H)$ BECAUSE THE         VOLUME THROV         VQ=090.0x24HRS         REMAINING VO         QBF = REMAINI         PLACE OPENIN         HEAD = hBF = $X$ A = QBF / (0.62 \times A)         A         QBF = REMAINING VO	F RUNOFF ALLOWABLE R ALLOWABLE R 4HRS) $\times$ (1HR/3) GS IN STANDPI (2 X 32.2 X hFF) 1 0.0055 SE THE FOLLOW HOLES, 0.233 OD WABLE RELEAS RIFICE TO SEE - BOTTOM OF E OLES X (AREA Q90.0 ) X VBF X ( HOLDING TIME JGH SX3600SEC/HR = L = NG VOLUME X 1 GS AT FIRST FI (BF -XFF = 2 * (2*32.2*hBF)/ 1 0.0055 SE 0.300 D LE RELEASE R	ELEASE RATE F 600SEC)= PE AT BOTTOM ASIN ELEV = <sup>0.5</sup> ) = INCH DIAMETEF = WING NUMBER AT ELEV. CFS SE RATE OF 24- IF ADDITIONAL BASIN = EACH HOLE <sub>FF</sub> )> 1HR / 3600SEC ) EXCEEDS 40 HF 4 = 17907 (1 / 24HRS) x (1 / LUSH ELEVATIO '0.5) = INCH DIAMETEF = 6 CFS ATE x AREA SIT OW. CALCULAT	OF BASIN = R ORIFICE HAS 4.41 OF 940.00 40 HOURS, CH HOLES ARE N (2 x 32.2 x h) = RS, ADDITIONA 1 14579 CF / 3600SEC) = N = 1.19 R ORIFICE HAS 6.99 1 E IN ACRES= E THE MAXIMU	0.132 1.22 0.024 3 AN AREA OF 1 ECK THE DISCHA ECESSARY. 2.42 <sup>0.5</sup> = 1 ORIFICES IN THINCH DIAMETER CF FT 0.038 3 AN AREA OF INCH DIAMETER S AN AREA OF INCH DIAMETER	CFS FT FT <sup>2</sup> 0.0055 INCH DIAMETER H ARGE THROUGH TH FT 53.48 HE STANDPIPE ARE HOLES IN 24 HOUF 941.22 SF 0.0055	940.00 SF OLES OLES 0.169 HRS E REQUIRED. RS: 0.207 SF SF	CFS 941.22
FIRST FLUSH O         THE AVERAGE $Q_{FF} = V_{FF} \times (1/2)$ PLACE OPENIN         HEAD = $h_{FF} = X_{FF}$ A = $Q_{FF} / (0.62 \times A)$ BANKFULL FLO         FOR THE ALLOW         FIRST FLUSH O         HEAD = $h = X_{0F}$ $Q_{90.0} = 0.62 \times \# H$ $T_{90.0} = (1SEC / B)$ BECAUSE THE         VOLUME THROW         VOLUME THROW         VOLUME THROW         VOLUME THROW         VOLUME THROW         VA = QBF / (0.62 \times A)         REMAINING VO         QBF = REMAINING         VOLUME THROW         VOLUME THROW         QBF = REMAINING         PLACE OPENIN         HEAD = hBF = $X$ A = QBF / (0.62 \times A)         A         QBF         HEAD = hBF = $X$ A = QBF / (0.62 \times A)         A         QBF         A = QBF / (0.62 \times A)	F RUNOFF         ALLOWABLE R         ALLOWABLE R         4HRS) $\times$ (1HR/3)         GS IN STANDPI         GS IN STANDPI $(2 \times 32.2 \times h_{FF})$ 1         0.0055         SE THE FOLLOW         HOLES,         0.233         OD         WABLE RELEAS         RIFICE TO SEE         - BOTTOM OF E         OLES $\times$ (AREA         Q90.0 ) $\times$ V <sub>BF</sub> $\times$ (         HOLDING TIME         JGH         SX 3600SEC/HR =         L =         NG VOLUME $\times$ 1         GS AT FIRST FI         & (2*32.2*hBF)'         1         0.0055         SE         0.300         DD         LE RELEASE R         NK FULL ORIFIC	ELEASE RATE F 600SEC)= PE AT BOTTOM ASIN ELEV = <sup>0.5</sup> ) = INCH DIAMETEF = WING NUMBER AT ELEV. CFS SE RATE OF 24- IF ADDITIONAL BASIN = EACH HOLE <sub>FF</sub> )> 1HR / 3600SEC ) EXCEEDS 40 HF 4 = 17907 (1 / 24HRS) x (1 / LUSH ELEVATIO '0.5) = INCH DIAMETEF = 6 CFS ATE x AREA SIT OW. CALCULAT	OF BASIN = R ORIFICE HAS 4.41 OF 940.00 40 HOURS, CH HOLES ARE N ( (2 x 32.2 x h)) = RS, ADDITIONA 1 14579 CF / 3600SEC) = N = 1.19 R ORIFICE HAS 6.99 1 E IN ACRES= E THE MAXIMU TOTAL HEAD,	0.132 1.22 0.024 3 AN AREA OF 1 ECK THE DISCHA ECESSARY. 2.42 <sup>0.5</sup> = L ORIFICES IN THINCH DIAMETER CF FT 0.038 3 AN AREA OF INCH DIAMETER S AN AREA OF INCH DIAMETER JM FLOW PASSIN AND SUBTRACT	CFS FT FT <sup>2</sup> 0.0055 INCH DIAMETER H ARGE THROUGH TH FT 53.48 HE STANDPIPE ARE HOLES IN 24 HOUF 941.22 SF 0.0055 HOLES AT ELEV. =	940.00 SF OLES OLES 0.169 HRS E REQUIRED. RS: 0.207 SF SF	CFS 941.22
FIRST FLUSH O         THE AVERAGE $Q_{FF} = V_{FF} \times (1/2)$ PLACE OPENIN         HEAD = $h_{FF} = X_{FF}$ A = $Q_{FF} / (0.62 \times A)$ BANKFULL FLO         FOR THE ALLOW         FIRST FLUSH O         HEAD = $h = X_{0F}$ $Q_{90.0} = 0.62 \times \# H$ $T_{90.0} = (1SEC / B)$ BECAUSE THE         VOLUME THROW         VOLUME THROW         VOLUME THROW         VOLUME THROW         VOLUME THROW         VA = QBF / (0.62 \times A)         REMAINING VO         QBF = REMAINING         VOLUME THROW         VOLUME THROW         QBF = REMAINING         PLACE OPENIN         HEAD = hBF = $X$ A = QBF / (0.62 \times A)         A         QBF         HEAD = hBF = $X$ A = QBF / (0.62 \times A)         A         QBF         A = QBF / (0.62 \times A)	F RUNOFF         ALLOWABLE R         ALLOWABLE R         4HRS) x (1HR/3)         GS IN STANDPI         GS IN STANDPI $(2 \times 32.2 \times h_{FF})$ 1         0.0055         SE THE FOLLON         HOLES,         0.233         OD         WABLE RELEAS         RIFICE TO SEE         - BOTTOM OF E         OLES x (AREA         Q90.0 ) X V <sub>BF</sub> x (         HOLDING TIME         JGH         SX3600SEC/HR =         L =         NG VOLUME x (         GS AT FIRST FI $(2^*32.2^*hBF)^2$ 1         0.0055         SE         0.300         2D         LE RELEASE R         NK MAXIMUM FL         NKFULL ORIFIC         ZE TO RELEASE	ELEASE RATE F 600SEC)= PE AT BOTTOM ASIN ELEV = <sup>0.5</sup> ) = INCH DIAMETEF = WING NUMBER AT ELEV. CFS SE RATE OF 24- IF ADDITIONAL BASIN = EACH HOLE <sub>FF</sub> )> 1HR / 3600SEC ) EXCEEDS 40 HF 4 = 17907 (1 / 24HRS) x (1, LUSH ELEVATIO '0.5) = INCH DIAMETEF = 6 CFS ATE x AREA SIT OW. CALCULAT ES, USING THE E THE 100 YEAF	OF BASIN = R ORIFICE HAS 4.41 OF 940.00 40 HOURS, CH HOLES ARE N ( (2 x 32.2 x h)) = RS, ADDITIONA 1 14579 CF / 3600SEC) = N = 1.19 R ORIFICE HAS 6.99 1 E IN ACRES= E THE MAXIMU TOTAL HEAD,	0.132 1.22 0.024 3 AN AREA OF 1 ECK THE DISCHA ECESSARY. 2.42 <sup>0.5</sup> = L ORIFICES IN THINCH DIAMETER CF FT 0.038 3 AN AREA OF INCH DIAMETER S AN AREA OF INCH DIAMETER JM FLOW PASSIN AND SUBTRACT	CFS FT FT <sup>2</sup> 0.0055 INCH DIAMETER H ARGE THROUGH TH FT 53.48 HE STANDPIPE ARE HOLES IN 24 HOUF 941.22 SF 0.0055 HOLES AT ELEV. =	940.00 SF OLES OLES 0.169 HRS E REQUIRED. RS: 0.207 SF SF	CFS 941.22
FIRST FLUSH O         THE AVERAGE $Q_{FF} = V_{FF} \times (1/2)$ PLACE OPENIN         HEAD = $h_{FF} = X_{FF}$ A = $Q_{FF} / (0.62 \times A)$ BANKFULL FLO         FIRST FLUSH O         HEAD = $h = X_{BF}$ $Q_{90.0} = 0.62 \times \# H$ $T_{90.0} = (1SEC / P)$ BECAUSE THE         VOLUME THROUND         VOLUME THROUND         QBF = REMAINING VO	F RUNOFF         ALLOWABLE R         4HRS) $\times$ (1HR/3)         GS IN STANDPI         GS IN STANDPI         GS IN STANDPI $(2 \times 32.2 \times h_{FF})$ 1         0.0055         SE THE FOLLON         HOLES,         0.233         OD         NABLE RELEAS         RIFICE TO SEE         - BOTTOM OF E         OLES $\times$ (AREA         Q90.0 ) $\times V_{BF} \times$ (         HOLDING TIME         JGH         SX3600SEC/HR =         NG VOLUME $\times$ (         GS AT FIRST FI         QS AT FIRST FI         NG VOLUME $\times$ (         BE 0.300         DD         LE RELEASE R         R MAXIMUM FL         NKFULL ORIFIC         ZE TO RELEASE         XX =	ELEASE RATE F 600SEC)= PE AT BOTTOM ASIN ELEV = $0^{0.5}$ = INCH DIAMETEF = WING NUMBER AT ELEV. CFS SE RATE OF 24 IF ADDITIONAL BASIN = EACH HOLE <sub>FF</sub> ) > 1HR / 3600SEC ) EXCEEDS 40 HF 4 = 17907 (1 / 24HRS) × (1 / USH ELEVATIO 0.5) = INCH DIAMETEF = 6 CFS ATE × AREA SIT OW. CALCULAT ES, USING THE E THE 100 YEAF 0.53	OF BASIN = R ORIFICE HAS 4.41 OF 940.00 40 HOURS, CH HOLES ARE N ( (2 x 32.2 x h)) = RS, ADDITIONA 1 14579 CF / 3600SEC) = N = 1.19 R ORIFICE HAS 6.99 1 E IN ACRES= E THE MAXIMU TOTAL HEAD, R STORM VOLU	0.132 1.22 0.024 3 AN AREA OF 1 ECK THE DISCHA ECESSARY. 2.42 <sup>0.5</sup> = L ORIFICES IN THINCH DIAMETER CF FT 0.038 3 AN AREA OF INCH DIAMETER S AN AREA OF INCH DIAMETER JM FLOW PASSIN AND SUBTRACT	CFS FT FT <sup>2</sup> 0.0055 INCH DIAMETER H ARGE THROUGH TH FT 53.48 HE STANDPIPE ARE HOLES IN 24 HOUF 941.22 SF 0.0055 HOLES AT ELEV. =	940.00 SF OLES OLES 0.169 HRS E REQUIRED. RS: 0.207 SF SF	CFS 941.22
FIRST FLUSH O         THE AVERAGE $Q_{FF} = V_{FF} \times (1/2)$ PLACE OPENIN         HEAD = $h_{FF} = X_{FF}$ A = $Q_{FF} / (0.62 \times A)$ BANKFULL FLO         FOR THE ALLOW         FOR THE ALLOW         FIRST FLUSH O         HEAD = h = $X_{BF}$ $Q_{90.0} = 0.62 \times \# H$ $T_{90.0} = (1SEC / M)$ BECAUSE THE         VOLUME THROW         V=Q90.0 × 24 HRS         REMAINING VO         QBF = REMAINI         PLACE OPENIN         HEAD = hBF = $X$ A = QBF / (0.62 \times A)         A = QBF / (0.62 \times A)	F RUNOFF         ALLOWABLE R         4HRS) x (1HR/3)         GS IN STANDPI         GS IN STANDPI $GS$ AT FIE $OLES$ X (AREA $Q_{90,0}$ ) X V <sub>BF</sub> X (         HOLDING TIME         JGH         SX3600SEC/HR =         L         ING VOLUME X         GS AT FIRST FI         GS AT FIRST FI         QS AT FIRST FI         QS AT FIRST FI         QBF -XFF =         2' (2*32.2*hBF)'         1         0.00055         SE         0.300         2D         LE RELEASE R         'R MAXIMUM FL         NKFULL ORIFIC         ZE TO RELEASE         QBF MAX)	ELEASE RATE F 600SEC)= PE AT BOTTOM ASIN ELEV = $0^{0.5}$ = INCH DIAMETEF = WING NUMBER AT ELEV. CFS SE RATE OF 24 IF ADDITIONAL BASIN = EACH HOLE <sub>FF</sub> ) > 1HR / 3600SEC ) EXCEEDS 40 HF 4 = 17907 (1 / 24HRS) X (1 / USH ELEVATIO 0.5) = INCH DIAMETEF = 6 CFS ATE X AREA SIT OW. CALCULAT ES, USING THE E THE 100 YEAF 0.53 0.71	OF BASIN = R ORIFICE HAS 4.41 OF 940.00 40 HOURS, CH HOLES ARE N (2 x 32.2 x h) (2 x 32.2 x h) (2 x 32.2 x h) (3 600SEC) = N = 1.19 R ORIFICE HAS 6.99 1 E IN ACRES= E THE MAXIMU TOTAL HEAD, R STORM VOLU CFS	0.132 1.22 0.024 3 AN AREA OF 1 ECK THE DISCHA ECESSARY. 2.42 <sup>0.5</sup> = L ORIFICES IN THINCH DIAMETER CF FT 0.038 3 AN AREA OF INCH DIAMETER S AN AREA OF INCH DIAMETER JM FLOW PASSIN AND SUBTRACT	CFS FT FT <sup>2</sup> 0.0055 INCH DIAMETER H ARGE THROUGH TH FT 53.48 IE STANDPIPE ARE HOLES IN 24 HOUF 941.22 SF 0.0055 HOLES AT ELEV. = NG THROUGH FIRS <sup>T</sup> FROM Q <sub>a</sub> TO DE TEI	940.00 SF OLES OLES 0.169 HRS E REQUIRED. RS: 0.207 SF SF	CFS 941.22
FIRST FLUSH O         THE AVERAGE $Q_{FF} = V_{FF} x (1/2)$ PLACE OPENIN         HEAD = $h_{FF} = X_{F}$ $A = Q_{FF} / (0.62 x)$ $A = Q_{FF} / ($	F RUNOFF         ALLOWABLE R         4HRS) x (1HR/3)         GS IN STANDPI         GS IN STANDPI $GS$ AT FIE $OLES$ X (AREA $Q_{90,0}$ ) X V <sub>BF</sub> X (         HOLDING TIME         JGH         SX3600SEC/HR =         L         ING VOLUME X         GS AT FIRST FI         GS AT FIRST FI         QS AT FIRST FI         QS AT FIRST FI         QBF -XFF =         2' (2*32.2*hBF)'         1         0.00055         SE         0.300         2D         LE RELEASE R         'R MAXIMUM FL         NKFULL ORIFIC         ZE TO RELEASE         QBF MAX)	ELEASE RATE F 600SEC)= PE AT BOTTOM ASIN ELEV = $0^{0.5}$ = INCH DIAMETEF = WING NUMBER AT ELEV. CFS SE RATE OF 24 IF ADDITIONAL BASIN = EACH HOLE <sub>FF</sub> ) > 1HR / 3600SEC ) EXCEEDS 40 HF 4 = 17907 (1 / 24HRS) X (1 / USH ELEVATIO 0.5) = INCH DIAMETEF = 6 CFS ATE X AREA SIT OW. CALCULAT ES, USING THE E THE 100 YEAF 0.53 0.71	OF BASIN = R ORIFICE HAS 4.41 OF 940.00 40 HOURS, CH HOLES ARE N (2 x 32.2 x h) (2 x 32.2 x h) (3 (2 x 32.2 x h) (4 (2 x 32.2 x h)) (4 (2 x 32.2 x h)) (5 (2 x 32.2 x h)) (7 (2 x 32.2 x h)) (7 (2 x 32.2 x h)) (8 (2 x 32.2 x h)) (9 (2 x 32.2 x h)) (9 (2 x 32.2 x h)) (1 (2 x 32.2 x h)) (2	0.132 1.22 0.024 S AN AREA OF 1 ECK THE DISCHA ECESSARY. 2.42 <sup>0.5</sup> = 1 ORIFICES IN TH INCH DIAMETER CF FT 0.038 S AN AREA OF INCH DIAMETER JM FLOW PASSIN AND SUBTRACT I JME: 0.097	CFS FT FT <sup>2</sup> 0.0055 INCH DIAMETER H ARGE THROUGH TH FT 53.48 IE STANDPIPE ARE HOLES IN 24 HOUF 941.22 SF 0.0055 HOLES AT ELEV. = NG THROUGH FIRS <sup>T</sup> FROM Q <sub>a</sub> TO DE TEI	940.00 SF OLES OLES 0.169 HRS E REQUIRED. RS: 0.207 SF SF	CFS 941.22
FIRST FLUSH O         THE AVERAGE $Q_{FF} = V_{FF} x (1/2)$ PLACE OPENIN         HEAD = $h_{FF} = X_{FF}$ $A = Q_{FF} / (0.62 x)$ BANKFULL FLO         FOR THE ALLON         FIRST FLUSH O         HEAD = $h = X_{BF}$ $Q_{90.0} = 0.62 x$ #H $T_{90.0} = (1SEC / 1)$ BECAUSE THE         VOLUME THRON         V= Q90.0 x 24 HRS         REMAINING VO         QBF = REMAINI         PLACE OPENIN         HEAD = hBF = 0 X         A = QBF / (0.62 x)         A $A = QBF / (0.62 x)$ $A = Q_BFMAX = 100$ $Q_a = ALLOWAB$ $Q_a = ALLOWAB$ $Q_a = (Q_FFMAX + Q_BFMAX)$ $A = Q_a / (0.62 x)$	F RUNOFF         ALLOWABLE R         4HRS) x (1HR/3)         GS IN STANDPI         GS IN STANDPI $(2 \times 32.2 \times h_{FF})$ 1         0.0055         SE THE FOLLOY         HOLES,         0.233         OD         NABLE RELEAS         RIFICE TO SEE         - BOTTOM OF E         OLES x (AREA         Q90.0 ) X V <sub>BF</sub> x (         HOLDING TIME         JGH         SX3600SEC/HR =         NG VOLUME x 1         GS AT FIRST FI         QBF -XFF =         2* (2*32.2*hBF)'         1         0.0055         SE         0.300         DD         LE RELEASE R         'R MAXIMUM FL         NK FULL ORIFIC         ZE TO RELEASE         'R         ALIONIC TELEAS         'X =         QBFMAX) =         2 *32.2 * ( X <sub>100</sub> -	ELEASE RATE F 600SEC)= PE AT BOTTOM ASIN ELEV = $^{0.5}$ ) = INCH DIAMETEF = WING NUMBER AT ELEV. CFS SE RATE OF 24 IF ADDITIONAL BASIN = EACH HOLE <sub>FF</sub> ) > 1HR / 3600SEC ) EXCEEDS 40 HF 4 = 17907 (1 / 24HRS) × (1 / LUSH ELEVATIO '0.5) = INCH DIAMETEF = 6 CFS ATE x AREA SIT OW. CALCULAT ES, USING THE E THE 100 YEAF 0.53 0.71 X <sub>BF</sub> )) <sup>0.5</sup> ) =	OF BASIN = R ORIFICE HAS 4.41 OF 940.00 40 HOURS, CH HOLES ARE N (2 x 32.2 x h) (2 x 32.2 x h) (3 (2 x 32.2 x h) (4 (2 x 32.2 x h)) (4 (2 x 32.2 x h)) (5 (2 x 32.2 x h)) (7 (2 x 32.2 x h)) (7 (2 x 32.2 x h)) (8 (2 x 32.2 x h)) (9 (2 x 32.2 x h)) (9 (2 x 32.2 x h)) (1 (2 x 32.2 x h)) (2	0.132 1.22 0.024 S AN AREA OF 1 ECK THE DISCHA ECESSARY. 2.42 <sup>0.5</sup> = 1 ORIFICES IN TH INCH DIAMETER CF FT 0.038 S AN AREA OF INCH DIAMETER JM FLOW PASSIN AND SUBTRACT I JME: 0.097	CFS FT FT <sup>2</sup> 0.0055 INCH DIAMETER H ARGE THROUGH TH FT 53.48 IE STANDPIPE ARE HOLES IN 24 HOUF 941.22 SF 0.0055 HOLES AT ELEV. = IG THROUGH FIRST FROM Q <sub>a</sub> TO DE TEI	940.00 SF OLES 0.169 HRS REQUIRED. REQUIRED. 3.5: 0.207 	CFS 941.22
FIRST FLUSH O         THE AVERAGE $Q_{FF} = V_{FF} x (1/2)$ PLACE OPENIN         HEAD = $h_{FF} = X_{FF}$ A = $Q_{FF} / (0.62 \times A)$ HEREFORE, US $A = 0$	F RUNOFF         ALLOWABLE R         4HRS) x (1HR/3)         GS IN STANDPI $4HRS$ ) x (1HR/3) $(2 \times 32.2 \times h_{FF})$ 1         0.0055         SE THE FOLLON         HOLES, 0.233         OD         NABLE RELEAS         RIFICE TO SEE         - BOTTOM OF E         OLES x (AREA         Q90.0) X VBF x (         HOLDING TIME         JGH         SX3600SEC/HR =         L         GS AT FIRST FI         QBF -XFF =         2* (2*32.2*hBF)?         1         0.0055         SE         0.300         DD         LE RELEASE R         R MAXIMUM FL         NK FULL ORIFIC         Z *32.2 * ( X <sub>100</sub> 2         32.2 * ( X <sub>100</sub> . <td>ELEASE RATE F 600SEC)= PE AT BOTTOM ASIN ELEV = <math>0^{0.5}</math> = INCH DIAMETEF = WING NUMBER AT ELEV. CFS SE RATE OF 24 IF ADDITIONAL BASIN = EACH HOLE<sub>FF</sub>) &gt; 1HR / 3600SEC ) EXCEEDS 40 HF 4 = 17907 (1 / 24HRS) × (1 / LUSH ELEVATIO '0.5) = INCH DIAMETEF = 6 CFS ATE x AREA SIT OW. CALCULAT ES, USING THE E THE 100 YEAF 0.53 0.71 X<sub>BF</sub> ))<sup>0.5</sup>) = INCH DIAMETEF = WING NUMBER</td> <td>OF BASIN = R ORIFICE HAS 4.41 OF 940.00 40 HOURS, CH HOLES ARE N (2 x 32.2 x h) (2 x 32.2 x h) (3 (2 x 32.2 x h) (4 (2 x 32.2 x h)) (4 (2 x 32.2 x h)) (5 (2 x 32.2 x h)) (5 (2 x 32.2 x h)) (7 (2 x 32.2 x h)) (7 (2 x 32.2 x h)) (8 (2 x 32.2 x h)) (9 (2 x 32.2 x h)) (9 (2 x 32.2 x h)) (1 (1 (1 (1 (1 (1 (1 (1 (1 (1 (1 (1 (1 (</td> <td>0.132 1.22 0.024 S AN AREA OF 1 ECK THE DISCHA ECESSARY. 2.42 <sup>0.5</sup> = 1 ORIFICES IN TH INCH DIAMETER CF FT 0.038 S AN AREA OF INCH DIAMETER JM FLOW PASSIN AND SUBTRACT I JME: 0.097</td> <td>CFS FT FT<sup>2</sup> 0.0055 INCH DIAMETER H ARGE THROUGH TH FT 53.48 IE STANDPIPE ARE HOLES IN 24 HOUF 941.22 SF 0.0055 HOLES AT ELEV. = NG THROUGH FIRST FROM Q<sub>a</sub> TO DE TEI</td> <td>940.00 SF OLES OLES 0.169 HRS REQUIRED. 0.169 HRS SF 0.207 0.207 1.244 F RMINE SF</td> <td>CFS 941.22</td>	ELEASE RATE F 600SEC)= PE AT BOTTOM ASIN ELEV = $0^{0.5}$ = INCH DIAMETEF = WING NUMBER AT ELEV. CFS SE RATE OF 24 IF ADDITIONAL BASIN = EACH HOLE <sub>FF</sub> ) > 1HR / 3600SEC ) EXCEEDS 40 HF 4 = 17907 (1 / 24HRS) × (1 / LUSH ELEVATIO '0.5) = INCH DIAMETEF = 6 CFS ATE x AREA SIT OW. CALCULAT ES, USING THE E THE 100 YEAF 0.53 0.71 X <sub>BF</sub> )) <sup>0.5</sup> ) = INCH DIAMETEF = WING NUMBER	OF BASIN = R ORIFICE HAS 4.41 OF 940.00 40 HOURS, CH HOLES ARE N (2 x 32.2 x h) (2 x 32.2 x h) (3 (2 x 32.2 x h) (4 (2 x 32.2 x h)) (4 (2 x 32.2 x h)) (5 (2 x 32.2 x h)) (5 (2 x 32.2 x h)) (7 (2 x 32.2 x h)) (7 (2 x 32.2 x h)) (8 (2 x 32.2 x h)) (9 (2 x 32.2 x h)) (9 (2 x 32.2 x h)) (1 (1 (1 (1 (1 (1 (1 (1 (1 (1 (1 (1 (1 (	0.132 1.22 0.024 S AN AREA OF 1 ECK THE DISCHA ECESSARY. 2.42 <sup>0.5</sup> = 1 ORIFICES IN TH INCH DIAMETER CF FT 0.038 S AN AREA OF INCH DIAMETER JM FLOW PASSIN AND SUBTRACT I JME: 0.097	CFS FT FT <sup>2</sup> 0.0055 INCH DIAMETER H ARGE THROUGH TH FT 53.48 IE STANDPIPE ARE HOLES IN 24 HOUF 941.22 SF 0.0055 HOLES AT ELEV. = NG THROUGH FIRST FROM Q <sub>a</sub> TO DE TEI	940.00 SF OLES OLES 0.169 HRS REQUIRED. 0.169 HRS SF 0.207 0.207 1.244 F RMINE SF	CFS 941.22
FIRST FLUSH O         THE AVERAGE $Q_{FF} = V_{FF} x (1/2)$ PLACE OPENIN         HEAD = $h_{FF} = X_i$ A = $Q_{FF} / (0.62 \times A)$ BANKFULL FLO         FOR THE ALLON         FIRST FLUSH O         HEAD = $h = X_{BF}$ $Q_{90.0} = 0.62 \times \#H$ $T_{90.0} = (1SEC / 1)$ BECAUSE THE         VOLUME THRON         V= Q90.0 × 24HRS         REMAINING VO         QBF = REMAINI         PLACE OPENIN         HEAD = hBF = $X_{A}$ A = QBF / (0.62         A         A/         THEREFORE, US         Q_BF MAX =         100 YEAR FLOO         Qa IS A PEAK C         FLUSH AND BA         THE ORIFICE SI         Qa - (Q_{FF}MAX + Q_{BF}MA)         Qa - (Q_{FF}MAX + Q_{BF}MA)         Qa - (Q_{FF}MAX + Q_{BF}MA)         A / A/	F RUNOFF         ALLOWABLE R         4HRS) x (1HR/3)         GS IN STANDPI         GS IN STANDPI $(2 \times 32.2 \times h_{FF})$ 1         0.0055         SE THE FOLLOY         HOLES,         0.233         OD         NABLE RELEAS         RIFICE TO SEE         - BOTTOM OF E         QUES x (AREA         Q90.0 ) X VBF X (         HOLDING TIME         JGH         SX3600SEC/HR =         NG VOLUME x         GS AT FIRST FI         QBF -XFF =         2* (2*32.2*hBF)?         1         0.0055         SE         0.300         DD         LE RELEASE R         VR MAXIMUM FL         NKFULL ORIFIC         ZE TO RELEASE         VX =         QBF MAX) =         2 *32.2 * (X100 <sup>-1</sup> )         2         0.022	ELEASE RATE F 600SEC)= PE AT BOTTOM ASIN ELEV = $^{0.5}$ ) = INCH DIAMETEF = WING NUMBER AT ELEV. CFS SE RATE OF 24- IF ADDITIONAL BASIN = EACH HOLE <sub>FF</sub> ) > 1HR / 3600SEC ) EXCEEDS 40 HF 4 = 17907 (1 / 24HRS) × (1 / LUSH ELEVATIO '0.5) = INCH DIAMETEF = 6 CFS ATE x AREA SIT OW. CALCULAT ES, USING THE E THE 100 YEAF 0.53 0.71 X <sub>BF</sub> )) <sup>0.5</sup> ) = INCH DIAMETEF = WING NUMBER ' V. =	OF BASIN = R ORIFICE HAS 4.41 OF 940.00 40 HOURS, CH HOLES ARE N (2 x 32.2 x h) (2 x 32.2 x h) (3 (2 x 32.2 x h) (4 (2 x 32.2 x h)) (4 (2 x 32.2 x h)) (5 (2 x 32.2 x h)) (7 (2 x 32.2 x h)) (7 (2 x 32.2 x h)) (8 (2 x 32.2 x h)) (9 (2 x 32.2 x h)) (9 (2 x 32.2 x h)) (1 (1 (1 (1 (1 (1 (1 (1 (1 (1 (1 (1 (1 (	0.132 1.22 0.024 S AN AREA OF 1 ECK THE DISCHA ECESSARY. 2.42 <sup>0.5</sup> = 1 ORIFICES IN TH INCH DIAMETER CF FT 0.038 S AN AREA OF INCH DIAMETER JM FLOW PASSIN AND SUBTRACT I JME: 0.097 S AN AREA OF	CFS FT FT <sup>2</sup> 0.0055 INCH DIAMETER H ARGE THROUGH TH FT 53.48 IE STANDPIPE ARE HOLES IN 24 HOUF 941.22 SF 0.0055 HOLES AT ELEV. = 0.0055 HOLES AT ELEV. = 0.0055 SF 0.0022	940.00 SF OLES OLES 0.169 HRS REQUIRED. 0.169 HRS SF 0.207 0.207 1.244 F RMINE SF	CFS 941.22
FIRST FLUSH O         THE AVERAGE $Q_{FF} = V_{FF} x (1/2)$ PLACE OPENIN         HEAD = $h_{FF} = X_{FF}$ A = $Q_{FF} / (0.62 \times A)$ HEREFORE, US $A = 0$	F RUNOFF         ALLOWABLE R         ALLOWABLE R         4HRS) x (1HR/3)         GS IN STANDPI $F = BOTTOM BA         (2 x 32.2 x hFF)         1         0.0055         SE THE FOLLOY         HOLES,         0.233         OD         NABLE RELEAS         RIFICE TO SEE         - BOTTOM OF E         OLES x (AREA         Q90.0 ) x VBF x (         HOLDING TIME         JGH         SX3600SEC/HR =         NG VOLUME x (         BE THE FOLLOY         MG VOLUME x (         BE THE FOLLOY         MADUATION OF E         OLES x (AREA         Q90.0 ) X VBF X (         HOLDING TIME         JGH         SX3600SEC/HR =         L =         NG VOLUME x (         BE - XFF =         2* (2*32.2*hBF)?         1         0.0055         SE         0.300         DD         LE RELEASE R         R MAXIMUM FL         NKFULL ORIFIC         Z TO RELEASE         NX =      $	ELEASE RATE F 600SEC)= PE AT BOTTOM ASIN ELEV = $^{0.5}$ ) = INCH DIAMETEF = WING NUMBER AT ELEV. CFS SE RATE OF 24- IF ADDITIONAL BASIN = EACH HOLE <sub>FF</sub> ) > 1HR / 3600SEC ) EXCEEDS 40 HF 4 = 17907 (1 / 24HRS) × (1 / LUSH ELEVATIO V0.5) = INCH DIAMETEF = 6 CFS ATE x AREA SIT OW. CALCULAT ES, USING THE E THE 100 YEAF 0.53 0.71 X <sub>8F</sub> )) <sup>0.5</sup> ) = INCH DIAMETEF = WING NUMBER / V. = 0.643	OF BASIN = R ORIFICE HAS 4.41 OF 940.00 40 HOURS, CH HOLES ARE N (2 x 32.2 x h) (2 x 32.2 x h) (3 (2 x 32.2 x h) (4 (2 x 32.2 x h)) (4 (2 x 32.2 x h)) (5 (2 x 32.2 x h)) (5 (2 x 32.2 x h)) (7 (2 x 32.2 x h)) (7 (2 x 32.2 x h)) (8 (2 x 32.2 x h)) (9 (2 x 32.2 x h)) (9 (2 x 32.2 x h)) (1 (1 (1 (1 (1 (1 (1 (1 (1 (1 (1 (1 (1 (	0.132 1.22 0.024 S AN AREA OF 1 ECK THE DISCHA ECESSARY. 2.42 <sup>0.5</sup> = 1 ORIFICES IN TH INCH DIAMETER CF FT 0.038 S AN AREA OF INCH DIAMETER JM FLOW PASSIN AND SUBTRACT I JME: 0.097 S AN AREA OF	CFS FT FT <sup>2</sup> 0.0055 INCH DIAMETER H ARGE THROUGH TH FT 53.48 IE STANDPIPE ARE HOLES IN 24 HOUF 941.22 SF 0.0055 HOLES AT ELEV. = 0.0055 HOLES AT ELEV. = 0.0055 SF 0.0022	940.00 SF OLES OLES 0.169 HRS REQUIRED. 0.169 HRS SF 0.207 0.207 1.244 F RMINE SF	CFS 941.22
FIRST FLUSH O         THE AVERAGE $Q_{FF} = V_{FF} \times (1/2)$ PLACE OPENIN         HEAD = $h_{FF} = X_{F}$ A = $Q_{FF} / (0.62 \times A)$ A = $Q_{FF} / (0.62 \times A)$ A = $Q_{FF} / (0.62 \times A)$ THEREFORE, US $A/I$ THEREFORE, US $A_{00}$ Q_{FF}MAX =         BANKFULL FLO         FOR THE ALLOW         FIRST FLUSH O         HEAD = h = X_{BF} $Q_{90.0} = (1SEC / I)$ BECAUSE THE         VOLUME THROW         V=Q90.0x24HRS         REMAINING VO         QBF = REMAINI         PLACE OPENIN         HEAD = hBF = $X$ A = QBF / (0.62 $A$ Qa = ALLOWAB         Qa = ALLOWAB         Qa = (Q_FFMAX+Q_BFMA)         Qa = (Q_FFMAX+Q_BFMA)         Qa = (Q_FFMAX+Q_BFMA)         Qa = (Q_FFMAX+Q_BFMA)         Qa =	F RUNOFF         ALLOWABLE R         4HRS) x (1HR/3)         GS IN STANDPI         GS IN STANDPI $F_F$ - BOTTOM BA         (2 x 32.2 x h <sub>FF</sub> )         1         0.0055         SE THE FOLLON         HOLES,         0.233         OD         NABLE RELEAS         RIFICE TO SEE         - BOTTOM OF E         OLES x (AREA         Q90.0 ) X V <sub>BF</sub> x (         HOLDING TIME         JGH         SX3600SEC/HR =         NG VOLUME x 1         GS AT FIRST FI         QS AT FIRST FI         QS AT FIRST FI         QBF -XFF =         2' (2*32.2*hBF)'         1         0.0055         SE         0.300         DD         LE RELEASE R         'R MAXIMUM FL         NKFULL ORIFIC         Z         QBF MAX) =         2 *32.2 * ( X <sub>100</sub> -         2         0.022         SE THE FOLLON         HOLES AT ELE         MAX + Q <sub>FF</sub> MAX	ELEASE RATE F 600SEC)= PE AT BOTTOM ASIN ELEV = $^{0.5}$ ) = INCH DIAMETEF = WING NUMBER AT ELEV. CFS SE RATE OF 24- IF ADDITIONAL BASIN = EACH HOLE <sub>FF</sub> ) > 1HR / 3600SEC ) EXCEEDS 40 HF 4 = 17907 (1 / 24HRS) × (1 / LUSH ELEVATIO V0.5) = INCH DIAMETEF = 6 CFS ATE x AREA SIT OW. CALCULAT ES, USING THE E THE 100 YEAF 0.53 0.71 X <sub>8F</sub> )) <sup>0.5</sup> ) = INCH DIAMETEF = WING NUMBER / V. = 0.643	OF BASIN = R ORIFICE HAS 4.41 OF 940.00 40 HOURS, CH HOLES ARE N (2 x 32.2 x h) (2 x 32.2 x h) (2 x 32.2 x h) (3 CF (3 600SEC) = N = 1.19 CF (3 600SEC) = N = 1.19 R ORIFICE HAS 6.99 1 E IN ACRES= E THE MAXIMU TOTAL HEAD, 8 STORM VOLU CFS CFS CFS CFS R ORIFICE HAS 4.43 OF 942.42 CFS	0.132 1.22 0.024 S AN AREA OF 1 ECK THE DISCHA ECESSARY. 2.42 <sup>0.5</sup> = 1 ORIFICES IN TH INCH DIAMETER CF FT 0.038 S AN AREA OF INCH DIAMETER JM FLOW PASSIN AND SUBTRACT I JME: 0.097 S AN AREA OF	CFS FT FT <sup>2</sup> 0.0055 INCH DIAMETER H ARGE THROUGH TH FT 53.48 IE STANDPIPE ARE HOLES IN 24 HOUF 941.22 SF 0.0055 HOLES AT ELEV. = 0.0055 HOLES AT ELEV. = 0.0055 SF 0.0022	940.00 SF OLES OLES 0.169 HRS REQUIRED. 0.169 HRS SF 0.207 0.207 1.244 F RMINE SF	CFS 941.22
FIRST FLUSH O         THE AVERAGE $Q_{FF} = V_{FF} \times (1/2)$ PLACE OPENIN         HEAD = $h_{FF} = X_{F}$ A = $Q_{FF} / (0.62 \times A)$ A = $Q_{FF} / (0.62 \times A)$ A = $Q_{FF} / (0.62 \times A)$ THEREFORE, US $A/I$ THEREFORE, US $A_{00}$ Q_{FF}MAX =         BANKFULL FLO         FOR THE ALLOW         FIRST FLUSH O         HEAD = h = X_{BF} $Q_{90.0} = (1SEC / I)$ BECAUSE THE         VOLUME THROW         V=Q90.0x24HRS         REMAINING VO         QBF = REMAINI         PLACE OPENIN         HEAD = hBF = $X$ A = QBF / (0.62 $A$ Qa = ALLOWAB         Qa = ALLOWAB         Qa = (Q_FFMAX+Q_BFMA)         Qa = (Q_FFMAX+Q_BFMA)         Qa = (Q_FFMAX+Q_BFMA)         Qa = (Q_FFMAX+Q_BFMA)         Qa =	F RUNOFF         ALLOWABLE R         4HRS) x (1HR/3)         GS IN STANDPI         GS IN STANDPI $F_F$ - BOTTOM BA         (2 x 32.2 x h <sub>FF</sub> )         1         0.0055         SE THE FOLLON         HOLES,         0.233         OD         NABLE RELEAS         RIFICE TO SEE         - BOTTOM OF E         OLES x (AREA         Q90.0 ) X V <sub>BF</sub> x (         HOLDING TIME         JGH         SX3600SEC/HR =         NG VOLUME x 1         GS AT FIRST FI         QS AT FIRST FI         QS AT FIRST FI         QBF -XFF =         2' (2*32.2*hBF)'         1         0.0055         SE         0.300         DD         LE RELEASE R         'R MAXIMUM FL         NKFULL ORIFIC         Z         QBF MAX) =         2 *32.2 * ( X <sub>100</sub> -         2         0.022         SE THE FOLLON         HOLES AT ELE         MAX + Q <sub>FF</sub> MAX	ELEASE RATE F 600SEC)= PE AT BOTTOM ASIN ELEV = $^{0.5}$ ) = INCH DIAMETEF = WING NUMBER AT ELEV. CFS SE RATE OF 24 IF ADDITIONAL BASIN = EACH HOLE <sub>FF</sub> ) > 1HR / 3600SEC ) EXCEEDS 40 HF 4 = 17907 (1 / 24HRS) × (1 / LUSH ELEVATIO V0.5) = INCH DIAMETEF = 6 CFS ATE × AREA SIT OW. CALCULAT ES, USING THE E THE 100 YEAF 0.53 0.71 X <sub>8F</sub> )) <sup>0.5</sup> ) = INCH DIAMETEF = 0.53 0.71 X <sub>8F</sub> )) <sup>0.5</sup> ) =	OF BASIN = R ORIFICE HAS 4.41 OF 940.00 40 HOURS, CH HOLES ARE N (2 x 32.2 x h) (2 x 32.2 x h) (3 (2 x 32.2 x h) (4 (2 x 32.2 x h)) (5 (2 x 32.2 x h) (7 (2 x 32.2 x h)) (7 (	0.132 1.22 0.024 S AN AREA OF 1 ECK THE DISCHA ECESSARY. 2.42 <sup>0.5</sup> = 1 ORIFICES IN TH INCH DIAMETER CF FT 0.038 S AN AREA OF INCH DIAMETER JM FLOW PASSIN AND SUBTRACT I JME: 0.097 S AN AREA OF	CFS FT FT <sup>2</sup> 0.0055 INCH DIAMETER H ARGE THROUGH TH FT 53.48 IE STANDPIPE ARE HOLES IN 24 HOUF 941.22 SF 0.0055 HOLES AT ELEV. = 0.0055 HOLES AT ELEV. = 0.0055 SF 0.0022	940.00 SF OLES OLES 0.169 HRS REQUIRED. 0.169 HRS SF 0.207 0.207 1.244 F RMINE SF	CFS 941.22

