

February 14, 2018

Planning Commission Genoa Township 2911 Dorr Road Brighton, Michigan 48116

<b>Attention:</b>	Kelly Van Marter, AICP
	Planning Director and Assistant Township Manager
Subject:	Aldi Food Market – Site Plan Review #1
<b>Location:</b>	2260 E. Grand River Avenue - south side of Grand River, west of Chilson Road
Zoning:	GCD General Commercial District

#### Dear Commissioners:

At the Township's request, we have reviewed the site plan (cover sheet dated 1/29/18) proposing an expansion of the existing Aldi Food Market at 2260 E. Grand River Avenue.

We have reviewed the proposal in accordance with the applicable provisions of the Genoa Township Zoning Ordinance.

#### A. Summary

- 1. The proposal includes parking spaces across a property line. While this area is covered with an easement for such, it does not comply with the setback requirements for parking lots.
- 2. The Planning Commission has approval authority over the building elevations, including materials and colors.
- 3. The amount of parking proposed (125% of the minimum) exceeds the maximum allowed by Ordinance (120%). The applicant must either remove at least 4 spaces or demonstrate to the Planning Commission why the additional spaces are necessary.
- 4. Items #1 and #3 could be mitigated by the removal of the 6 spaces described above.
- 5. The Commission may wish to require looped striping for the parking spaces.
- 6. The greenbelt and parking lot landscaping include evergreen and ornamental trees, whereas the Ordinance requires canopy trees (2.5" deciduous).

#### B. Proposal

The applicant requests site plan review and approval for a 2,254 square foot addition to the existing 16,657 square foot grocery store.

Retail establishments with up to 30,000 square feet of gross floor area are permitted by right in the GCD. As such, the project requires site plan review/approval given that the size of the expansion (approximately 13%) exceeds the threshold for a "minor" deviation from an approved plan.



Aerial view of site and surroundings (looking north)

#### C. Site Plan Review

**1. Dimensional Requirements.** The proposal has been reviewed for compliance with the dimensional standards of the GCD, as follows:

	Lot	Size	N	Minimum	Setback	s (feet)		
District	Lot Area (acres)	Width (feet)	Front Yard	Side Yard	Rear Yard	Parking	Max. Height	Lot Coverage
GCD	1	150	70	15	50	20 front	35'	35% building
GCD	1	130	70	13	30	10 side/rear	2 stories	75% impervious
Proposal	2.66	358	91.2	15.4 (W)	57.6	4-29 front	24.5'	16.3% building
Froposai	∠.00	338	71.2	135 (E)		10 side/rear*	1 story	61.8% impervious

<sup>\*</sup> The majority of the existing and proposed parking meets the side/rear setback standard; however, there are 6 spaces proposed at the rear of the site that cross over the property line. This area is noted as being covered by an easement for "parking, landscaping and public utilities," but, the proposed design results in a non-compliant setback.

With the exception of the east side yard, the building and parking setbacks noted are existing conditions. The reduced parking front setback was allowed as part of the original development in 2008 via variance from the ZBA.

**2. Building Materials and Design.** The proposed elevations, including colors and materials, are subject to review and approval by the Planning Commission.

Building materials include brick and a CMU base to match the existing building. New materials include decorative wood panels and metal architectural panels (replacing glass) on the upper level of the store entrance.

In our opinion, the proposed building provides a complementary mix of materials that match the existing building, while also introducing new materials to modernize the overall design.

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**3. Parking.** Per Section 14.04, 76 parking spaces are required, while 95 are proposed, including the 5 required barrier free spaces.

The amount of parking proposed slightly exceeds that allowed by Ordinance. More specifically, Section 14.02.06 requires Planning Commission approval for parking that exceeds 120% of the minimum requirement.

The proposal is 125% of the minimum requirement. As such, the applicant must either explain to the Commission why the additional spaces are necessary or reduce the amount parking proposed to 91 spaces (or less). As an alternative, the applicant may wish to remove the 6 new spaces that cross the property line (as noted under item #1 above).

The parking spaces and drive aisles meet or exceed the dimensional standards of Section 14.06; however, neither the existing nor proposed parking spaces provide the looped striping required. The Commission may wish to require this as part of the project.

- **4. Pedestrian Circulation.** There is an existing public sidewalk along Grand River, as well as existing walks along the north and east sides of the building. The proposal includes widening of the sidewalk along the east side of the building and crosswalk striping across the parking lot connecting the public and private walks.
- **5. Vehicular Circulation.** The site has direct access to Grand River via a restricted turning movement drive, as well as indirectly by a shared access easement with the County complex to the east. No changes are proposed to the existing circulation patterns.
- **6. Loading.** The development has an existing truck well at the rear of the building for deliveries. The size, design and location are all in accordance with current standards and no changes are proposed.
- 7. Waste Receptacle and Enclosure. There is an existing waste receptacle and enclosure within the truck well at the rear of the building. Our original review of the site plan (2008) indicated that the receptacle/enclosure complied with the standards of Section 12.03 and no changes are currently proposed.
- **8. Landscaping**. We reviewed the landscape plan for compliance with the standards of Section 12.02, as noted in the following table:

Location	Requirements	Proposed	Comments
Greenbelt	20' width	0-29' width (existing)	The Ordinance requires 2.5" trees
	9 canopy trees	4 existing trees	within the greenbelt (as opposed
	2' tall hedgerow	5 ornamental trees (proposed)	the 2" ornamentals proposed)
Buffer Zone C (E)	10' width	10' width	Requirements met; however, 5 of
	23 canopy trees OR	7 existing trees	the 6 evergreen trees are within the
	23 evergreen trees OR	3 canopy trees (proposed)	easement on the adjacent property
	90 shrubs	6 evergreen trees (proposed)	
		28 shrubs (proposed)	
Buffer Zone C (W)	10' width	10' width	Requirements met
	16 canopy trees OR	8 existing trees	
	16 evergreen trees OR	2 evergreen trees (proposed)	
	64 shrubs	24 existing shrubs	
Parking lot	950 SF landscaped area	1,458 SF landscaped area	Total tree plantings are met;
	10 canopy trees	2 existing trees	however, the Ordinance specifies
		5 canopy trees (proposed)	2.5" deciduous (as opposed to the
		2 evergreen trees	evergreen and ornamental trees
		1 ornamental tree	proposed)
Detention pond	etention pond 8 trees (deciduous or 10 trees (existing)		Requirements met
	evergreen)	73 shrubs (existing)	
	80 shrubs	7 shrubs (proposed)	

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**9. Exterior Lighting.** The submittal includes a lighting plan (Sheet LT1), which includes 9 light poles throughout the parking lot, as well as 24 wall mounted/under canopy fixtures.

The proposed lighting plan, including pole heights, fixture details and light intensities, complies with current Township standards.

**10. Signs.** The existing monument sign is to remain, though replacement of the existing sign cabinet is proposed. This will not alter the sign height or area, both of which comply with current standards.

The building elevation drawings show the two existing wall signs in the northeast corner of the building. As a side note, the 2<sup>nd</sup> wall sign was allowed by the Planning Commission as part of the 2008 approval.

**11. Impact Assessment.** The submittal includes an amended Impact Assessment (January 25, 2018). In summary, the Assessment notes that the project is not anticipated to adversely impact natural features, public services/utilities, surrounding land uses or traffic.

Should you have any questions concerning this matter, please do not hesitate to contact our office. I can be reached by phone at (248) 586-0505, or via e-mail at <a href="mailto:borden@lslplanning.com">borden@lslplanning.com</a>.

Respectfully,

LSL PLANNING, A SAFEBUILT COMPANY

Brian V. Borden, AICP Planning Manager



February 13, 2018

Ms. Kelly Van Marter Genoa Township 2911 Dorr Road Brighton, MI 48116

Re: Aldi Expansion Site Plan Review #1

Dear Ms. Van Marter:

Tetra Tech conducted a site plan review of the Aldi expansion plans submitted by Desine, Inc. The submission included site plans dated January 29, 2018, an impact assessment, and site plan review application. We offer the following comments:

#### **SUMMARY**

- 1. Heavy duty bituminous section
- 2. Stormwater calculations

#### SITE PLAN

- 1. The delivery truck driveway and turnaround is specified as heavy duty bituminous pavement. The Township standards requires an industrial parking lot to be 1.5-inch wearing course followed by a 4-inch leveling course. The proposed plans have a 1.5-inch wearing course followed by a 2.5-inch leveling course. The proposed leveling course should be increased to 4 inches to meet heavy truck load requirements.
- 2. The proposed storm runoff area and coefficients do not differ greatly from the original design. However, utilizing the surveyed elevations and new calculations, it appears that the existing detention basin is not adequate for the proposed stormwater flows. Genoa Township stormwater management standards require on-site detention basins to meet the requirements of the Livingston County Drain Commissioner's "A Simple Method of Detention Design" for a 100-year frequency storm. The following comments identify where the current stormwater detention basin does not meet the Drain Commissioner's requirements:
  - O A detention basin shall have a minimum of 1-foot free board above the 100-year storage level. The proposed 100-year elevation is 922.7 while the site plan shows the highest elevation of the detention basin is 923.
  - O The conveyance pipes for stormwater shall be free flowing and self-draining, therefore, no portion of a storm drainage system shall be permanently submerged. Because the required 100-year storage volume, 16,651 cubic feet, is not able to be stored in the existing detention basin the petitioner has proposed utilizing conveyance storage to meet this requirement. Storage of stormwater in conveyance pipes is not recommended and the existing detention basin should be enlarged to account for the required 100-year storage volume.

Ms. Kelly Van Marter

Re: Aldi Expansion Site Plan Review #1

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o The petitioner has not accurately calculated the 'bankfull' flood volume. It should be calculated using "8160 x A x C". This will affect the elevation and size of the outlet holes in the discharge standpipe. Due to the detention basin requiring expansion, the discharge standpipe elevations and hole sizes will need to be recalculated and likely revised in the field. In addition, a standpipe detail showing the discharge holes will need to be added to the plans.

The petitioner should address the above comments and resubmit the plans for review. Because there are no changes to the existing public water main or sanitary sewer, these plans will not require review through MHOG Construction Plan Review process. It should be noted, however, that the two most southerly sanitary sewers on the plans are two 12-inch force mains. Care should be taken when constructing the new parking stalls over these force mains.

Sincerely,

Gary J. Markstrom, P.E.

Unit Vice President

Marguerite K. Davenport

Project Engineer

copy: Christopher A. Grzenkowicz, P.E., DESINE, Inc.