THE LOCATION AND ELEVATION OF EXISTING UNDERGROUND UTILITIES AS SHOWN ON THESE DRAWINGS ARE ONLY APPROXIMATE. NO GUARANTEE IS ETHHER EXPRESED OR IMPLIED AS TO THE COMPLETENESS OR ACCURACY THEREOF. THE CONTRACTOR SHALL BE EXCLUSIVELY RESPONSIBLE FOR	DETERMINING THE EXACT LOCATION AND ELEVATION OF EXISTING UTILITIES AND PROPOSED UTILITY CROSSINGS IN THE FIELD PRIOR TO CONSTRUCTION. THE CONTRACTOR SHALL NOTIFY THE ENGINEER IF ANY CONFLICTS ARE APPARENT OR IF THE LOCATION OR DEPTH DIFFERS SIGNIFICANTLY FROM THE PLANS.	BEFORE YOU DIG	1-800-482-7171   (TOLL FREE)	
<b>SSUUE</b>	Engineers Surveyors Planners Landscape Architects	3121 E. GRAND RIVER AVE.	R00.246.6735 FAX 517.548.1670	
PROJECT LAKESHORE VILLAGE APARTMENTS – PHASE 3	PREPARED FOR THE LOCKWOOD COMPANIES 2777 FRANKLIN ROAD, SUITE 1410	200 INFIELD, MI 46034 248.433.7401	CONSTRUCTION DETAILS	
		PER CLIENT 2/22/18	TOWNSHIP ENCINEER, FIRE DEPT., PLANNER 2/24/2016 REVISION PER DATE DATE	
DRAWN CHECK SCALE JOB 1 DATE SHEET	ED BY: I NO. <b>1</b> 1 2-	RD N/A 6-01 -3-20		