# **BRIGHTON AREA FIRE AUTHORITY**



615 W. Grand River Ave. Brighton, MI 48116 o: 810-229-6640 f: 810-229-1619

February 14, 2018

Kelly VanMarter Genoa Township 2911 Dorr Road Brighton, MI 48116

RE: Aldi expansion

2260 E. Grand River Genoa Twp., MI

#### Dear Kelly:

The Brighton Area Fire Authority has reviewed the above mentioned site plan. The plans were received for review on February 1, 2018 and the drawings are dated January 29, 2018. The project is based on an existing 16,657 sq.ft. Mercantile occupancy that will undergo site alteration to accommodate a new 2,254 sq.ft. addition. The plan review is based on the requirements of the International Fire Code (IFC) 2018 edition.

1. The existing site condition for the FDC location is not identified, however a relocation is proposed. The FDC shall be located on the Grand River side of the structure within 100' of the fire hydrant.

IFC 912.2

- 2. A new fire hydrant must be added to the rear of the parking lot near the new parking areas. Preferable location would be along the southern curb east of the new parking areas. This will provide for additional fire flow and proper spacing.
- 3. The building addition shall be provided with automatic sprinkler protection in accordance with NFPA 13, Standard for the Installation of Automatic Sprinkler Systems.

IFC 903

4. The building address shall be a <u>minimum of 6"</u> high letters of contrasting colors and be clearly visible from the street. The location and size shall be verified prior to installation.

IFC 505.1

5. The access road along the east and south sides of the structure shall be a minimum of 26' wide. With a width of 26' wide, the building side of the drive along the east shall be marked as a fire lane as well as along the southern drive along the rear. Include the location of the proposed fire lane signage and include a detail of the fire lane sign in the submittal. Access roads to site shall be provided and maintained during construction.

IFC D103.1 IFC D103.6

6. Provide an emergency vehicle circulation plan for the site entering from the Grand River Entrances and the County Office Tie-in. Access throughout the site and access drives shall provide emergency vehicles with a turning radius of 30' inside and 50' outside.

IFC D103.3





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7. The location of the Knox Box shall be indicated on future submittals. The existing Knox box will likely need to be relocated to be adjacent to the new main entrance of the structure. Coordinate with the fire authority for placement and follow manufacturers instruction regarding installation.

IFC 506.1

Additional comments will be given during the building plan review process (specific to the building plans and occupancy). The applicant is reminded that the fire authority must review the fire protection systems submittals (sprinkler & alarm) prior to permit issuance by the Building Department and that the authority will also review the building plans for life safety requirements in conjunction with the Building Department.

If you have any questions about the comments on this plan review please contact me at 810-229-6640.

Cordially,

Rick Boisvert, CFPS Fire Marshal

CIVIL ENGINEERS

2183 PLESS DRIVE, BRIGHTON, MICHIGAN 48114-9463 (810) 227-9533 FAX (810) 227-9460

EMAIL: desine@desineinc.com

February 22, 2018

DESINE INC

**GENOA TOWNSHIP** 

FEB 2 3 2018

RECEIVED

Ms. Kelly VanMarter Genoa Township 2911 Dorr Road Brighton, MI 48116

Re: ALDI FOOD MARKET #51 Remodel and Expansion

Section 6, Genoa Township, Property I.D. #4711-06-100-044

Dear Ms. VanMarter;

Please find enclosed the following documentation for the above referenced project:

- Four (4) sets of the revised Site Plan
- Four (4) copies of the Impact Assessment

The enclosed Site Plan has been revised to address review comments prepared by the Township consultants. In response to the LSL Planning, Inc., review letter dated February 14, 2018 we offer the following (please refer to the numbered summary items on the review letter):

- 1. We are requesting a variance to the property line setback for parking to allow the proposed parking spaces within the existing parking easement.
- 2. We concur. Please note materials on the proposed expansion will match existing building.
- 3. ALDI, Inc. requires new stores to provide 1 parking space per each 200 square feet of the total building area.
- 4. See comments #1 and #3, above.
- 5. We have noted the looped striping for the parking spaces on Sheet SP, Note 8.
- 6. Pertaining to the greenbelt landscaping, it was determined that Evergreen trees would be acceptable to screen adjacent properties. Ornamental trees size has been revised to 2.5" caliper. Refer to Table on Sheet LA1.

In response to the Brighton Area Fire Authority., review letter dated February 14, 2018 we offer the following (please refer to the numbered summary items on the review letter):

- The proposed FDC location as depicted on the plans is acceptable to the Fire Marshal.
- 2. A new fire hydrant is proposed to be constructed South of the existing parking lot.
- 3. Noted
- 4. Noted
- 5. Fire lane signage has been added to the plans and a sign detail added to Sheet DT2.
- 6. Emergency vehicle circulation has been depicted on Sheet DT4 with turning radii of 30 feet inside and 50 feet outside.
- 7. The Knox box location has been noted on Sheet A-201, adjacent to the new main entrance.

In response to the Tetra Tech., review letter dated February 14, 2018 we offer the following (please refer to the numbered summary items on the review letter):

- 1. Heavy duty bituminous pavement cross-section has been revised to provide a 4-inch leveling course.
- 2. Modifications to the storm water management system plans include:
  - o Additional spot grades have been added to Sheet EX1 around the existing detention pond to more accurately reflect the existing ground elevation. The existing detention basin provides the required 1-foot free board above the 100-year storage elevation.
  - o The existing Detention Volume Calculations and Control Structure Calculations have been updated on Sheet UT2. The Control Structure will be modified to provide the required detention pond storage volume. A Control Structure Modification Detail has been added to Sheet DT3 to depict the proposed structure modifications.
  - The Control Structure Calculations on Sheet UT2 have been revised to reflect the noted Bankfull Flood Volume calculation change. A control structure standpipe detail, CS-101, has been added to sheet DT3.

If you have questions pertaining to the revised plans, or should you require additional information, please contact me at your convenience.

Respectfully submitted,

DESINE INC.

Wayne M. Perry, P.E.

encl.

cc: Mr. David Kapusansky/Aldi Inc.

#### **IMPACT ASSESSMENT**

for the

# ALDI FOOD MARKET SITE DEVELOPMENT

Developer:
ALDI Inc.
2625 North Stockbridge Road
Webberville, Michigan 48892

Prepared by:
DESINE Inc.
2183 Pless Drive
Brighton, Michigan 48114

April 30, 2008 Amended January 25, 2018

#### **Introduction**

This impact assessment has been prepared pursuant to Section 18.07 of the Genoa Township Zoning Ordinance. This assessment addresses the impact of development of the proposed ALDI Food Market Commercial Development on the natural features, economic condition and social environment of the Township.

The project site consists of approximately 3.13 acres of property zoned **General Commercial District (GCD)** located on the south side of Grand River Avenue, west of Golf Club Road, in Section 6 as shown in Figure 1. The site is currently developed with a 16,657 SF single story commercial building, paved parking area and associated infrastructure. The existing building contains an ALDI Food Market limited assortment grocery store. The existing parking area has a limited access right in / right out driveway on Grand River Avenue and a full access driveway along the Livingston County Complex driveway.

The existing building, paved parking area and infrastructure will remain. A 2,254 SF building addition is proposed along the east side of the existing building within a portion of the existing parking area to provide a total of 18,911 SF that will continue to be utilized as an ALDI Food Market. Minor modifications of the existing parking area are proposed to accommodate the proposed building addition. 24 new parking spaces are proposed along the existing rear drive. The proposed site improvements will increase the parking count from 71 to 95 spaces to accommodate the increase in customer base since 2008. Additional site improvements include site lighting and landscaping improvements as depicted on the Site Plan.

The subject property is located in a significantly developed area, surrounded by restaurant, retail, office and industrial uses. An aerial photograph (circa 2017) of the surrounding area and existing conditions is provided in Figure 2.

This impact assessment has been prepared under the direction of and by:

Christopher A. Grzenkowicz, P.E. DESINE Inc. 2183 Pless Drive Brighton, Michigan 48114 (810) 227-9533

The civil engineering / surveying firm of DESINE Inc. has been in practice since 1989. Mr. Grzenkowicz is a licensed Civil Engineer with experience in private and municipal developments including a number of projects within Genoa Township and Livingston County.

#### A. IMPACT ON NATURAL FEATURES

Prior to the construction of the existing ALDI Food Market, the subject property was previously developed with a fast food restaurant and a bank. The majority of the site had been disturbed during the construction of the previous development. The previous buildings, parking areas and infrastructure was removed to allow for construction of the existing ALDI Food Market development. The southern portion of the site contained brush and small trees prior to the existing development. This area was utilized to construct a detention basin for treatment of storm water runoff from the ALDI development. Following construction of the detention basin, this area was stabilized with vegetative growth and now contains a mixture of grasses, shrubs and mature trees. The existing topography of the site generally slopes from Grand River in the northeast to the railroad corridor in the southwest. Slopes are ranging from 1 percent to 5 percent. Elevation difference across the property is approximately 8 feet. Surface water runoff from the subject and adjacent properties flows generally from northeast to southwest toward the existing drainage swale adjacent to the railroad tracks. An existing underground storm sewer system collects runoff from the subject property and directs it to an existing detention basin which discharges at an agricultural rate to the serving the existing drainage swale adjacent to the railroad tracks. The existing site topography is depicted on the Existing Conditions Plan.

The soils on the subject property are primarily Boyer-Oshtemo loamy sands. These soils are generally well-drained, moderately permeable, loamy sands. Soil classifications are prepared by the United States Department of Agriculture, National Resource Conservation Service. The Soils Map, shown in Figure 3, shows the locations of specific soil types as classified.

Previous development of the subject property required land balancing to establish final

grades and provide proper drainage. The proposed building addition and parking improvements are designed to mesh with the existing site improvements as close as possible. Minimal demolition and site grading is necessary to accommodate the proposed building and site improvements.

The limits of disturbance have been depicted on the Soil Erosion and Sedimentation Control Plan. Grading for this project will maintain the general character of the existing site conditions. Development of the proposed site improvements will require minimal exporting of excess topsoil and minimal importing of additional granular fill material.

Vegetation in the area of the proposed parking area improvements will be disturbed. Existing vegetation in these areas consists of open lawn and landscape trees planted during the original development of the existing ALDI Food Market. No landmark trees have been identified on the site. Existing landscape trees consist of spruce, pine, pear, locust, maple and ginko. Existing on-site mature trees consist of elm, cottonwood, box elder, poplar, apple, aspen and oak. The existing mature trees are located in southwestern portion of the property surrounding the existing detention basin. No existing mature trees will be removed as a part of this project. Existing landscape trees within the proposed building addition area and proposed parking areas will be removed to accommodate the proposed site improvements. Those trees to be removed will be replaced with new plantings. Additional landscaping is proposed to ensure the site is in conformance with the current Zoning Ordinance requirements. Existing trees, including those trees to be removed, are depicted on the Existing Conditions Plan. Proposed replacement trees and other proposed landscaping improvements are depicted on the Landscape Plan.