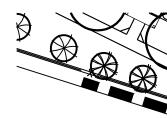
PLANT LIST- (BS) SR ZONING						
QUAN.	<u>KEY</u>	COMMON/ BOTANICAL NAME	<u>SIZE</u>	SPEC.		
7	AB12	Autumn Blaze Maple - clump Acer x. fremanii 'Autumn Blaze'	12' Ht. 4 stem m	B&B in		
6	AB	Autumn Blaze Maple Acer x. fremanii 'Autumn Blaze'	3" Cal.	B&B		
5	со	Hackberry Celtis occidentalis	3" Cal.	B&B		
5	GT	Thornless Honeylocust <i>Gleditsia 'Skyline'</i>	3" Cal.	B&B		
7	QB	Swamp White Oak <i>Quercus bicolor</i>	3" Cal.	B&B		
8	PA10	Norway Spruce <i>Picea abies</i>	10' Ht.	B&B		
9	PA6	Norway Spruce <i>Picea abies</i>	6' Ht.	B&B		
5	PD10	Black Hill Spruce <i>Picea glauca var. densata</i>	10' Ht.	B&B		
8	PD6	Black Hill Spruce <i>Picea glauca var. densata</i>	6' Ht.	B&B		
35	СМ	Cornelian Cherry - clump Cornus mas	4' Ht.	Cont.		
40	sv	Common Lilac S <i>yringa vulgari</i> s	4' Ht.	Cont.		
45	VD	Arrowood Viburnum <i>Viburnum dentatum</i>	36" Ht.	Cont.		