No wetland areas are present on the project site.

Surface drainage characteristics of the site will not be significantly impacted. The proposed site improvements are designed to direct storm water runoff to the existing storm water management system. The proposed site improvements will result in a minimal reduction of permeable area on the subject property as compared to existing conditions and a minimal increase in the surface water runoff generated from the development site. The proposed increase in lot coverage is approximately 2.8%. The overall proposed lot coverage is 61.8%, well below the 75% allowable. Surface water runoff generated by the proposed site improvements will be directed to the existing detention basin.

The existing detention basin was designed and constructed to accept storm water runoff from the project site in accordance with Genoa Township and Livingston County Drain Commissioner rules and regulations. An analysis of the existing storm water management system has been performed to verify that the existing storm water management system can

accommodate the runoff to be generated by the proposed site improvements. The storm sewer analysis calculations are provided on sheet UT2 of the Site Plan.

The minimal impact of surface drainage alterations will not significantly impact local aquifer characteristics or groundwater recharge capacity. Surface water runoff from the developed site will flow into the existing detention basin which discharges along the historical drainage path adjacent to the railroad tracks.

Wildlife habitats exist primarily on the southern portion of the property which includes the existing detention basin, tall grasses, brush, shrubs and mature trees. Wildlife supported in these habitats are generally smaller woodland creatures, field animals and birds. Larger animals, such as deer, may traverse the site. The existing developed portion of the site does not contain any significant wildlife habitat areas. The proposed site improvements do not result in a significant impact to the existing wildlife habitats. No site improvements or disturbance is proposed in the southern portion of the site.

The project site does not currently support significant wildlife habitats and development of the proposed site improvements will not have a significant impact on the overall habitat quality. No significant adverse impact to existing natural features are anticipated due to the proposed site improvements for this property.

#### B. IMPACT ON STORM WATER MANAGEMENT

The subject property contains an existing storm water management system consisting of an underground storm sewer network, a sedimentation basin and a detention basin. Minimal earthwork and site grading is necessary to accommodate the proposed site improvements and to direct surface drainage from the proposed site improvements to the existing storm water management system. Earthwork will be limited to the areas of proposed building and parking improvements.

Catch basins will collect surface water from parking and drive areas, building areas, and open space areas. Grading for the proposed site improvements will mesh with existing grades immediately surrounding the improvement areas. No adverse impact to adjoining properties is anticipated due to grading or construction of the proposed site improvements.

Soil erosion and sedimentation control is governed by the Soil Erosion Control Act No. 347 of the Public Acts of 1972, as amended as administered by the Livingston County Drain Commissioner. Silt fencing, sediment inlet filters and other soil erosion control measures will be required around the areas of the proposed site improvements as depicted on the Soil Erosion and Sedimentation Control Plan. The Contractor shall be responsible for initiating

and maintaining adequate dust control measures during construction. The Contractor shall also be responsible for complying with all soil erosion and sedimentation control regulations during and after construction until the project site is fully stabilized and vegetative cover is established within the disturbed areas outside of hard surfacing.

Impact to adjoining properties due to the construction of the site improvements will be minimized by implementing soil erosion and sedimentation control measures. No adverse impact to adjacent properties due to surface water runoff will be created as a result of the proposed site improvements.

#### C. IMPACT ON SURROUNDING LAND USES

Property to the North of the site is zoned Neighborhood Service (NS) and is occupied by CVS Pharmacy. Property to the East is zoned General Commercial District (GCD) and serves as the entryway to the Livingston County East Complex, east of that is PNC Bank. The Livingston County East Complex is located south of the subject parcel and is zoned GCD. West of the property is Mourad's Grill, also zoned GCD.

The Genoa Township Future Land Use Plan designates the subject property for General Commercial uses. The surrounding property is also designated for General Commercial use.

The existing use is consistent with the existing development in the surrounding area and is consistent with the long term planning within the Township. The proposed landscaping and architecture is consistent with the existing site development to allow this site to continue to be in harmony with the surrounding area. No adverse impact to the surrounding area is anticipated due to the proposed site improvements. All areas disturbed by construction will require restoration.

The continued use of the property as a general commercial use will not create any significant emissions of smoke, airborne solids, odors, gases, vibrations, noise or glare discernable and substantially annoying or injurious to persons and/or property beyond the lot lines. Radioactive emissions and electromagnetic radiation shall not be emitted in excess of quantities established as safe in accordance with the ordinance when measured at the property lines. No flammable liquids, gases or explosives shall be stored or used on the property. No underground or aboveground storage tanks are proposed on the property.

#### D. IMPACT ON PUBLIC FACILITIES AND SERVICES

Police protection is and will continue to be provided by the Livingston County Sheriff and the Michigan State Police. Additional services required to accommodate the proposed site improvements are anticipated to be minimal.

Fire protection is provided by the Brighton Area Fire Department. The subject property is within the water district and fire hydrants are readily accessible for utilization in the event of a fire. Adequate fire protection systems are provided within the existing building and are proposed within the building addition. Additional fire protection services required to accommodate the proposed site improvements are anticipated to be minor.

Construction of the proposed site improvements will not create any direct adverse impact on the schools.

#### E. IMPACT ON PUBLIC UTILITIES

The subject property is located within the municipal sewer and water districts. Existing water main and sanitary sewer are located in easements to the south and east of the existing building. The existing building is connected to the existing sanitary sewer and water main.

Water service to the building is provided from the existing water main located within an easement that traverses the property south of the existing building. No significant increase in water use is anticipated as a result of the proposed site improvements. Capacity is available within the existing water system to provide adequate service to this development.

Sanitary sewer service for the development is provided by the existing sanitary sewer main which traverses the site south of the existing building. No significant increase in sanitary sewage flow is anticipated as a result of the proposed site improvements. Capacity is available within the existing sanitary sewer system to provide adequate service to this development.

The site is serviced by electric, gas, phone and cable TV systems located along Grand River Avenue. No significant increase in demand or use of the existing public utility systems is anticipated as a result of the proposed site improvements.

#### F. STORAGE AND HANDLING OF ANY HAZARDOUS MATERIALS

The proposed general commercial space will not utilize or store any potentially hazardous or polluting materials other than standard household type cleaning products. All solid wastes should be properly disposed of through a licensed waste disposal firm on a regular basis.

#### G. IMPACT ON TRAFFIC AND PEDESTRIANS

The project site is located along the Grand River Avenue corridor. Grand River Avenue, in the area of the proposed development, is major arterial public street consisting of four directional traffic lanes and a center left turn lane. Access to the site from Grand River Avenue is provided via a limited access, right in – right out driveway near the west side of the site and thru the driveway serving the Livingston County East Complex.

Traffic Engineering Associates, Inc. located in Lansing Michigan has prepared a Traffic Assessment for the existing development (Attachment 1). The Traffic Assessment has been provided to and reviewed by both the Livingston County Road Commission and Michigan Department of Transportation.

The Traffic Assessment projected the ALDI development to generate 49 vehicle trips in the AM Peak hour, 208 vehicle trips in the PM peak hour and 1534 vehicle trips daily. A significant amount of the trips are classified as Pass-By Trips. Pass-by trips are already present in the existing passing traffic.

The actual traffic generated by the development of the ALDI Food Market is now existing in the traffic stream. The existing ingress and egress points and signalized intersection of Grand River / Golf Club / Livingston County Complex Driveway operate at an acceptable level of service. The proposed site improvements are not anticipated to result in a significant increase in traffic. No significant impact on the major thoroughfares of Livingston County is anticipated as a result of the proposed site improvements.

The existing development provides service to pedestrian traffic through a pedestrian sidewalk connection from the existing sidewalk along Grand River to the building entrance. The pedestrian access will be maintained as a part of the proposed site improvements. No significant increase in pedestrian traffic or adverse impact upon pedestrian traffic is anticipated as a result of the proposed site improvements.

#### H. SPECIAL PROVISIONS

A variance has been obtained for the reduction of the parking setback from the Right-of – Way of Grand River Avenue. A reduction of 16 feet was granted to allow the 4-foot parking setback from the R.O.W for the portion of the parking area adjacent to the 75-foot wide ½ R.O.W.

An easement agreement to utilize the driveway to the Livingston County East Complex has been entered into. No additional special provisions or requirements are currently proposed for this facility.

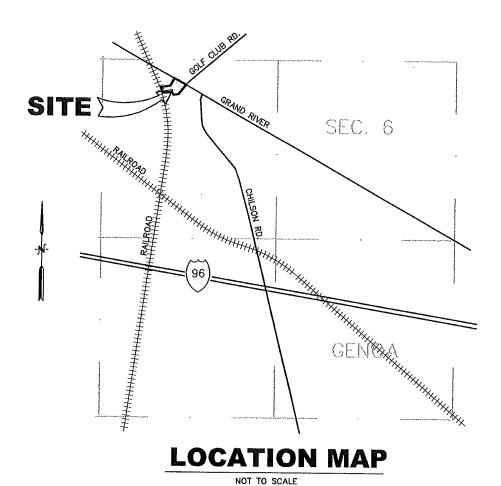
#### I. SITE LIGHTING

All site lighting shall meet the requirements of the Genoa Township ordinances. Exterior building mounted site lighting shall be shielded and down directed. Pole mounted site lighting is proposed for this project. All pole-mounted lighting shall be shielded and down directed on the site. General site lighting, excluding safety and emergency lighting, shall be used between the times from dusk to 12:00 a.m. and from 5:00 a.m. to dawn.