# PLANT LIST - (P

QUAN.	<u>KEY</u>	COMMON/ BOTANICAL NAME	<u>SIZE</u>	SPEC.
7	GT	Thornless Honeylocust Gleditsia 'Skyline'	3" Cal	B&B
9	UA	Accolade Elm <i>Ulmus parviflora 'Morton'</i>	3" Cal	B&B
12	UR	Regal Elm <i>Ulmus carpinifolia 'Regal'</i>	3" Cal	B&B

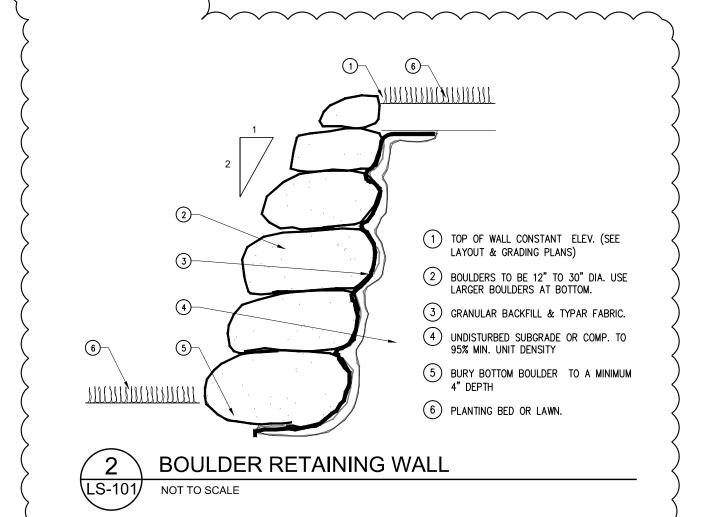
#### PLANT LIST - (F) FRONTAGE TREES COMMON/ BOTANICAL NAME QUAN. KEY GT Thornless Honeylocust 2 Gleditsia 'Skyline'

8	UA	Accolade Elm
		Ulmus parviflora 'Morton'
16	PA7	Norway Spruce <i>Picea abies</i>

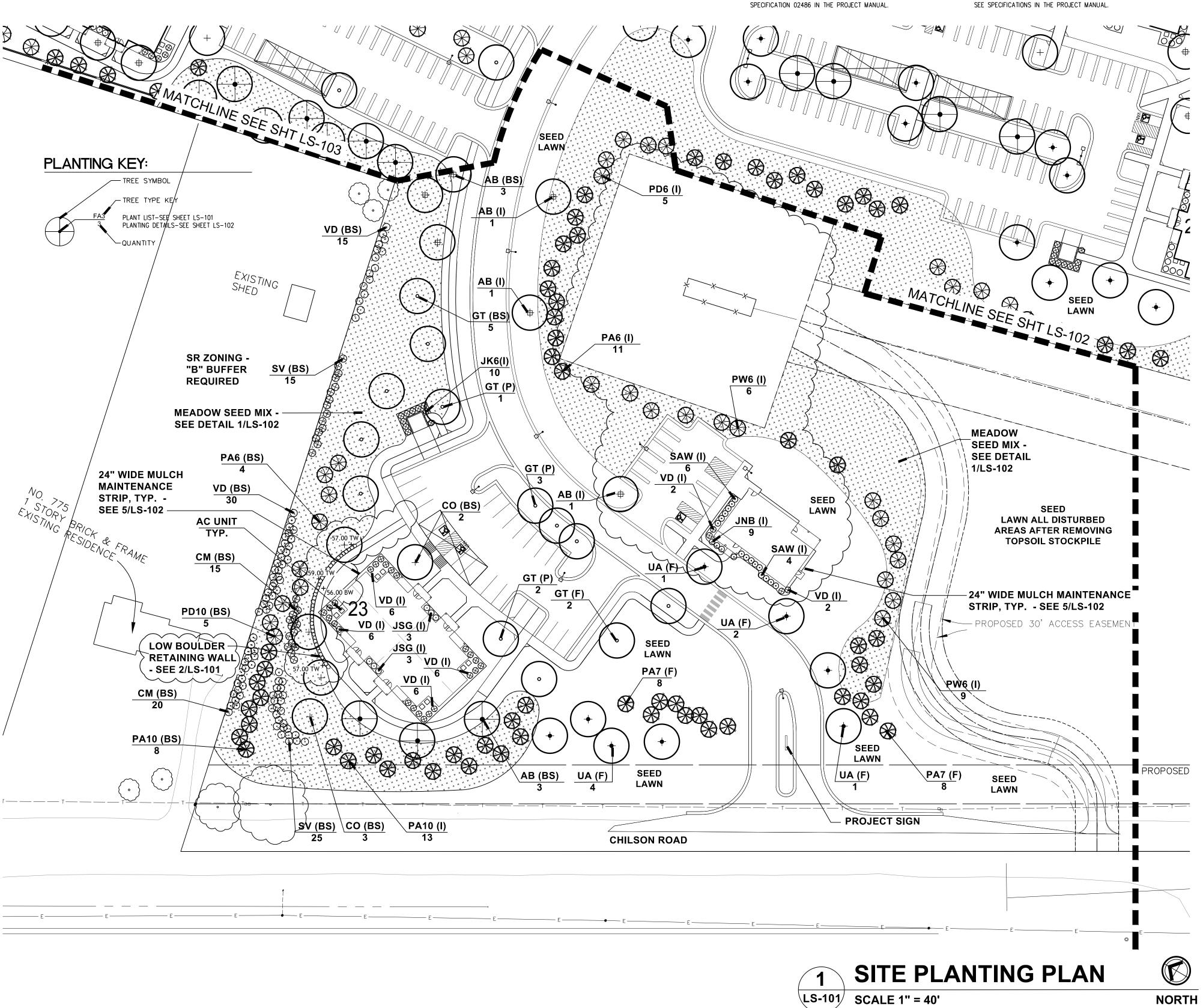


# PLANT LIST - (I) INTERIOR

<u>QUAN.</u>	<u>KEY</u>	COMMON/ BOTANICAL NAME	<u>SIZE</u>	<u>SPEC.</u>
11	AB	Autumn Blaze Maple A <i>cer x. fremanii 'Autumn Blaz</i> e'	3" Cal.	B&B
4	AB12	Autumn Blaze Maple - clump A <i>c</i> er x. fremanii 'Autumn Blaze'	12' Ht. 4 stem mi	B&B in
9	со	Hackberry Celtis occidentalis	3" Cal.	B&B
34	GT	Thornless Honeylocust Gleditsia 'Skyline'	3" Cal.	B&B
10	LT	Tulip Tree <i>Liriod</i> endron tulipifera	3" Cal.	B&B
7	UA	Accolade Elm <i>Ulmus parviflora 'Morton'</i>	3" Cal	B&B
5	UR	Regal Elm <i>Ulmus carpinifolia 'Regal'</i>	3" Cal	B&B
29	PA6	Norway Spruce Picea Abies	6' Ht.	B&B
13	PA10	Norway Spruce Picea Abies	10' Ht.	B&B
27	PD6	Black Hill Spruce <i>Picea glauca var. densata</i>	6' Ht.	B&B
25	PW6	White Spruce Picea glauca	6' Ht.	B&B
122	CS	Redtwig Dogwood Cornus sericea	36" Ht.	BB
34	FI	Forsythia Forsythia x. intermedia	36" Ht.	B&B
57	JK6	Ketler Juniper <i>J. 'Ketlerii'</i>	6' Ht.	B&B
66	JSG	Sea Green Juniper <i>Junip</i> erus 'Sea Green'	24" Spr.	Cont.
24	LA	Amur Privet <i>Ligustrum amurense</i>	36" Ht. Full	B&B
52	SAW	Anthony Waterer Spirea Spirea 'Anthony Waterer'	24" Ht.	Cont.
9	JNB	New Blue Tams Juniper Juniperus t. 'New Blue'	24" Spr.	Cont.
169	VD (	Arrowwood Viburnum Viburnum dentatum	36" Ht.	Cont.







### PLANT LIST - (D) DETENTION BASINS KEY QUAN.

6

14

150

90

70

SIZE SPEC.

3" Cal. B&B

3" Cal B&B

7'Ht. B&B

<u>KEY</u>	COMMON/ BOTANICAL NAME	SIZE	SPEC.
AB12	Autumn Blaze Maple - clump Acer x. fremanii 'Autumn Blaze'	12' Ht. 4 stem m	B&B in
GD	Kentucky Coffee Tree - fully branched <i>Gymnocladus dioicus</i>	3" Cal.	B&B
GT	Thornless Honeylocust <i>Gleditsia 'Skylin</i> e'	3" Cal.	B&B
QB	Swamp White Red Oak <i>Quercus bicolor</i>	3" Cal.	B&B
СВ	Buttonbush Cephalacanthus occidentalis	36" Ht.	B&B
CR	Grey Dogwood Cornus racemosa	36" Ht.	B&B
SC	American Elderberry Sambucus canadensis	36" Ht.	B&B

PLANT MIX

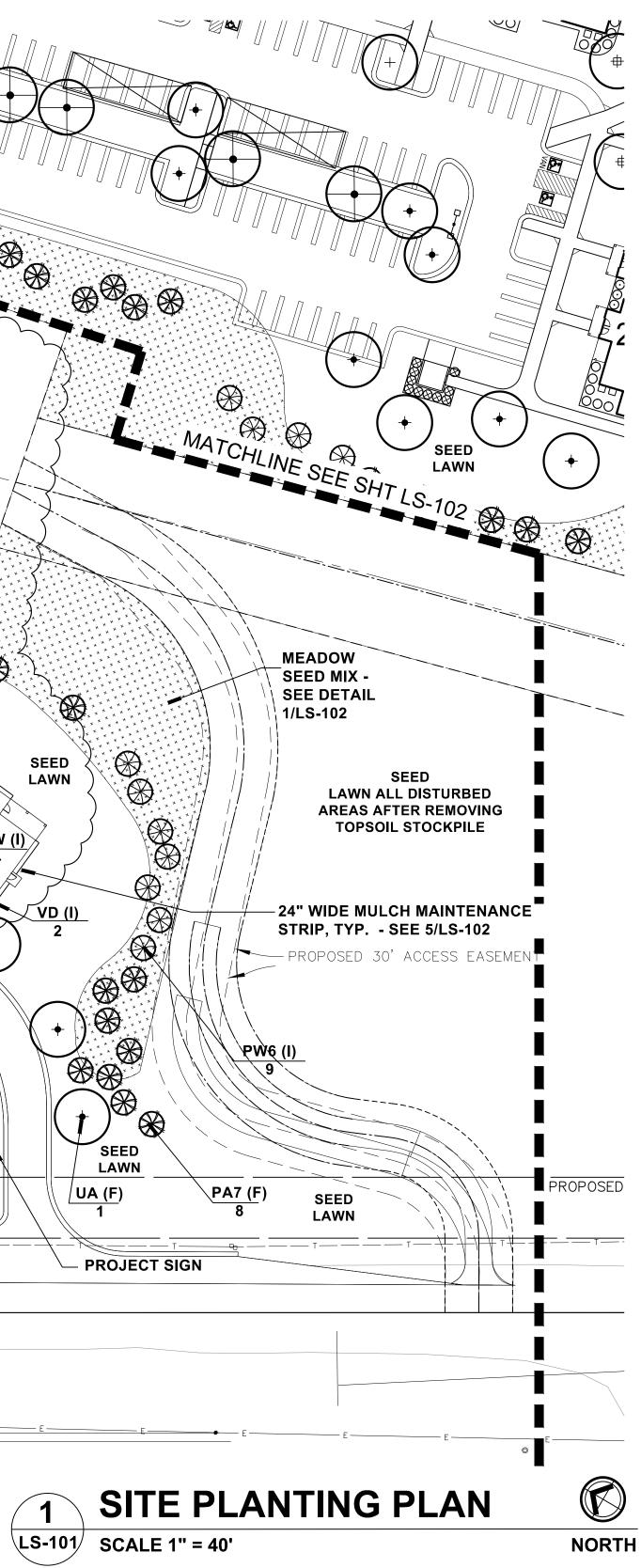
HAND TILL INTO PLACED PLANT MIX: PER 100 SQ FT BED AREA.

PLANT BEDS

MULCH

NO GROUND WOOD PALETTE MULCH PERMITTED

LAWNS:



PLANTING BEDS TO RECEIVE 70% LOAM TOPSOIL, 10% COMPOST, 20% SAND; EXCAVATE PLANT BED, DISPOSE OF SPOILS OFF SITE, INSTALL PLANT MIX

(1) 6 CU. FT. BALE CANADIAN PEAT

(1) 40 LB BAG COMPOSTED POULTRY MANURE "CHICK MAGIC" 5-3-2 WWW.CHICKMAGIC.NET (262)495-6220 (1) 10 LB BAG SHEMINS 13-13-13 MULTI PURPOSE FERTILIZER

HAND TILL INTO PROVIDED PLANT MIX TO A DEPTH OF 12" MINIMUM

ALL PLANT BEDS TO BE FULLY EXCAVATED TO DEPTH SHOWN ON DETAILS AND AREAS SHOWN ON PLANS, AND TO RECEIVE CONTINUOUS PLANT MIX AS SPECIFIED (<u>NOT</u> INDIVIDUAL PLANT HOLES)

MULCH TO BE DOUBLE SHREDDED HARDWOOD BARK MULCH

ALL PROPOSED LAWN AREAS TO BE NON IRRIGATED SEED - SEE

## TOPSOIL

CONTRACTOR TO TILL OR DISK SUBGRADE TO 6" DEPTH AND INSTALL 4" COMPACTED DEPTH TOPSOIL IN ALL LAWN AREAS - FROM ONSITE STOCKPILE OR PROVIDED TO COMPLETE THE PROJECT

# LANDSCAPE EDGING

ALL LANDSCAPE EDGES ARE SHOVEL CUT

# PLANT SPACING

FILL BED WITH PLANTS SPECIFIED. KEEP PLANTS AWAY FROM BUILDING 12" MATURE SIZE

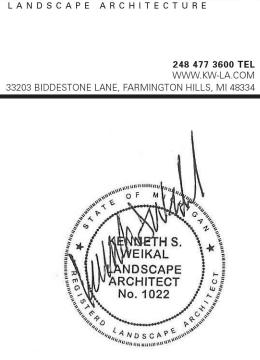
# WATERING

CONTRACTOR RESPONSIBLE FOR WATERING ALL PLANTINGS THROUGH THE CONSTRUCTION PERIOD, AND FOR ONE YEAR FROM THE START OF THE WARRANTY PERIOD.

PLANTINGS THAT PERISH DUE TO LACK OF WATER/ TOO MUCH WATER DO NOT QUALIFY AS THE ONE REQUIRED REPLACEMENT PLANT AS STATED IN THE SPECIFICATION, AND SHALL BE REPLACED.

CONTRACTOR IS ALSO RESPONSIBLE FOR WATERING ALL NEWLY PLANTED LAWN AREAS THROUGH THE CONSTRUCTION PERIOD AND FOR ONE YEAR FROM THE START OF THE WARRANTY PERIOD. NEWLY PLANTED LAWN AREAS THAT PERISH DUE TO LACK OF WATER/ TOO MUCH WATER DO NOT QUALIFY AS THE REQUIRED REPLACEMENT TO ESTABLISH A HEALTHY FULL DENSE LAWN AS STATED IN THE SPECIFICATION, AND SHALL BE REPLACED.

SEE SPECIFICATIONS IN THE PROJECT MANUAL.



HAGENBUCH WEIKA

# SEED MIX NOTES

- 1. THE SEED MIXES SHALL BE APPLIED AT THE SPECIFIED RATE OF FOR EACH MIX.
- MUST BE INSTALLED TO MANUFACTURER SPECIFICATION AND REQUIREMENTS.

MANUFACTURER:	CARDNO NATIVE F 605 SOUTH MAIN ANN ARBOR, MICF 734–222–9690	STREET, #1
		SEE SHEET LS-10

SEED MIX	KEY:	SEE SHEET LS-102 FOR MIX INFORMATION
* * * * * * * * * * * * * * * * * * *		W SEED MIX SION FABRIC - SEE 1/LS-102
	STORM WITH ERO	WATER SEED MIX SION FABRIC - SEE 1/LS-102
· · · · · · · · · · · · · · · · · · ·		BOTTOM SEED MIX SION FABRIC - SEE 1/LS-102

LOCKWOOD COMPANIES 27777 FRANKLIN ROAD **SUITE 1410** SOUTHFIELD, MI 48304

# LAKESHORE VILLAGE **APARTMENTS** PHASE 3

# GENOA TWP., MI

LANDSCAPE CONSTRUCTION DOCUMENTS

SHEET

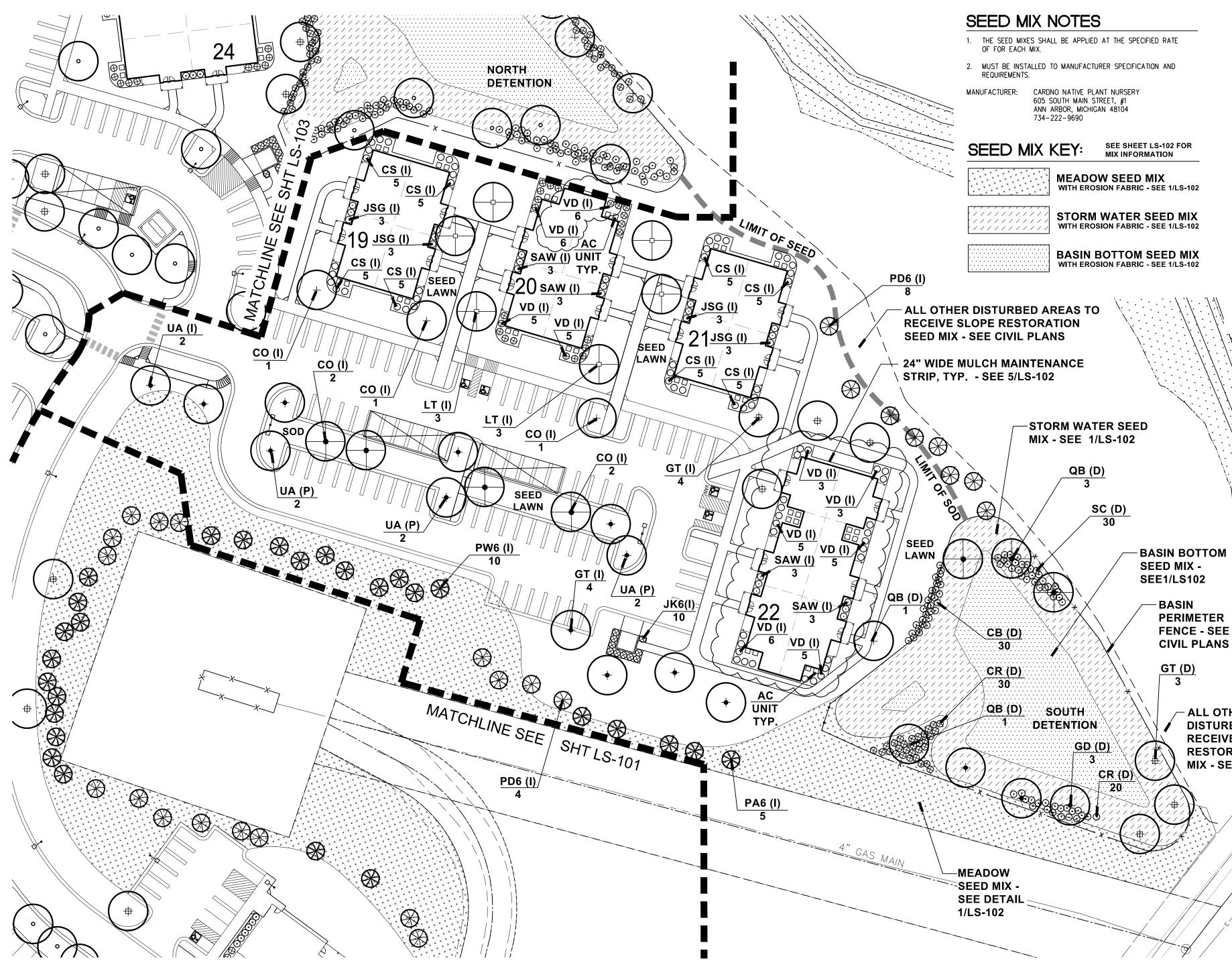
SITE **PLANTING PLAN** 

PRELIMINARY DATE	
2016-02-02	SPA
2016-02-24	SPA

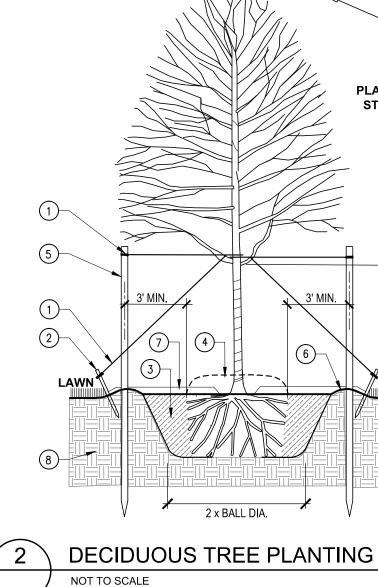
<b>ISSUE DATE</b>	
2016-03-18	CD
<b>REVISION DA</b>	TE
2016-05-09	SPA
2016-06-28	REV.
2016-08-18	REVISED
2016-09-20	REVISED
2016-12-05	PERMIT
2018-02-28	TWP. REV.
SHEET NUME	BER
	_S-101