#### J. HOURS OF OPERATION

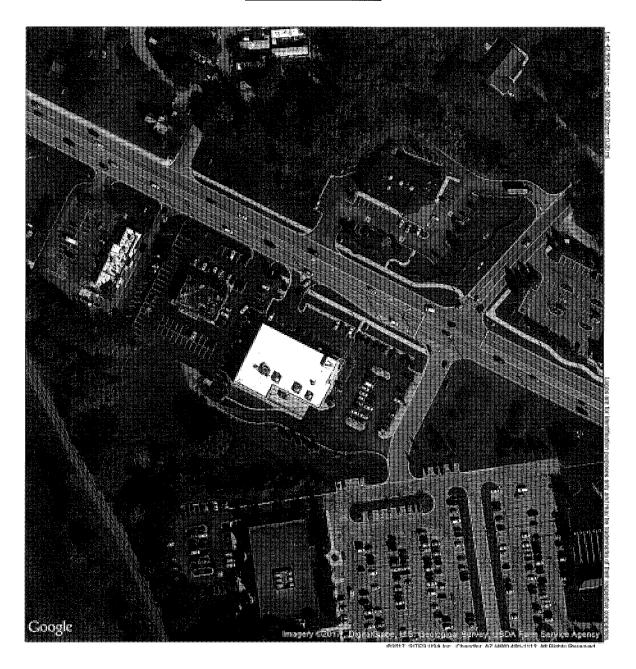
Hours of operation will generally be between 9:00 am to 8:00 pm seven (7) days a week. Corporate deliveries are the bulk of the product line and are scheduled when the store is closed. Corporate deliveries typically do not exceed one (1) delivery per day. Supplier delivery trucks deliver milk, bread, pop and similar products periodically during regular business hours. Supplier deliveries typically occur once or twice a week depending on the product delivered.

# FIGURE #1



Site Address 2260 E. Grand River Avenue Howell, MI 48843

# FIGURE #2



# **AERIAL PHOTOGRAPH**

NO SCALE

# FIGURE #3



# **SITE SOILS**

NO SCALE

Source:

Web Soil Survey

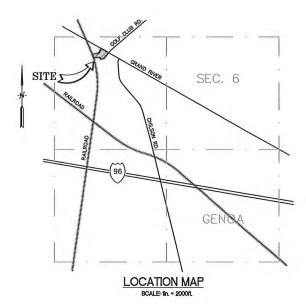
United States Department of Agriculture, Natural Resources Conservation Service

#### Soil Legend:

BtE Boyer-Oshtemo loamy sands, 18 to 25 percent slopes BtA Boyer-Oshtemo loamy sands, 0 to 2 percent slopes

MoE Miami loam, 18 to 25 percent slopes

# **ATTACHMENT #1**



#### LEGAL DESCRIPTION:

LEGAL DESCRIPTION:

Commencing at the Northwest Corner of Section 6, Town 2 North, Range 5 East, Genoa Township, Livingston County, Michigan; thence SB7'07'30"E 841.85 feet (SB7'02'00"E 844.70 feet record) along the North line of said Section 6; thence S60'06'45"E 609.89 feet (606.69 feet record) along the centerline of Grand River Avenue to the PLACE OF BEGINNING; thence continuing S60'06'45"E 358.81 feet along said centerline of Grand River Avenue (variable width Right-of-way); thence S50'33'30"W 64.79 feet; thence S29'56'47"W 193.35 feet; thence S84'35'37"W 100.61 feet; thence N03'34'18"W 14.51 feet; thence N60'06'45"W 85.63 feet; thence S69'20'23"W 346.13 feet (S69'14'28'W 346.09 feet record, also recorded as S69'14'28'W 345.99 feet); thence N60'06'45"W 120.00 feet along the Easterly line of the Tuscola and Saginaw Bay Railway (66 foot wide Right-of-Way), formerly known as Ann Arbor Railroad; thence (66 foot wide Right-of-Way), formerly known as Ann Arbor Rallroad; thence N68'26'35'E 254.80 feet; thence N29'53'15'E 300.00 feet to the Place of Beginning. Being a part of the Northwest 1/4 of Section 6, Town 2 North, Range 5 Eost, Genoa Township, Livingston County, Michigan. Containing 3.14 acres of land, more or less. Subject to the rights of the public over that portion thereof taken for Grand River Avenue, also subject to and together with all easements and restrictions affecting title to the above described

#### BENCHMARKS:

DATUM BASED ON PREVIOUS BENCHMARK AS DEPICTED ON AS-BUILT PLANS PREPARED BY DESINE INC., JOB No. 81389, REVISED DATE DECEMBER 3, 2008. DATUM AS DEPICTED = NGVD

#### PRIMARY (OFF SITE)

AT HOWELL, LIMINGSTON COUNTY, ON THE ANN ARBOR RAILROAD, 35 YARDS EAST OF THE NORTHEAST CORNER OF THE STATION; 60 YARDS WEST OF A HIGHWAY BRIDGE OVER THE TRACK; 7 YARDS NORTH OF THE WEST RAIL; A STANDARD TABLET STAMPED "G 105 1934" AND SET IN THE TOP OF A CONCRETE POST.

ELEVATION = 921.931 (NGVD) REF: SEE ABOVE

PREVIOUSLY DESCRIBED AS:

RAILROAD SPIKE IN THE NORTH SIDE OF AN UTILITY POLE. LOCATED ON THE SOUTH SIDE OF GRAND RIVER AND NEAR ABANDONED McDONALDS RESTAURANT

LOCATED NEAR THE EASTERLY SIDE OF THE NORTHERLY ENTRANCE FROM GRAND RIVER TO "ALDI"

ELEVATION = 925.46 (NGVD)

REF: SEE ABOVE

FIELD BOOK D405, PG. 43

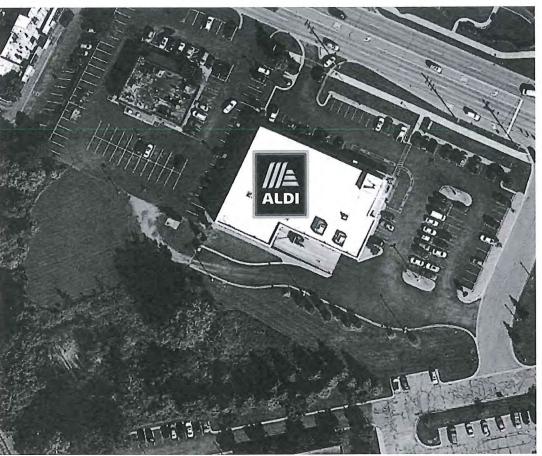
ARROW OF HYDRANT, LOCATED 57± FEET NORTHERLY OF THE NORTHEASTERLY BUILDING CORNER

ELEVATION = 930.09 (NGVD) REF: POINT #200

# SITE PLAN

# ALDI FOOD MARKET #51

REMODEL AND EXPANSION 2260 EAST GRAND RIVER AVENUE HOWELL, MI 48843 A PART OF THE NORTHWEST 1/4 OF SECTION 6, T2N, R5E GENOA TOWNSHIP, LIVINGSTON COUNTY, MICHIGAN