STORMWATER SEED	MIX			
Botanical Name	<u>Common Name</u>	Ounces/Acre	Seeds/Oz	Seeds/SQ FT
Permanent Grasses/Sedg	es/Rushes:			
Carex crisatella	Crested Oval Sedge	1.00	59000	1.35
Carex Iurida	Bottlebrush Sedge	2.00	12000	0.55
Carex vulpinoidea	Brown Fox Sedge	6.00	125000	17.22
Elymus virginicus	Virginia Wild Rye	12.00	4375	1.21
Glyceria striata	Fowl Manna Grass	1.25	125000	3.59
Juncus effusus	Common Rush	1.00	281000	6.45
Juncus torreyi	Torrey's Rush	0.25	1134000	6.51
Leersia oryzoides	Rice Cut Grass	1.00	94500	2.17
Panicum virgatum	Switch Grass	8.00	28356	5.21
Scirpus atrovirens	Dark Green Rush	1.00	187500	4.30
Scirpus cypernus	Wool Grass	0.50	562500	6.46
Scirpus fluviatilis	River Bulrush	0.25	27500	0.16
Scirpus validus	Great Bulrush	6.00	37813	5.21
-	Total	40.25		60.38
Temporary Cover:				
Avena sativa	Common Oat	360.00	8125	67.15
Lolium multiflorum	Annual Rye	100.00	14188	32.57
	Total	460.00		99.72
Forbs & Shrubs:				
Alisma spp.	Water Plantain (Various Mix)	4.25	70175	6.85
Asclepias incarnata	Swamp Milkweed	1.50	4540	0.16
Bidens spp.	Bidens (Various Mix)	2.00	14175	0.65
Helenium autumnale	Sneezeweed	2.00	141750	6.51
Lycopus americanus	Common Water Horehound	0.25	235000	1.35
Mimulus ringens	Monkey Flower	1.00	283500	6.51
Penthorum sedoides	Ditch Stonecrop	0.50	36063	0.41
Polygonum pensylvanicum	Pinkweed	4.00	4063	0.37
Rudbeckia subtomentosa	Sweet Black-Eyed Susan	1.00	46000	1.06
Sagittaria latifolia	Common Arrowhead	1.00	56700	1.30
Senna hebacarpa	Wild Senna	1.00	1400	0.03
Thalictrum dasycarpum	Purple Meadow Rue	2.00	13500	0.62
	Total	20.50		25.82

MEADOW SEED MIX				
Botanical Name	Common Name	Ounces/Acre	Seeds/Oz	Seeds/SQ FT
Permanent Grasses:				
Bouteloua curtipendula	Side Oats Grama	10.00	9375	2.15
Carex spp.	Prairie Carex Mix	4.00	33422	3.07
Elymus canadensis	Canada Wild Rye	32.00	4258	3.13
Koeleria cristata	June Grass	1.00	150000	3.44
Panicum virgatum	Switch Grass	1.00	28356	0.65
Schizachyrium scoparium	Little Bluestem	32.00	8800	6.46
	Total	80.00		18.91
Temporary Cover:				
Avena sativa	Common Oat	360.00	8125	67.15
Lolium multiflorum	Annual Rye	100.00	14188	32.57
	Total	460.00		99.72
Forbs:				
Anemone cylindrica	ThimbleWeed	0.50	20938	0.24
Asclepias tuberosa	Butterfly MilkWeed	2.00	3500	0.16
Aster ericoides	Heath Aster	0.25	140000	0.80
Aster laevis	Smooth Blue Aster	0.75	48000	0.83
Aster novae-angliae	New England Aster	0.25	76000	0.44
Baptisia lactea	White Wild Indigo	2.00	1600	0.07
Chamaecrista fasciculata	Partridge Pea	14.00	3800	1.22
Coreopsis lanceolata	Sand Coreopsis	5.00	12500	1.43
Coreopsis palmata	Prairie Coreopsis	1.00	11500	0.26
Dalea candida	White Prairie Clover	1.50	26250	0.90
Dalea purpurea	Purple Prairie Clover	1.50	20000	0.69
Echinacea purpurea	Broad-Leaved Purple Coneflov	7.00	6600	1.06
Eryngium yuccifolium	Rattlesnake Master	2.50	8000	0.46
Lespedeza capitata	Round-Head Bush Clover	2.00	10000	0.46
Liatris aspera	Rough Blazing Star	0.50	13000	0.15
Lupinus perennis	Wild Lupine	2.00	1000	0.05
Monarda fistulosa	Wild Bergamot	0.75	78000	1.34
Parthenium integrifolium	Wild Quinine	1.00	6800	0.16
Penstemon digitalis	Fox glove Beard Tongue	0.50	115000	1.32
Pycnanthemum virginianum		1.00	331250	7.60
Ratibida pinnata	Yellow Coneflower	4.00	25250	2.32
Rudbeckia hirta	Black-Eyed Susan	5.00	110000	12.63
Rudbeckia subtomentosa	Sweet Black-Eyed Susan	1.00	46000	1.06
Silphium integrifolium	Rosin Weed	3.00	4000	0.28
Silphium terebinthinaceum	Prairie Dock 0.50 1100		0.01	
Solidago nemoralis	Old-Field Goldenrod 0.50		240000	2.75
Solidago rigida	Stiff Goldenrod	1.00	46000	1.06
Tradescantia ohiensis	Common Spiderwort	0.75	8000	0.14
Vernonia spp.	Ironweed (Various Mix)	1.75	24000	0.96
Veronicastrum virginianum	Culvers Root	0.25	750000	4.30
3	Total	63.75		45.16



BASIN BOTTOM SEED MIX	Common Name	Quinas a/A ara	Seeds/Oz	Seeds/SQ FT
Botanical Name	Common Name	Ounces/Acre	36609/02	Seeus/SQ FI
Permanent Grasses/Sedges:				
Andropogon gerardii	Big Bluestem	4.00	8188	0.75
Carex comosa	Bristly Sedge	2.50	41183	2.36
Carex cristatella	Crested Oval Sedge	2.00	59000	2.71
Carex Iurida	Bottlebrush Sedge	2.50	12000	0.69
Carex spp.	Prairie Sedge Mix	8.00	33422	6.14
Carex vulpinoidea	Brown Fox Sedge	4.00	125000	11.48
Elymus virginicus	Virginia Wild Rye	8.00	4375	0.80
Glyceria striata	Fowl Manna Grass	1.00	125000	2.87
Panicum virgatum	Switch Grass	2.00	28356	1.30
Scirpus atrovirens	Dark Green Rush	2.00	187500	8.61
Scirpus cyperinus	Wool Grass	1.00	562500	12.91
Spartina pectinata	Prairie Cord Grass	3.00	15750	1.08
	Total	40.00		51.71
Temporary Cover:				
Avena sativa	Common Oat	360.00	8125	67.15
Lolium multiflorum	Annual Rye	100.00	14188	32.57
	Total	460.00		99.72
Forbs:				
Alisma spp.	Water Plantain (Various Mix)	1.00	70175	1.61
Asclepias incarnata	Swamp Milkweed	2.00	4540	0.21
Coreopsis tripteris	Tall Coreopsis	1.00	11500	0.26
Eutrochium maculatum	Spotted Joe-Pye Weed	0.25	78125	0.45
Iris virginica	Blue Flag	4.00	1400	0.13
Liatris spicata	Marsh Blazing Star	1.00	12000	0.28
Lobelia cardinalis	Cardinal Flower	0.25	437000	2.51
Lobelia siphilitica	Great Blue Lobelia	0.50	520000	5.97
Lycopus americanus	Common Water Horehound	0.25	235000	1.35
Pycnanthemum virginianum	Common Mountian Mint	0.50	331250	3.80
Rudbeckia triloba	Brown-Eyed Susan	0.50	33000	0.38
Sagittaria latifolia	Common Arrowhead	0.25	56700	0.33
Senna hebecarpa	Wild Senna	1.00	1400	0.03
Silphium terebinthinaceum	Prairie Dock	1.00	1100	
Symphyotrichum novae-angliae		1.00	76000	
Verbena hastata	Blue Vervain	1.50	125000	
Zizia aurea	Golden Alexanders	0.75	12000	



23.37

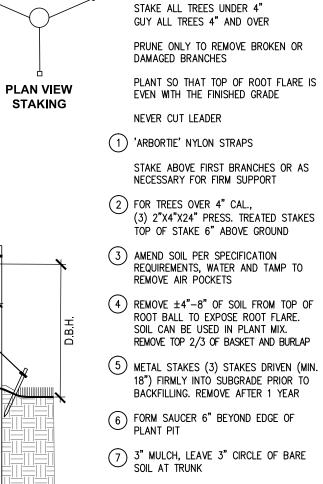


DETENTION BASIN AND MEADOW SEED MIXES ALL SEED MIX AREAS TO HAVE EROSION MAT

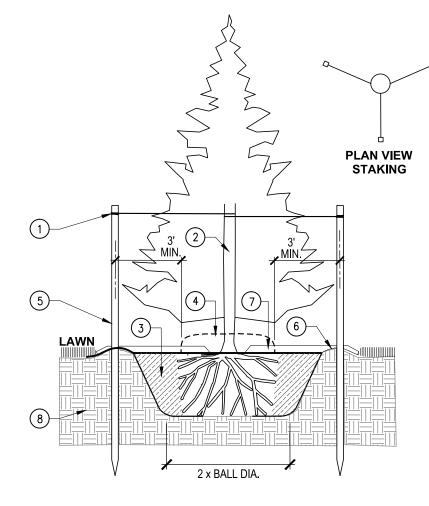
Total

16.75

SEE 1/LS-103



8 UNDISTURBED SUBSOIL



- REMOVE ALL TAGS, STRINGS, PLASTICS AND OTHER MATERIALS WHICH ARE UNSIGHTLY OR COULD CAUSE GIRDLING PLANT SO THAT TOP OF ROOT FLARE IS EVEN WITH THE FINISHED GRADE
- STAKE ALL EVERGREEN TREES UNDER 12'HT.
- GUY ALL EVERGREEN TREES 12' HT. AND OVER
- (1) 'ARBORTIE' NYLON STRAPS 2 NEVER CUT LEADER
- (3) Amend Soil PER SPECIFICATION REQUIREMENTS, WATER AND TAMP TO REMOVE AIR POCKETS
- (4) REMOVE ±4"-8" OF SOIL FROM TOP OF ROOT BALL TO EXPOSE ROOT FLARE. SOIL CAN BE USED IN PLANT MIX. REMOVE TOP 2/3 OF BASKET AND BURLAP
- (5) METAL STAKES (3) STAKES DRIVEN (MIN. 18") FIRMLY INTO SUBGRADE PRIOR TO BACKFILLING. REMOVE AFTER
- 1 YEAR 6 FORM SOIL SAUCER 6" BEYOND EDGE
- OF PLANT PIT
- 3" MULCH, LEAVE 3" CIRCLE OF BARE SOIL AT TRUNK
- 8 UNDISTURBED SUBSOIL
- EVERGREEN TREE PLANTING NOT TO SCALE

CARDNO NATIVE PLANT NURSERY 605 SOUTH MAIN STREET, #1 ANN ARBOR, MICHIGAN 48104 734-222-9690			
XK	(EY:	SEE SHEE MIX INFOR	T LS-102 FOR
* * * * * * * * * * *		W SEED	<b>MIX</b> C - SEE 1/LS-102
/ / / / / /			SEED MIX
			SEED MIX - SEE 1/LS-102

SEE1/LS102 BASIN PERIMETER FENCE - SEE **CIVIL PLANS** <u>GT (D)</u>

3

- ALL OTHER DISTURBED AREAS TO **RECEIVE SLOPE RESTORATION SEED** MIX - SEE CIVIL PLANS

> Ź PIØ ZONING -NO BUFFER REQUIRED

GENERAL PLANTING NOTES:

(A) ALL TREES TO HAVE CLAY OR LOAM BALLS, TREES WITH SAND

B ALL SINGLE STEM SHADE TREES TO HAVE STRAIGHT TRUNKS AND SYMMETRICAL CROWNS.

C ALL SINGLE TRUNK SHADE TREES TO HAVE A CENTRAL LEADER, TREES WITH FORKED OR IRREGULAR TRUNKS <u>WILL NOT BE</u>

(D) ALL MULTI-STEM TREES SHALL BE HEAVILY BRANCHED AND HAVE SYMMETRICAL CROWNS. ONE SIDED TREES OR THOSE WITH THIN

(E) ALL EVERGREEN TREES SHALL BE HEAVILY BRANCHED AND FULL TO THE GROUND, SYMMETRICAL IN SHAPE AND NOT SHEARED

F NO MACHINERY IS TO BE USED WITHIN THE DRIPLINE OF EXISTING TREES. HAND GRADE ALL LAWN AREAS WITHIN DRIPLINE OF EXISTING TREES.