**AERIAL PHOTOGRAPH** 

Google maps

#### SHEET INDEX

**EXISTING CONDITIONS & DEMOLITION PLAN** EX2 **EXISTING SITE DATA & DEMOLITION NOTES** 

SITE PLAN SP

UT1 **UTILITY PLAN** 

STORM WATER MANAGEMENT CALCULATIONS UT2

**GRADING PLAN** GR

SOIL EROSION & SEDIMENTATION CONTROL PLAN SE1

SOIL EROSION & SEDIMENTATION CONTROL NOTES & DETAILS

LANDSCAPE PLAN LA1

LANDSCAPE NOTES & DETAILS LA2

SITE LIGHTING PLAN LT1

LT2 SITE LIGHTING NOTES AND DETAILS

SITE PAVEMENT NOTES & DETAILS DT1

SIGNAGE & PAVEMENT MARKING NOTES & DETAILS DT2

STORM SEWER NOTES & DETAILS DT3

WATER MAIN & SANITARY SEWER NOTES & DETAILS

M.H.O.G. STANDARD DETAILS DT5

DT6 M.H.O.G. STANDARD DETAILS

BUILDING FLOOR PLAN / OPERATIONS PLAN

EXTERIOR ELEVATIONS A-201



#### ENGINEER/SURVEYOR

DESINE INC. 2183 PLESS DRIVE BRIGHTON, MICHIGAN 48114 PHONE: (810) 227-9533

## DEVELOPER / APPLICANT

ALDI Inc. 2625 N. STOCKBRIDGE ROAD WEBBERVILLE, MICHIGAN 48892 PHONE: (517) 521-3907

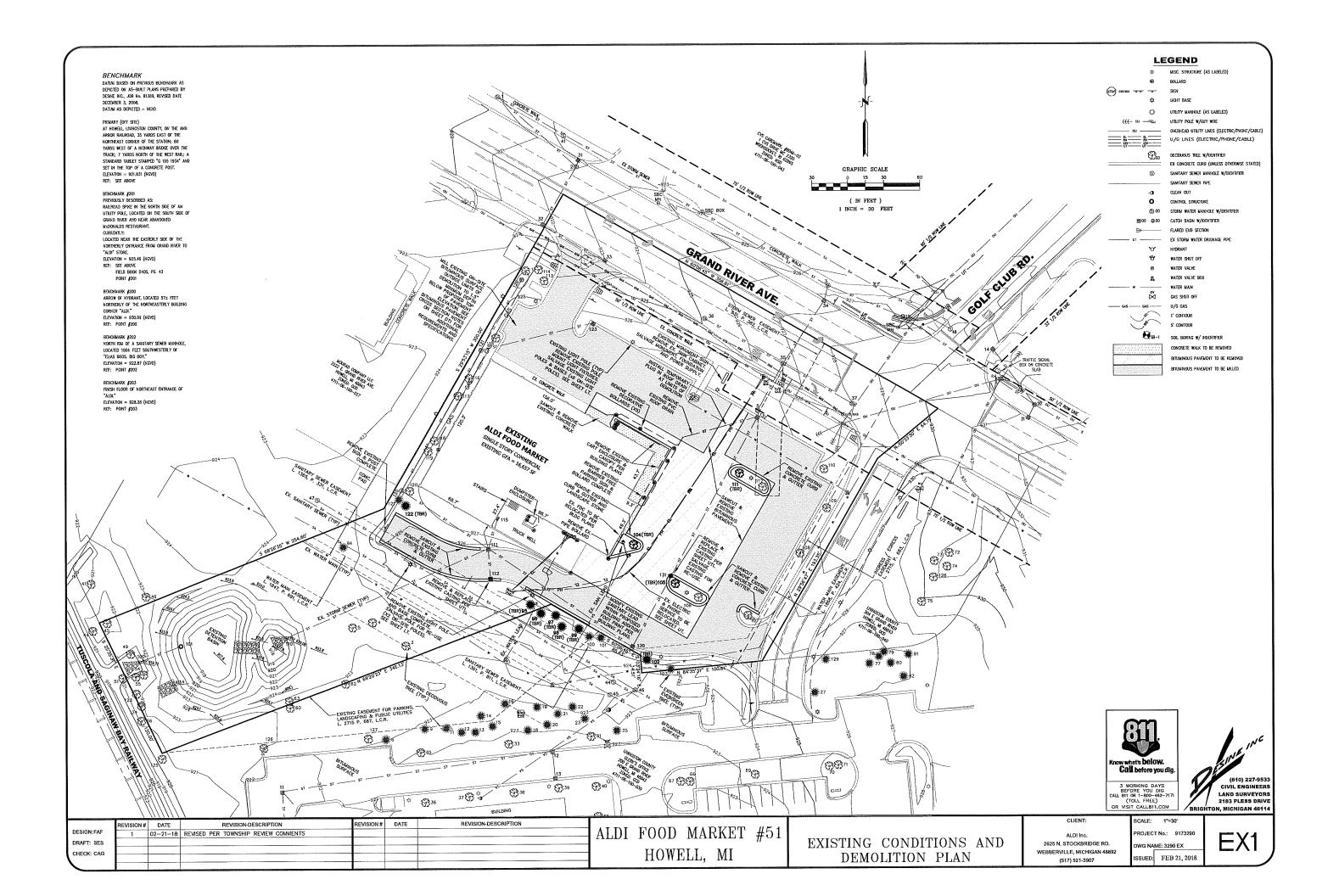
#### ARCHITECT

APD ENGINEERING AND ARCHITECTURE, PLLC 615 FISHERS RUN VICTOR, NEW YORK 14564 PHONE: (585) 742-2222



(810) 227-953 LAND SURVEYOR 2183 PLESS DRIV ON, MICHIGAN 4811 SCALE:

REVISED FEB 21, 2018 PROJECT No: 9173290 3290-CO PRINT: FEB 21, 2018



EXISTING TREE SCHEDULE:
1 8" MAPLE
3 18" ELM 4 24" COTTONWOOD 5 18" COTTONWOOD 6 9" PINE 9 8" PINE 11 9" PINE 12 8" PINE 13 9" PINE 14 8" PINE 15 9" PINE 16 8" PINE 18 8" PINE 19 9" PINE 20 9" PINE 21 10" PINE 22 10" PINE 23 6" PINE 25 6" PINE 27 6" PINE 31 11" CRABAPPLE 33 12" CRABAPPLE 35 9" CRABAPPLE 36 8" CRABAPPLE 37 12" CRABAPPLE 38 9" CRABAPPLE 39 9" CRABAPPLE 40 7" CRABAPPLE 41 10" BOX ELDER 44 7" TWIN OAK 45 11" BOX ELDER 46 12" TWIN BOX ELDER 47 12" TWIN BOX ELDER 48 24" TWIN LOCUST 9 18" POPLAR 50 18" LOCUST - DEAD 51 12" POPLAR 52 12" POPLAR 54 20" POPLAR 55 22" POPLAR 56 13" TWIN BOX ELDER 60 12" POPLAR - DEAD 62 10" APPLE 62 10" APPLE 63 19" ASPEN 64 7" CRABAPPLE 65 7" CRABAPPLE 66 3" CHERRY 67 3" CHERRY 69 3" CHERRY 70 4" CHERRY 71 4" CHERRY 72 9" CRABAPPLE 73 5" CRABAPPLE 74 9" CRABAPPLE 75 7" CRABAPPLE 77 7" PINE 78 8" PINE 82 6" PINE 83 6" TWIN PINE 84 9" PINE 85 9" PINE 86 9" PINE 87 9" PINE 88 9" PINE 89 6" CRABAPPLE 90 6" CRABAPPLE 91 6" CRABAPPLE 92 6" CRABAPPLE 93 6" CRABAPPLE 94 6" PINE 95 6" PINE (TBR) 96 6" PINE (TBR) 97 6" PINE (TBR) 98 6" PINE (TBR) 99 6" PINE (TBR) 100 6" PINE 101 6" PINE 102 6" SPRUCE (TBR) 103 6" SPRUCE 104 5" PEAR (TBR) 105 4" LOCUST (TBR) 106 4" LOCUST 107 4" PEAR 108 5" GINKO 108 5" GINKO 109 3" GINKO 110 6" PEAR 111 4" LOCUST (TBR) 112 3" LOCUST 113 6" PEAR 114 6" PEAR 114 6" PEAR 115 5" PEAR 116 6" PEAR 117 6" PEAR 118 6" PEAR 120 6" PEAR 121 6" SPRUCE 122 6" SPRUCE (TBR) 123 11" TRIPLE OAK 124 9" TWIN OAK 125 9" TWIN OAK 126 12" TWIN OAK 128 7" CRABAPPLE 129 6" PINE

EXISTING STRUCTURE INVENTORY

STORM MANHOLE #1	CATCH BASIN #14	C8-100
	RM 928.13	R34 926.60
RIM 920.89	KM 920.13	
		CS-101
STORM MANHOLE #2	YAROBASIN (115	
RtM 921.98	RM 926.12	R2M 922.67
		15" SW RCP 919.92
STORM MANHOLE #3	YARDBASIN #20 Ru 930 43	
RV 926.74	TAKURUASIN #20	CB-111
KM 925.74	RM 930.43	RM 925.08
	CATCH BASH #21	
STORN MANHOLE #5	CATCH BASIN #21	12" N PVC 920.58
RM 926.44	RM 931.09	12" HW HOPE-S 920.75
HORTHEAST 15" RCP 914.69	ica solies	12" NE RCP 920.74
SOUTH 8" RCP 914.64	CATCH BASH #22	
300 In 6 KCF 314.04	CATCH BASH 122	15" SE RCP 920.73
	R3M 934.08	12" S RCP 921.00
CATCH BASH 431		18" SW RCP 920.45
RN 921.77	YAROBASIN #23	10 011 110 020 10
SOUTHWEST 15" RCP	RN 934.28	
oppositional to the	/C= 05 1.20	CB-112
CATCH BASN \$32	ALTAL BLOW MA	RM 925.07
CATCH BASA PIZ	CATCH BASN #24	12" N RCP 921.08
RSM 921.69	RM 934.60	
NORTHEAST 15" RCP 916.29		CS-115
SOUTHEAST 15" RCP 916.49	CATCH BASIN #25	RBM 923.97
	RN 933.14	
CATCH BASH A33	Ida Specia	12° SW RCP 921.87
CATOT DATA POO	OLUMI CARNI PIE	
RSM 922.96 NORTHWEST 15" RCP 920.96	CAICH BASH \$20	MH-120
MORIHMEST 10 KCP 920190	K31 921,38	R94 924,97
	SOUTHEAST 15" HDPE 918.24 NORTHWEST 24" RCP 918.24	12" NE HDPE-S 921,15
CATCH BASN #34	NORTHWEST 24" RCP 918.24	
RtM 926.63		12" SE HDPE-S 921.11
SOUTHMEST 15" RCP	SANITARY SEWER WANHOLE #41 RM 922.15	
SOUTHER TO NO	DEL DOZIE	MH-121
WINGE DE	KM 92213	RSM 925.01
WANHOLE \$35		
RIM 926.70	SANITARY SEWER MANHOLE #42	12" NE RCP 921.51
NORTHWEST 12" RCP 920.60	RM 925.13	12" E RCP 921.49
NORTHEAST 15" RCP 914.10		12" SW HDPE-S 921.45
STITHEAST 12" RCP 922.10	SANITARY SEWER MANHOLE #43	12 34 10/2 3 3211/4
NORTHEAST 15" RCP 914.10 SOUTHEAST 12" RCP 922.10 SOUTHWEST 15" RCP 914.25	RM 924.83	YB-122
300 III I I I I I I I I I I I I I I I I	(VIII 327,CL)	
THE PARTY AND	ALLESTON MONEY WITHOUT ALL	RM 923.69
WANHOLE #36	SANTARY SEWER WANHOLE #44	12" SW RCP 921.69
Paul 925.01	RM 923.14	
SOUTHEAST 12" RCP 921.86	NORTHWEST SOR TOP OF PIPE 918.04	CB-123
	SOUTHEAST SDR TOP OF PIPE 917.79	R3M 926.02
NANHOLE #37	500()(2)(2)( (2)()(1)()(1)()(1)()(1)()(1)()	
RM 927.96	SANTARY SEWER MANHOLE #45	12" W RCP 921.93
NORTHWEST 12" RCP 922.61	RM 923.37	
MOKIHMEZI 15 MOS ASS 61	KM 923.37	08-130
		R34 925.63
CATCH BASH #11	SANITARY SEWER WANHOLE #46	12" NE RCP 921,39
E-NORTHEAST 24" RCP 916.11 SOUTH 24" RCP 916.11 SOUTHEAST 8" PVC 917.06	SOUTHEAST R' PVC 913.67	15" NW RCP 921.23
E-NUCIPEASI 24 NUP 910.11	MODINECT OF DIE 013.47	
SUUTH 24 RUP STELT	NORTHWEST OF THE STATE	CB-131
SOUTHEAST 8° PVC 917.06	SOUTHWEST 8 LAC 31791	FGM 926.60
		12" NF RCP 921.67
CATCH BASIN #12	SANITARY SEWER MANHOLE #47	
		12" SW RCP 921.58
W-SOUTHWEST 24" RCP 917.37	SOUTHEAST 8" PVC 912.02 HORTHWEST 8" CLAY 911.92	
E-300 IN ELECT OF DOD 017.57	MUDITACEL B. CLYA 011 05	CB-132
E-NORTHEAST 24" RCP 917.22 S-SOUTHWEST 12" RCP 917.37	manimizatio scali 311.32	RW 926.50
S-SOUTHMEST 12" RCP 917.37	ALLEST OF THE STATE OF THE STAT	
	SANITARY SEWER WANHOLE #48	8" W PVC 922.51
CATCH BASEN #13	RN 928.05	12" SW RCP 922.16
RM 921.09		
N-NORTHEAST 12" RCP 917.94		