CONTRACTOR AND ARE SUBJECT TO THE APPROVAL OF THE LANDSCAPE ARCHITECT PRIOR TO INSTALLATION OF THE PLANT

(H) IT IS MANDATORY THAT POSITIVE DRAINAGE IS PROVIDED AWAY FROM ALL BUILDINGS, WALKS AND PAVED AREAS.

() ALL PLANTING BEDS SHALL RECEIVE 3" SHREDDED BARK MULCH.

(G) ALL TREE LOCATIONS SHALL BE STAKED BY LANDSCAPE

OR OPEN CROWNS SHALL NOT BE ACCEPTED.

FOR THE LAST FIVE GROWING SEASONS.

BALLS <u>WILL BE REJECTED.</u>

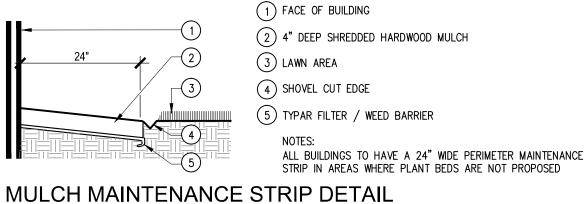
<u>ACCEPTED.</u>

MATERIAL.

SEE SPECIFICATIONS.

SITE PLANTING PLAN

NORTH



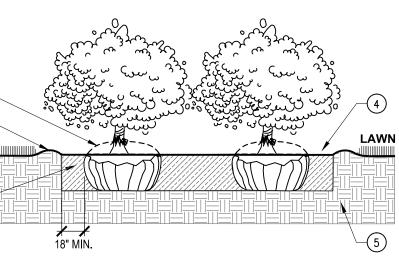
NOT TO SCALE

5

LS-101/

(2)

SCALE 1" = 40'



SHRUB BED PLANTING DETAIL NOT TO SCALE

(1) REMOVE TOP 1/3 OF BURLAP FROM ROOTBALL 2 MOUND TO FORM SAUCER FORM SOIL SAUCER WITH 3" HT. CONTINUOUS RIM

(3) SPECIFIED PLANTING MIX – WATER & TAMP TO REMOVE AIR POCKETS – 12" MIN. DEPTH OR DEPTH OF ROOT BALL.

(4) 3" DEPTH MULCH – 6" BARE AT TRUNK – DO NOT BURY CROWN OF PLANT 5) UNDISTURBED SUBSOIL

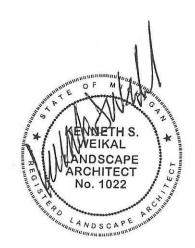
NOTES: PRUNE BRANCHES TO THIN OR MATCH, RETAIN NORMAL PLANT SHAPE. PRUNE DEAD BRANCHES

SHRUBS SHALL BEAR THE SAME RELATIONSHIP TO FINISH GRADE AS THEY BORE TO ORIGINAL GRADE.

REMOVE ALL FIBER, PLASTIC OR METAL CONTAINERS.



248 477 3600 TEL WWW.KW-LA.COM 33203 BIDDESTONE LANE, FARMINGTON HILLS, MI 48334



LOCKWOOD COMPANIES 27777 FRANKLIN ROAD SUITE 1410 SOUTHFIELD, MI 48304

# LAKESHORE VILLAGE **APARTMENTS** PHASE 3

**GENOA TWP., MI** 

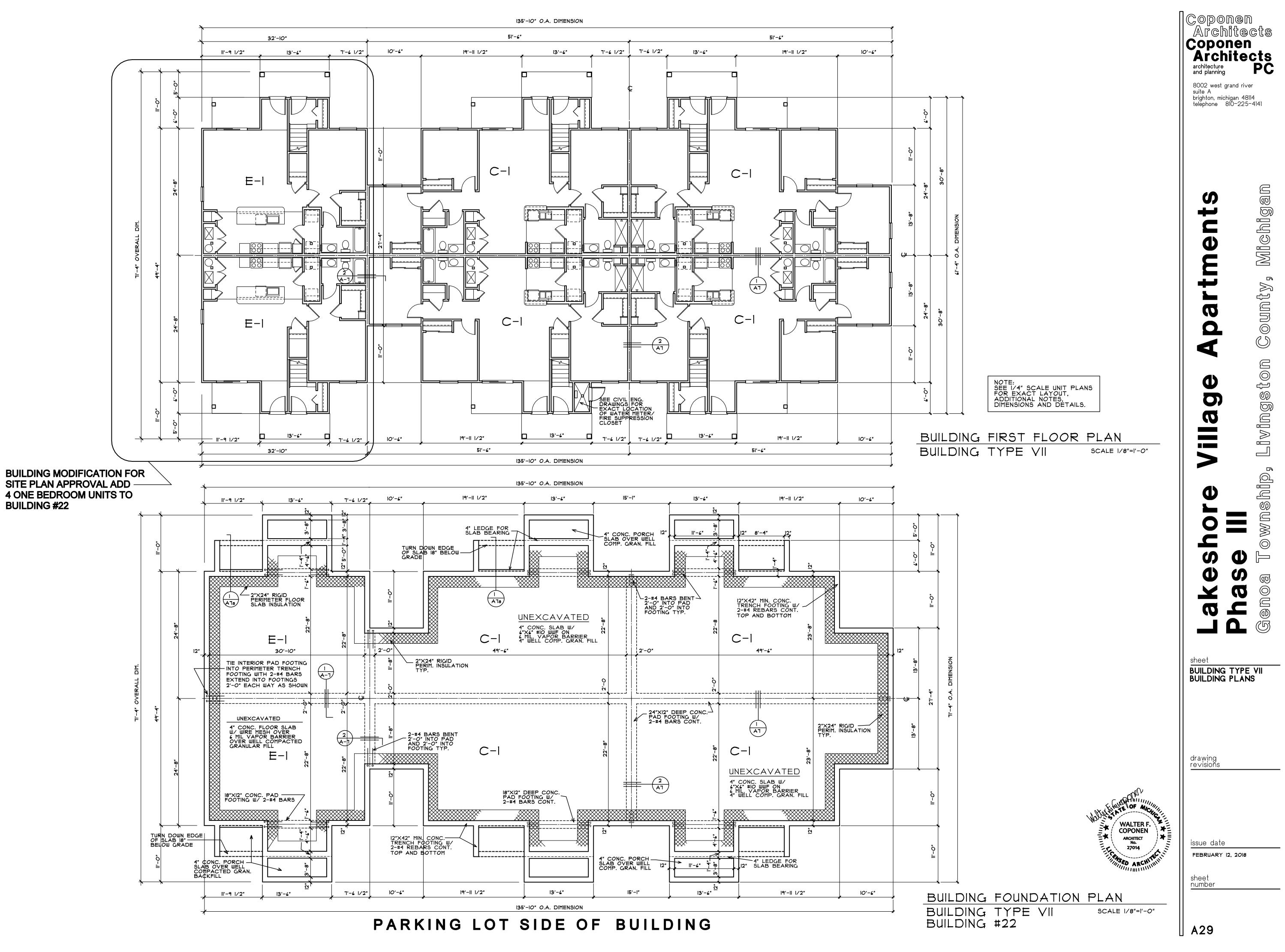
LANDSCAPE CONSTRUCTION DOCUMENTS

SHEET

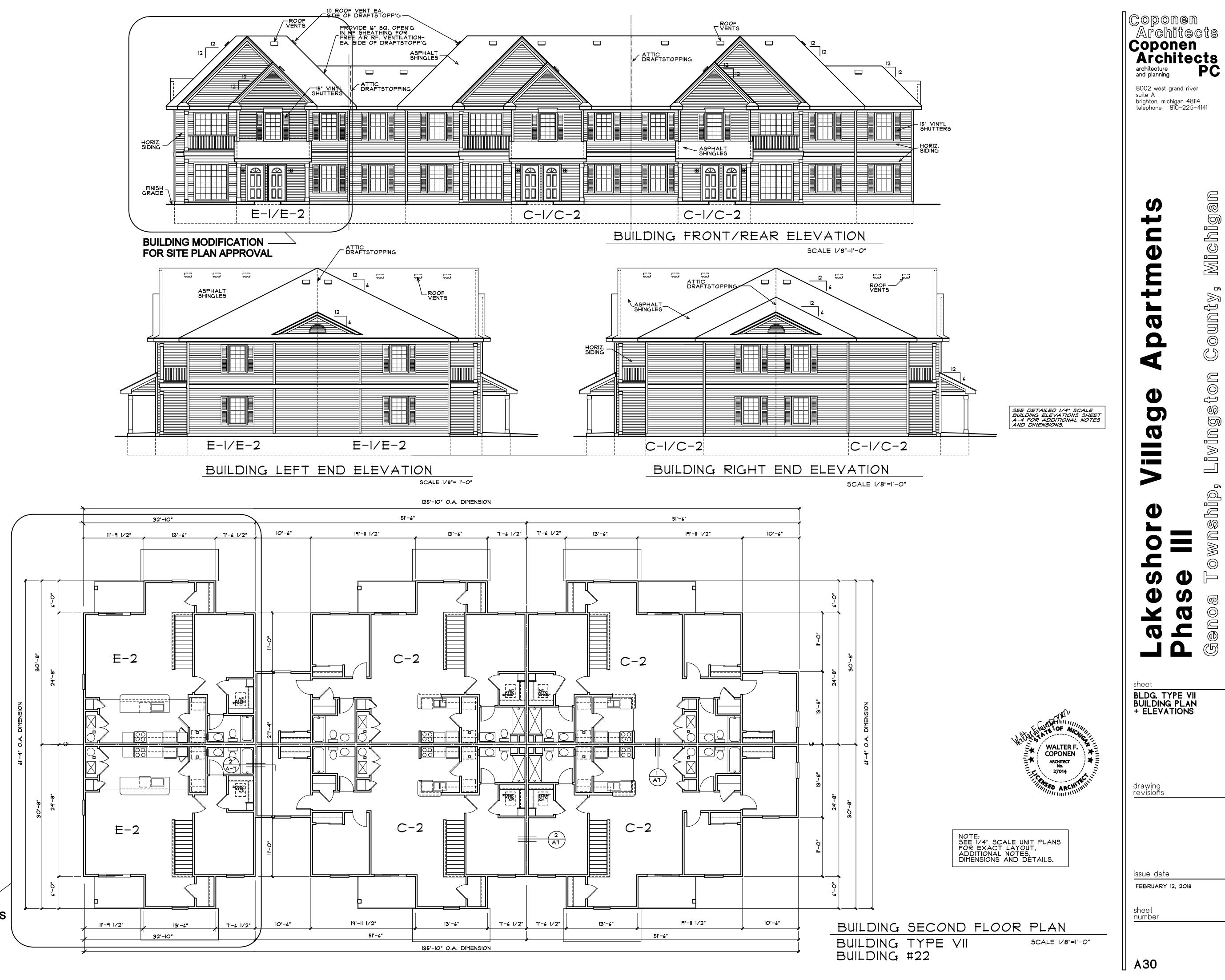
# SITE **PLANTING PLAN**

PRELIMINARY DATE		
2016-02-02	SPA	
2016-02-24	SPA	

ISSUE DATE	
2016-03-18	CD
<b>REVISION D</b>	ATE
2016-05-09 2016-06-28 2016-08-18 2016-09-20 2016-12-05 2018-02-28	SPA REV. REVISED REVISED PERMIT TWP. REV.
SHEET NUM	BER
	LS-102







**BUILDING MODIFICATION** FOR SITE PLAN APPROVAL ADD 4 ONE BEDROOM UNITS TO BUILDING #22

#### GENOA CHARTER TOWNSHIP PLANNING COMMISSION PUBLIC HEARING FEBRUARY 12, 2018 6:30 P.M. MINUTES

<u>CALL TO ORDER</u>: The meeting of the Genoa Charter Township Planning Commission was called to order at 6:30 p.m. Present were Chairman Doug Brown, Jim Mortensen, Eric Rauch, Chris Grajek, John McManus, and Marianne McCreary. Absent was Jill Rickard. Also present was Kelly VanMarter, Community Development Director/Assistant Township Manager, Brian Borden of LSL Planning, Gary Markstrom of Tetra Tech, and an audience of 21.

<u>PLEDGE OF ALLEGIANCE</u>: The pledge of allegiance was recited.

#### **ELECTION OF OFFICERS:**

**Moved** by Commissioner Grajek, seconded by Commissioner Mortensen, to nominate Doug Brown for Chairperson, Eric Rauch for Vice Chairperson, and Marianne McCreary for Secretary. **The motion carried unanimously**.

#### APPROVAL OF AGENDA:

**Moved** by Commissioner McManus, seconded by Commissioner Grajek, to approve the agenda as presented.

CALL TO THE PUBLIC: The call to the public was made at 6:34 pm with no response.

OPEN PUBLIC HEARING #1... REQUEST TO POSTPONE INDEFINITELY (due to failure to submit) review of a special use, site plan and environmental impact assessment for a proposed used automobile sale facility located at 3439 E. Grand River Avenue, Howell 48843. The request is petitioned by Car Nation, LLC.

Planning Commission Disposition of Petition:

A. Disposition of request to Postpone Indefinitely.

The call to the public was made at 6:35 pm.

Mr. Walt Cieslik of 501 Snowfall Court lives behind the proposed car dealership. He is a board member of his homeowner's association. They want to know what type of lighting and landscaping would be used at the rear of the property. They would like to ensure that all drainage from the car dealership site does not negatively affect their properties.