#### DEMOLITION NOTES:

- 1. The demolition specifications of the Local Municipality are a part of this work. Refer to the General Notes on the project plans for additional requirements.
- 2. Contractor shall contact the 811 Underground Public Utility Locating System or other appropriate local underground utility locating Agency, a minimum of three (3) working days prior to performing demolition work. Existing utility information on the project plans may be from information disclosed to this firm by the Utility Companies, Local, County or State Agencies, and/or various other sources. No guarantee is given as to the completeness or accuracy thereof. Prior to construction, locations and depths of all existing utilities (in possible conflict with the proposed improvements) shall be verified in the field.
- 3. Contractor shall contact the appropriate Agencies to coordinate disconnect of the electric, gas, phone, cable and other public utilities as necessary prior to performing demolition work.
- 4. Contractor shall contact the appropriate Agencies to coordinate removal and/or relocation of any underground and/or overhead public utility lines as necessary prior to performing demolition work.
- Contractor shall recycle and/or dispose of all demolition material and debris in accordance with the appropriate Local, County, State and Federal regulations.
- 6. All bituminous and concrete pavement to be removed shall be saw cut at the limits of removal to provide for a clean straight edge for future abutment. The existing pavement that is to be removed, shall remain in place as long as possible to be used as a soil crossion control measure. Contractor shall coordinate the pavement removal work with the appropriate stages of construction. Contractor work shall include removing the existing pavement in sections / stages as necessary to minimize intended to the addresser tracel. impacts to the adjacent parcel.
- 7. All existing irrigation lines to be removed shall be terminated at the limits of demolition or as necessary to allow for construction of the proposed site improvements. Ends of pipe shall he capped and the location of marked for future connection.
- 8 All existing water main and sanitary sewer to be removed shall be terminated at the limits of 6. All existing water finals and samualy sever to be relativest same to estimate a via time to demolition or as indicated on the project plans. Temporary plugs shall be installed in the ends of pipe in accordance with the appropriate Agency and the locations of inarked for future connection. Permanent plugs shall be installed in the ends of pipe in accordance with the appropriate Agency. The Contractor shall record the location of all permanent plugs and provide the location information.
- 9. All existing storm sewer to be removed shall be terminated at the limits of demolition or as y. An existing storm sewer to be removed spat to eterminated at the initial condition of an indicated on the project plans. Temporary plugs shall be installed in the ends of pipe in accordance with the appropriate Agency and the locations of marked for future connection. Permanent bulkheads shall be installed in the ends of pipe and/or openings in terminating structures in accordance with the appropriate Agency. The Contractor shall record the location of all permanent bulkheads and provide the location information to the appropriate Agency.
- 10. All existing light sources to be removed shall have their power cables removed up to the power source or properly terminated for future connection at the limits of demolition or as necessary to allow for construction of the proposed site improvements. Removal and termination of power cables shall be performed in accordance with local electric codes.
- 11. All existing utility meters to be removed shall be properly removed to allow for reuse. Any existing utility meters that are not to be reused as a part of this project shall be returned to the appropriate Agency.
- 12. All trenches and/or excavations resulting from the demolition of underground utilities, building foundations, etc., that are located within the 1 on 1 influence zone of proposed structures, paved areas and/or other areas subject to vehicular traffic shall be backfilled with MDOT Class III granular material (or betre) to the proposed subgrade elevation. Backfill shall be shall be placed using the controlled density method (12" maximum lifts, compacted to 95% maximum unit weight, Modified Proctor).



(810) 227-953 CIVIL ENGINEER LAND SURVEYOR RIGHTON, MICHIGAN 4811

ı											
r		REVISION #	DATE	REVISION-DESCRIPTION	REVISION#	DATE	REVISION-DESCRIPTION	1777	HOOD	MADIZIO	11 = 4
ı	DESIGN:FAF	1	02-21-18	REVISED PER TOWNSHIP REVIEW COMMENTS				I ALDI	FOOD	MARKET	#51
1	DRAFT: SES							1			11
l	CHECK: CAG							1	HOWI	ELL, MI	
`								1		,	

**EXISTING** SITE DATA & DEMOLITION NOTES

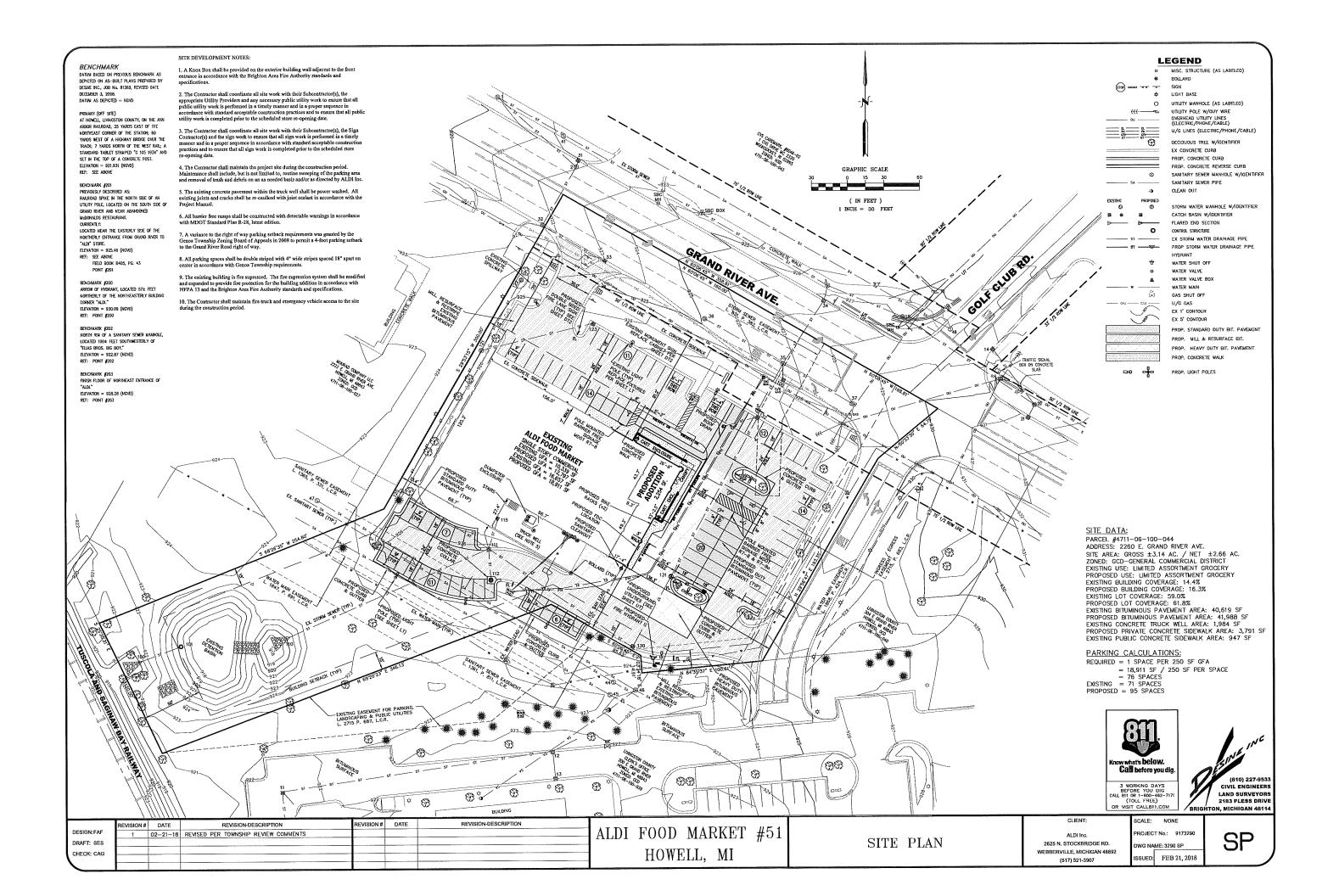
CHENT: ALDI Inc.