Mr. Jim Strand of 3445 Dewdrop Lane agrees with what Mr. Cieslik stated. He wants to ensure that there is proper security. Automobile dealerships are notorious for theft.

The call to the public was closed at 6:40 pm.

**Moved** by Commissioner McManus, seconded by Commissioner Grajek to postpone indefinitely review of a special use, site plan and environmental impact assessment for Car Nation, LLC. **The motion carried unanimously.** 

OPEN PUBLIC HEARING #2... Review of a special use, sketch plan and environmental impact assessment for a proposed Four Seasons Veterinary Services veterinary clinic. The property in question is located at 6936 Grand River Avenue, Brighton, 48114. The request is petitioned by Dr. Michelle Bradford, DVM.

Planning Commission Recommendation of Petition:

- A. Recommendation of Special Use Application.
- B. Recommendation of Environmental Impact Assessment (01/22/18).
- C. Recommendation of Sketch Plan.

Dr. Michelle Bradford was present. She would like to build a veterinary clinic in a commercial lease space to offer medical veterinary services. She would not have overnight boarding or grooming at her clinic.

Mr. Borden reviewed his letter dated February 6, 2018.

The applicant has met the requirements of the Special Land Use standards; however, he defers to the Township Engineer and Fire Department regarding any of their concerns.

The applicant has submitted a sketch plan and not a Site Plan. He had requested that the dog walk area be depicted on the plans. The applicant submitted updated plans showing this.

The required tree plantings in the Grand River greenbelt are not shown on the plan. The requirement is six trees. The Planning Commission has the discretion to require this waive the requirement. Mr. Gary Laundroche, who represents the property owner, stated they are aware of this deficiency and assured the Planning Commission that the six trees will be installed in the spring.

Commissioner Mortensen does not want the dog walk in the front of the site along Grand River. Dr. Bradford stated it will not be a dedicated area where dogs will be walked. The area would be used when a dog is coming to or leaving from their appointment. She typically has one to two appointments per hour. She would also use it occasionally for patients after surgery.

Commissioner Grajek is concerned that the grass in this area will die if it is being used by the dogs. He suggested using the rear of the building.

Commissioner Rauch does not believe that the intensity of the use is as high as it is being discussed this evening.

Commissioner Mortensen suggested a hedgerow be placed in the front of the building to screen the dogs from Grand River. Mr. Laundroche agrees to adding the hedgerow for patients coming into the building as well as special grass that is appropriate for areas where dogs use the bathroom. Staff could use the rear of the site for patients they bring outside.

Commissioner McCreary questioned what occurs when a patient needs to spend the night after surgery. Dr. Bradford stated those animals are transferred to an emergency clinic or sent home with the owner.

Mr. Markstrom stated he has determined the discharge from the area of the proposed dog walk does not discharge into the storm sewer. He noted that the signage and clean-up plan noted in the Impact Assessment must be maintained.

Chairman Brown reviewed the Fire Department's letter dated February 1, 2018.

The call to the public was made at 7:12 pm with no response.

**Moved** by Commissioner McManus, seconded by Commissioner Mortensen, to recommend to the Township Board approval of the Special Use Application for Four Seasons Veterinary Services veterinary clinic located at 6936 Grand River Avenue, Brighton, 48114 as The Planning Commission finds that the Special Use Application meets all of the requirements of Section 19.03 of the Township Ordinance, based on the following conditions:

- Six deciduous trees shall to be planted in the spring along Grand River.
- The petitioner will add a hedge row to screen the dogs using the area. The hedge row shall be reviewed and approved by Township staff.

- The petitioner agrees that they will maintain the landscape around the dog walk area to keep the vegetation alive.
- All requirements of the Brighton Area Fire Authority's letter of February 1, 2018 shall be met.
- Township Board approval of the Impact Assessment and sketch plan.

## The motion carried unanimously.

**Moved** by Commissioner McManus, seconded by Commissioner Grajek, to recommend to the Township Board approval of the Environmental Impact Assessment dated January 22, 2018 for Four Seasons Veterinary Services veterinary clinic. The property in question is located at 6936 Grand River Avenue, Brighton, 48114. **The motion carried unanimously.** 

**Moved** by Commissioner McManus, seconded by Commissioner Mortensen, to recommend to the Township Board approval of the Sketch Plan for Four Seasons Veterinary Services veterinary clinic. The property in question is located at 6936 Grand River Avenue, Brighton, 48114, conditioned upon the following:

- The sketch plan is sufficient instead of a Site Plan. This is an existing building with a modest amount of exterior and landscaping changes.
- Six deciduous trees shall to be planted in the spring in the easement along Grand River
- The petitioner will add a hedge row to screen the dogs using the area. The hedge row shall be reviewed and approved by Township staff.
- Township Board approval of the Special Land Use and Impact Assessment. **The motion carried unanimously.**

OPEN PUBLIC HEARING # 3... Review of a special use, site plan and environmental impact assessment for a proposed commercial outdoor display, sales and storage area for a new Family Farm and Home store. The property in question is located in the former TJ Maxx retail space at 3685 E. Grand River Avenue, Howell 48843. The request is petitioned by Family Farm and Home.

Planning Commission Recommendation of Petition:

- A. Recommendation of Special Use Application.
- B. Recommendation of Environmental Impact Assessment (01/04/18).
- C. Recommendation of Site Plan (01/24/18).

Mr. Martin Renel of Ashmark Construction and Robert Kerr of Metro Group Architects were present.

Mr. Renel provided a review of the project and the proposed use. They would be performing minor renovations on the interior of the building. They would be adding an outdoor display area and reconstructing the dumpster enclosure.

Mr. Borden reviewed his letter of February 6, 2018.

They find that the general standards of the Special Land Use have been met.

The screening for the outdoor storage area is required to be six-feet high and the applicant is proposing four-foot high fencing. The Planning Commission can approve the difference.

He is asking for clarification for what the applicant is calling the outdoor staging area. It appears to be an additional outdoor storage area. Mr. Renel stated that since the submittal was made, they have confirmed with Family Farm and Home that this area will be storage so the fencing will need to be extended to include this, which is an additional 19 feet.

There was a discussion regarding the removal of some of the grass area and then the same amount of grass area being replaced by removing parking spaces and adding turf. Commissioner Mortensen questioned if this would affect the parking. Ms. VanMarter stated there is sufficient parking on this site.

Commissioner Mortensen asked that the proposed white PVC fencing being redesigned. Mr. Borden noted that this fencing is also being proposed for the dumpster enclosure area and it is not an approved material. Mr. Eugene Franks of Family Farm and Home stated that there are dumpsters along the back of the building and none of them have enclosures. Mr. Renel stated they could use wood for the enclosure.

Commissioner Rauch asked staff for their experience with other commercial businesses in the Township who have outdoor storage. Ms. VanMarter stated the Township has had continued problems with businesses that have not complied with the ordinance as it pertains to outdoor storage for retail uses. Commissioner Rauch wants to ensure that there is sufficient screening on the front of this building so the same problem does not occur here.

Mr. Borden noted that the architectural plan and the engineering plan are not consistent.

There was a discussion regarding the completion of the cross access driveway to the property to the east. The applicant has declined to finish the driveway to connect the

two properties. Ms. VanMarter stated that the connection is the responsibility of Meijer; however, at the time it was developed, they were not given an easement from Kroger. Mr. Markstrom stated there is a large grade change in this area so significant work would need to be done to connect these two properties.

Mr. Markstrom reviewed his letter dated January 30, 2018. He would like to see flow direction shown for the storm-water runoff. The trade-off of the paving and turf is a viable alternative; however, he suggested the applicant look at the entire site it could possibly be determined that there may be sufficient impervious surface so there may not be a need to remove the parking spaces.

He needs to see a complete site plan with details, such as foundation details, curbing details, etc.

Chairman Brown reviewed the Brighton Area Fire Authority letter dated February 1, 2018. The applicant has agreed to meet all of their requirements.

The call to the public was made at 8:08 pm.

Mr. Eric Unatin with Mid-American Real Estate has been working on this lease. Their experience over the past 18 months since this site has been vacant has been that Family Farm and Home has been the only company interested. He noted the discussion regarding the material for the fencing and the dumpster enclosure and stated there are costs associated for both the property owner and the tenant when a new owner moves into a space and it must be economical for both of them.

The call to the public was closed at 8:12 pm.

Commissioner McManus agrees with what Mr. Unatin said; however, the Planning Commission needs to ensure that the ordinance is met. He agrees that the proposed fencing needs to be upgraded.

**Moved** by Commissioner Mortensen, seconded by Commissioner Rauch, table the request for a special use, site plan and environmental impact assessment for a proposed commercial outdoor display, sales and storage area for a new Family Farm and Home store located at 3685 E. Grand River Avenue, Howell 48843 until the March 12, 2018 meeting.

## The motion carried unanimously.

OPEN PUBLIC HEARING # 4... Review for preliminary and final site condominium approval for a proposed 6 unit site condominium. The property in question is located on the east side of the Latson Road and Grand Oaks Drive intersection, Howell 48843. The request is petitioned by St. John Providence.

Planning Commission Recommendation of Petition:

- A. Recommendation of Environmental Impact Assessment (01/04/18).
- B. Recommendation of Preliminary site condominium.
- C. Recommendation of Final site condominium.

Mr. Mark Yagerlender, Regional Director of Real Estate for St. John Providence, was present. Their new location has been successful and they are now requesting approval for their site condominium, which will allow them to develop the outlots on their site.

Mr. Borden reviewed his letter of February 7, 2018. The applicant is requesting to develop a site condominium complex. All of the required documents and plans have been submitted so he is recommending that the preliminary and final site plans be approved this evening.

There was a discussion between the Commission and the applicant's engineers regarding how each of the outlots would be developed, connecting roadways, the ownership and maintenance of the interior roadways, etc. It was noted that Flagstar Bank has already been approved. Each individual site will be before the Planning Commission and Township Board for approval.

Mr. Markstrom reviewed his letter dated January 29, 2018. He noted that when each individual site is to be developed, there will be engineering reviews. This may require that the buildings, etc. to be redesigned.

The call to the public was made at 9:15 pm with no response.

**Moved** by Commissioner Mortensen, seconded by Commissioner Grajek, to recommend to the Township Board approval of Environmental Impact Assessment dated January 4, 2018 for a six-unit site condominium for St. John Providence located on the east side of the Latson Road and Grand Oaks Drive intersection, Howell 48843. **The motion carried unanimously**.

**Moved** by Commissioner Mortensen, seconded by Commissioner McManus, to recommend to the Township Board approval of the Preliminary Site Condominium dated January 23, 2018 for a six-unit site condominium for St. John Providence located on the

east side of the Latson Road and Grand Oaks Drive intersection, Howell 48843 with the following conditions:

- Approval of the Master Deed by the Township Attorney
- Acknowledgement by the petitioner that the Master Deed for St. John Providence Site Condominium does not supersede the Planned Unit Development or the Genoa Township Zoning Ordinance.

### The motion carried unanimously.

**Moved** by Commissioner Mortensen, seconded by Commissioner Grajek, to recommend to the Township Board approval of the Final Site Plan dated January 23, 2018 for a sixunit site condominium for St. John Providence located on the east side of the Latson Road and Grand Oaks Drive intersection, Howell 48843 with the following conditions:

- Approval of the Master Deed by the Township Attorney
- Acknowledgement by the petitioner that the Master Deed for St. John Providence Site Condominium does not supersede the Planned Unit Development or the Genoa Township Zoning Ordinance.

### The motion carried unanimously.

## OPEN PUBLIC HEARING #5...Consideration of Zoning Ordinance Text amendments to Article 10 of the Zoning Ordinance, entitled "Planned Unit Development" is proposed to be amended to add standards related to "Interchange Commercial PUD" and "Interchange Campus PUD".

Planning Commission Recommendation of Petition:

A. Recommendation of Zoning Ordinance Text Amendment.

Ms. VanMarter provided details of the proposed text amendments to Article 10 of the Township's Zoning Ordinance. She showed maps that depicted the locations of the proposed changes. She and the engineers have developed the plan for the water and sewer services for this area. These changes are needed because to the continued demand due to the installation of the Latson Road / I-96 Interchange.

Mr. Todd Wyatt, who owns 200 acres of property in this area, stated that he is interested in developing the property with low-intensity industrial uses. He has been assisting with the development of the text amendments. He would like to show the Planning Commission some proposed designs.

The Planning Commission is interested in seeing Mr. Wyatt's ideas for the area. It was suggested to have him return to a future meeting.

The call to the public was made at 10:08 pm with no response.

**Moved** by Commissioner McManus, seconded by Commissioner Mortensen, to table the consideration of Zoning Ordinance Text amendments to Article 10 of the Zoning Ordinance to allow staff and the Planning Commission to do some additional studies. **The motion carried unanimously**.

#### Administrative Business:

- <u>Staff Report</u> : Ms. VanMarter had nothing to report.
- Approval of the December 11, 2017 Planning Commission meeting minutes

**Moved** by Commissioner McManus, seconded by Commissioner Rauch, to approve the minutes of the December 11, 2017 Planning Commission Meeting as presented. **The motion carried unanimously.** 

- <u>Annual Report:</u> Ms. VanMarter submitted the Annual Report of action taken by the Planning Commission in 2017.
- <u>Member Discussion</u>: There were not items discussed.
- <u>Adjournment</u>

**Moved** by Commissioner McManus, seconded by Commissioner Rauch, to adjourn the meeting at 10:11 pm. The motion carried unanimously.

Respectfully Submitted,

Patty Thomas, Recording Secretary