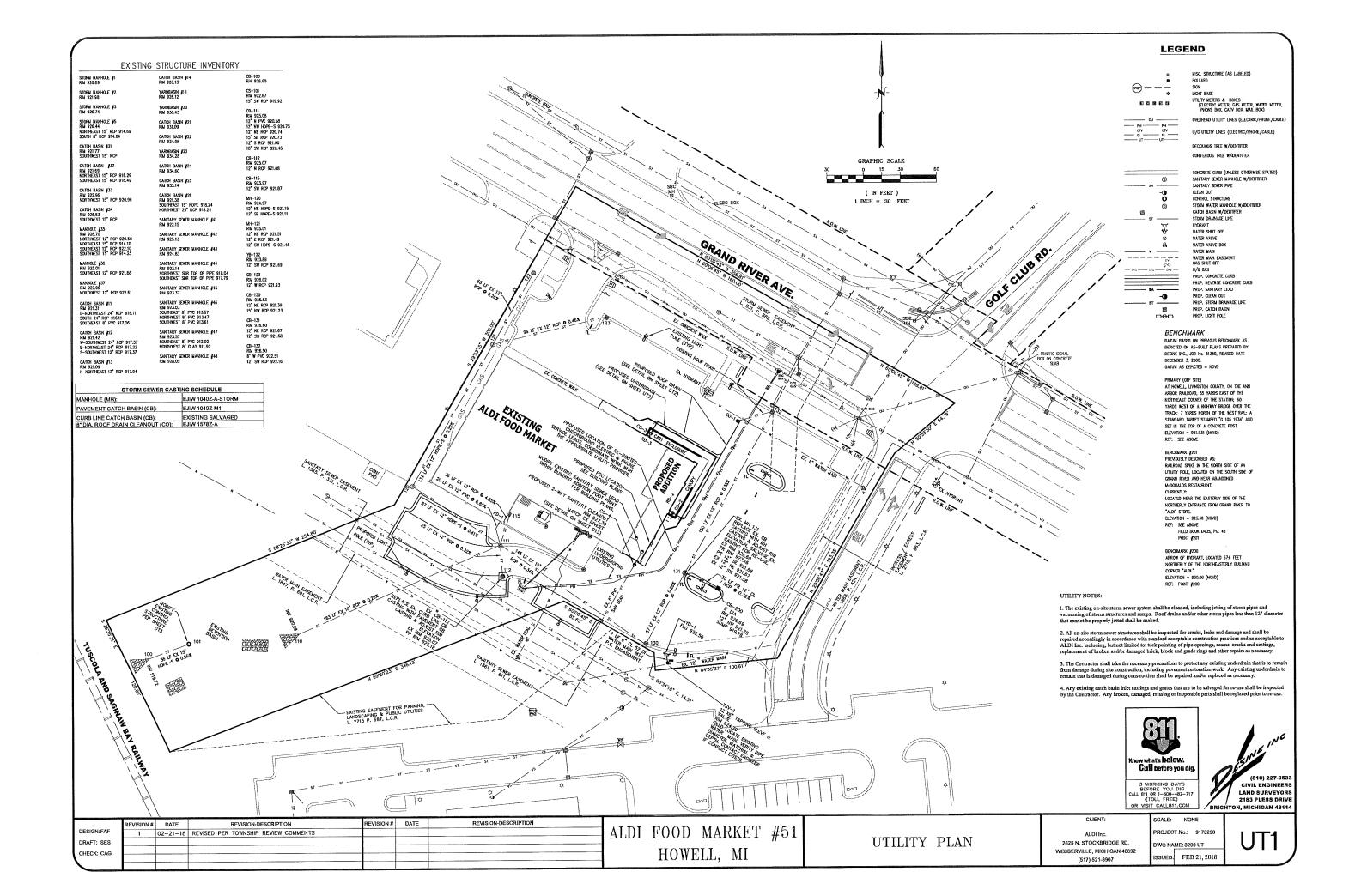
2625 N. STOCKBRIDGE RD. WEBBERVILLE, MICHIGAN 48892 (517) 521-3907

SCALE: NONE PROJECT No.: 9173290 WG NAME: 3290 EX

SSUED: FEB 21, 2018







Design Crit	eria:	10 year eve	ent (I = 175/	t + 25)	RCP (n=0.0	013)	*HDPE-S (	n=0.010)	**PVC (n=	0.013)							Date:	2/20/2018		
From	To	Inc.	Γ	Eqv.	Total	Т	T I	Q	Dia.	Slope	Slope	Length	Vel.	Time	Сар	H.G.	Grout	nd Elev.	Invert	Elev.
MH#	MH#	Acres		Area	Area	Time	Inch	(CIA)	of	pipe	H.G.	of	Flow	of	of	Elev.	Upper	Lower	Upper	Lower
CB#	CB#	1		100%	100%		Per	` ′	pipe			line	full	flow	pipe	upper	end	end	end	end
FES#	FES#	*A*	*C*	CA	CA	Min.	Hour	c.f.s.	inch	%	%	ft.	ft./sec.	min.	c.f.s.	end				
RD 3	WYE 1	0.01	0.80	0.01	0.01	15.0	4.38	0.04	8**	1.00	0.01	73	3.46	0.4	1.31	923.67	928.28	927.92	923.72	922.99
RD 2	WYE 1	0.01	0.80	0.01	0.01	15.0	4.38	0.04	8**	0.80	0.01	75	3.10	0.4	1.08	923.67	928.28	927.92	923.59	922.99
WYE 1	CO 1	0.00	0.00	0.00	0.02	15.4	4,33	0.09	8**	0.80	0.01	32	3.10	0.2	1.08	923.19	927.92	927.45	922.99	922.73
CO 1	EX 132	0.00	0.00	0.00	0.02	15.6	4,31	0.09	8**	0.80	0.01	27	3.10	0.1	1.08	923.18	927.45	926.48	922.73	922.51
001			0,00																	
EX 132	EX 131	0.18	0.74	0.13	0,15	15.7	4.30	0.65	12	0.33	0.03	150	2,61	1.0	2.05	922.72	926.50	927.16	922.16	921.67
LA IJZ	LX 101	0.,0	0.77	0.10	1 0.70		1.00													
200	EX 131	0.28	0.85	0.24	0.24	15.0	4,38	1.05	12	0.32	0.09	30	2.57	0.2	2,02	922.71	926.69	927.16	921.78	921.68
200	EX 101	0.20	0.00	0.24		10.0	7.00	1,00	<u> </u>											
EX 131	EX 130	0.00	0.00	0.00	0.39	16.7	4.20	1.64	12	0.28	0.21	67	2.40	0.5	1.89	922.53	927.16	925.63	921.58	921.39
	EX 111	0.00	0.87	0.00	0.57	17.2	4.15	2.37	15	0.34	0.13	145	3.07	0.8	3.77	922,28	925.63	925.08	921.23	920.73
EX 130	EXIII	0.21	0.07	0.10	0.57	11.4	4.10	2.07	1	0.04	0.10	1,10								
51/ 100	EX 121	0.17	0.86	0.15	0.15	15.0	4.38	0.66	12	0.46	0.03	96	3.08	0.5	2.42	922.52	926.02	925.01	921.93	921.49
EX 123	EX 121	0.17	0.86	0.15	0.15	15.0	4.30	0,00	12	0.40	0.00		0.00	0.0			-	1		
	=1, , , , ,		0.74	0.00	0.00	15.0	4.38	0.35	12	0.26	0.01	68	2.31	0.5	1.82	922.52	923.89	925.01	921.69	921.51
EX 122	EX 121	0.11	0.71	0.08	0.08	15.0	4.30	0.33	12	0.20	0.01	- 00	2.01	0.0	1.02	0	020.00	020.0.	021100	1
						45.5	4.00	0.99	12*	0.22	0.05	134	2.77	0.8	2.17	922.22	925.01	924.97	921,45	921.15
EX 121	EX 120	0.00	0.00	0.00	0.23	15.5	4.32			0.22	0.03	87	3.78	0.4	2.97	922.12	924.97	925.08	921.11	920.75
EX 120	EX 111	0.00	0.00	0.00	0.23	16.3	4.24	0.98	12*	0.41	0.04	01	3.10	0.4	2.51	322.12	324.31	320.00	321.11	320.70
		ļ							4000	0.05	0.47		11.87	0.1	9.32	922,12	926.74	925.08	921.95	920.58
EX RD 1	EX 111	0.43	0.80	0.34	0.34	15.0	4.38	1.49	12**	6.85	0.17	20	11.67	0.1	9.32	922.12	920.14	920.00	521.55	320,30
													0.40	- 0.4	7.43	922.09	923,97	925.08	921.87	920.74
EX 115	EX 111	0.05	0.90	0.05	0.05	15.0	4.38	0.22	12	4.35	0.01	26	9.46	0.1	7.43	922.09	923.97	920.00	921.07	920,74
																			004.00	004.00
EX 112	EX 111	0.12	0.84	0.10	0.10	15.0	4.38	0.44	12	0.32	0.02	25	2.57	0.2	2.02	922.10	925.07	925.08	921.08	921.00
					1												ļ	<del> </del>		
EX 111	EX 110	0.10	0.69	0.07	1.36	18.0	4.07	5.54	18	0.20	0.28	183	2.66	1.1	4.70	922.09	925.08	920.08	920.45	920.08
																ļ	<u> </u>	<b> </b>		
													ļ				ļ			
OUTLET P	IPE SIZED	TO CARRY	100 YEAR	STORM E	VENT (I = 2	75/t + 25):										ļ	<u> </u>			ļ
EX 101	EX 100	2.16	0.67	1.45	1.45	120.0	1.90	78.00	15*	0.56	0.11	36	5.12	0.1	6.28	921.01	922.67	919.72	919.92	919.72

# ALDI Food Market #51 Remodel & Expansion Control Structure Calculations CS - 101: Tributary Area : Compound Runoff Coefficient : Orifice Flow Coefficient : Allowable Outflow Rate : 100 Year Flood Volume : Bankfull Flood Volume = 8160 x A x C First Flush Volume = 1815 x A x C Low Water Level : First Flush Elevation : Bankfull Flood Elevation : 100 Year Flood Elevation : (Use available high water elevation) Use Nff = GRAPHIC SCALE 100 YEAR FLOOD: Qff + Qbf = [c\* Nif\* 0.0055 \* SQRT(2\* 32.2 \* (X100 - LWL)]] + [c\* Nif\* 0.0055 \* SQRT(2\* 32.2 \* (X100 - LWL)]] = Q100needed = Qa - (Qff + Qbf) = H100 = X100 - Xbf = A100needed = Q100needed / (c\* SQRT(2\* 32.2 \* H100) N100needed = A100needed / 0.0218 = ( IN FEET ) 1 INCH = 10 FEET Use N100 = 3 2" Holes at Elevation = 922.13 ROOF DRAIN & UNDERDRAIN DETAIL

PLAN VIEW

Project: ALDI FOOD MARKET #51 REMODEL & EXPANSION

#### ALD! FOOD MARKET #51

#### 100 YEAR STORM DETENTION CALCULATIONS

Pavement Area Building Area Grass Area	(C = 0.90) (C = 0.80) (C = 0.20)	= 1.07 Acres = 0.45 Acres = 0.65 Acres	Tributary Area (A) = Compound Runoff Coefficient (C) = Design Constant (K1) = A * C = Allowable Cutflow Rate (Co)* =	2.16 Acres 0.67 1.45 0.43 CFS
--	--	--	--	--

1	2	3	4	5	6	7
		Intensity	Col. #2	Inflow Volume	Outflow Volume	Storage Volume
Duration	Duration	(100-yr Storm)	x Col #3	Col. #4 x K1	Col. #2 x Qo	Col. #5 - Col. #6
(Minutes)	(Seconds)	(In/Hr)	(Inches)	(Cu. Ft.)	(Cu. FL)	(Cu. Ft.)
5	300	9.17	2750	3980	130	3850
10	600	7.86	4714	6823	259	6563
15	900	8.88	6188	8955	389	8566
20	1200	8.11	7333	10613	518	10094
30	1800	5.00	9000	13025	778	12247
60	3600	3.24	11647	16856	1555	15300
90	5400	2.39	12913	18688	2333	16355
120	7200	1.90	13855	19762	3110	16651
160 10800 1.34		14488	20967	4686	16301	

Note: Figures in Columns (3) and (4) are valid where the intensity is computed by the formula "1 = 275 / (t + 25)" (i.e. 100-yr Curve); appropriate revisions shall be made for geographical areas where this formula does not apply.

\*Allowable outflow rate Qo to be one of the following:
Case 1: Qo = capacity of existing discharge conduit or channel.
Case 2: Qo = q \* A where q = Permissible discharge rate per acre of tributary area = 0.2 cfs/Acre

Project: # 173290

2.16 Acres 0.67 0.6 0.43 CFS

t: Vff \* (1 / 24 hrs) \* (1 / 3500 sec) = Xff - LWL = Qff / (c \* SQRT(2 \* 32.2 \* Hff)) = Aff / 0.0055 =

1 1" Holes at Elevation = 919.94

Vneeded \* (1 / 24 hrs) \* (1 / 3600 sec) = Xbf - Xff = Qbf / (c \* SQRT(2 \* 32.2 \* Hbf)) = Abf / 0.0055 =

2 1" Holes at Elevation = 920.54

#### ALDI FOOD MARKET #51 DETENTION VOLUME CALCULATIONS

Elevation	Area (SF)	Average Area (SF)	Depth (FT)	Volume (CF)	Cumulative Volume (CF)
922,75	8,809				15,290
		7,980	0.75	5,985	
922	7,150				9,305
		6,695	0.5	3,348	
921.5	6,240				
921.5	4,840				5,958
	•	4,525	0.5	2,263	
921	4,210				3,695
		3,695	1.0	3,695	_
920	3,180	Existing Detention		15,290	o

levation	Area (SF)	Average Area (SF)	Depth (FT)	Volume (CF)	Cumulative Volume (CF)
921.5	1,400	1.240	0.5	620	1,470
921	1,080				850
920	620	850	1.0	850	0
	Exis	ting Sedimentation F	orebay Volume =	= 1,470	



(810) 227-9533 LAND SURVEYORS IGHTON, MICHIGAN 48114

						Sec. 11, 100 1-40, 4000	_
	REVISION#	DATE	REVISION-DESCRIPTION	REVISION#	DATE	REVISION-DESCRIPTION	ı
DESIGN:FAF	1	02-21-18	REVISED PER TOWNSHIP REVIEW COMMENTS				ı
DRAFT: SES							l
CHECK: CAG							ĺ

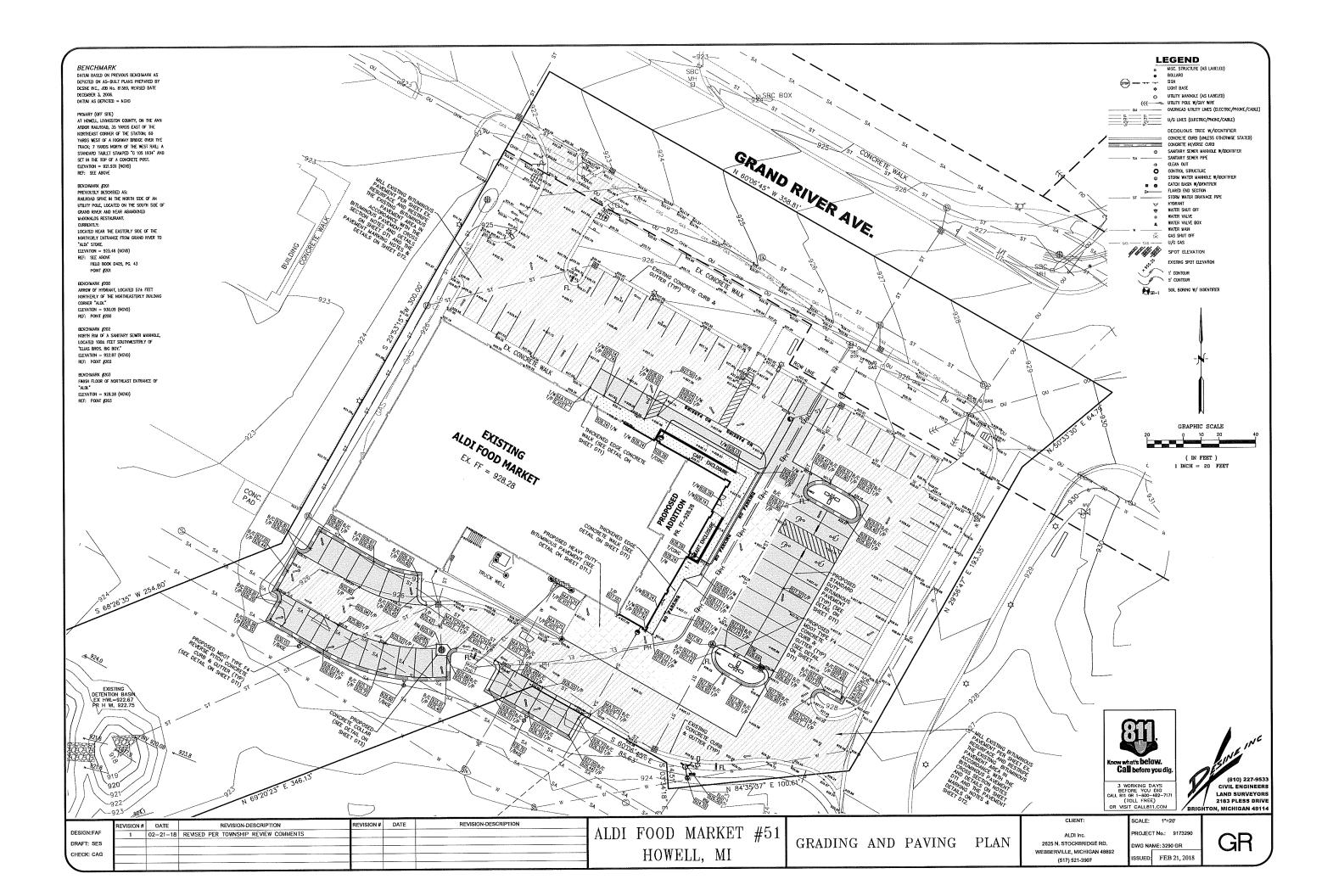
ALDI FOOD MARKET #51 HOWELL, MI

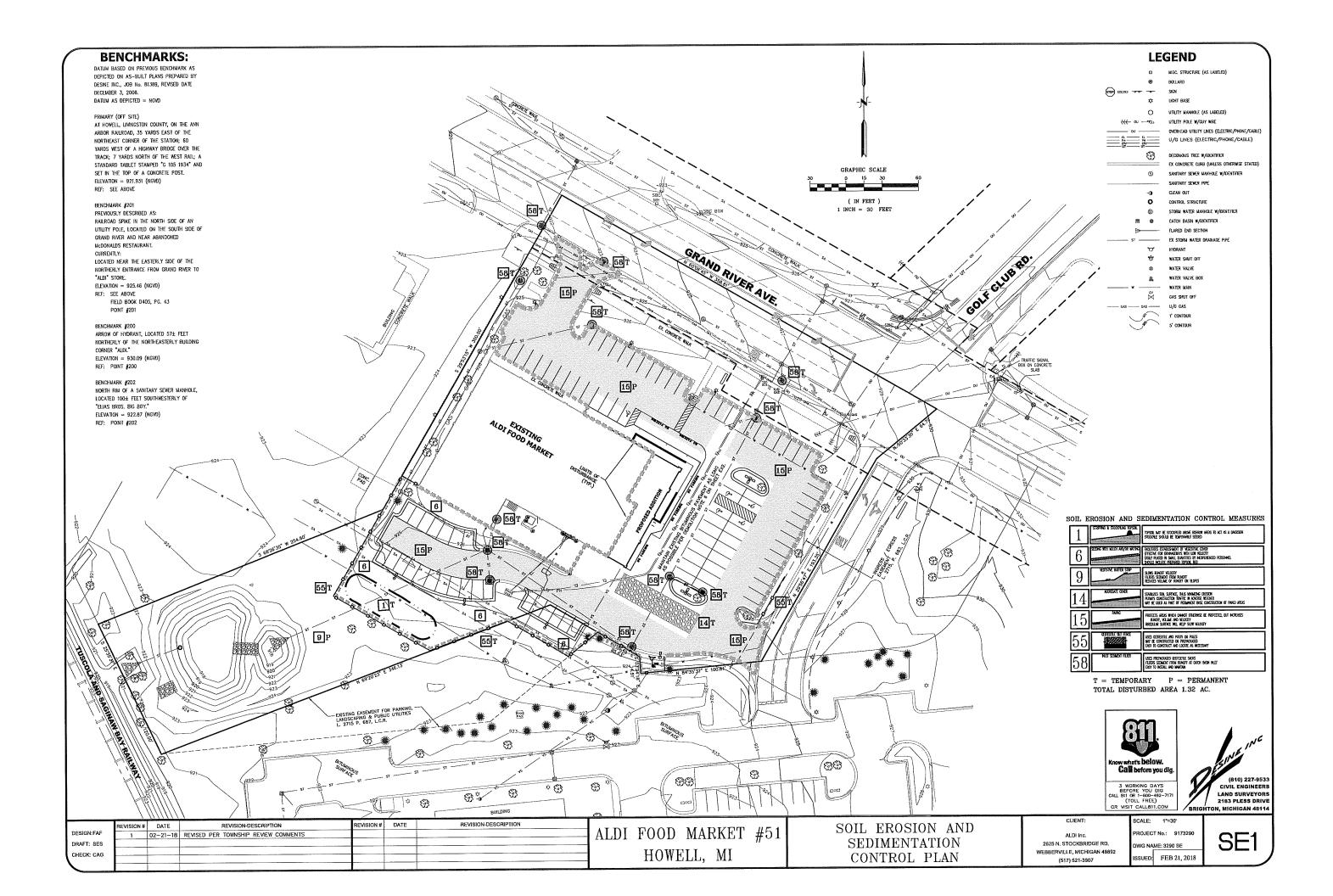
STORM WATER MANAGEMENT CALCULATIONS

ALDI inc. 2625 N. STOCKBRIDGE RD. WEBBERVILLE, MICHIGAN 48892 (517) 521-3907

CLIENT:

SCALE: NONE PROJECT No.: 9173290 SSUED: FEB 21, 2018





#### TIME LINE OF SOIL EROSION CONTROL AND CONSTRUCTION SEQUENCE

CONSTRUCTION & WORK							C	ΝC	ST	RΪ	IC.	ric	N	PE	RI	OE	)								
CATEGORIES*	Month		Ma	rcl	1		Ar	ril			M	ay		June				July				A	ug	us	t
	Week	1	2	3	4	1	2	3	4	1	2	3	4	1	2	3	4	1	2	3	4	1	2	3	4
1. OBTAIN PERMITS																									
2. INSTALL INITIAL SESC MEASURES									١	_					L			L							_
3. INSPECT & MAINTAIN SESC MEASU	RES																								
4. DEMOLITION WORK													L												
5. BUILDING CONSTRUCTION																									
6. UNDERGROUND UTILITY WORK								L						L					Ш				Ц		_
7. SITE LIGHTING WORK		_	_										L			L_									
8. CURB, SIDEWALK & PAVEMENT WO	RK												乚	L	_			L	L						L
9. BACKFILL & FINISH GRADE WORK								L					_	_	L										_
10. TOPSOIL, SEED & MULCH													L	L	_	L	_	L			L				_
11. LANDSCAPE WORK						_										L					L				_
12. REMOVE TEMPORARY SESC MEAS	URES																							لب	L

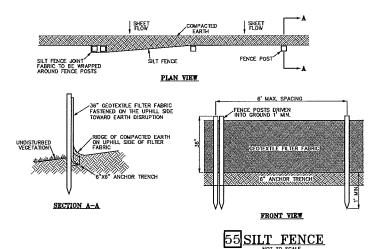
\*REFER TO THE MAJOR WORK ITEMS OUTLINED IN THE SOIL EROSION CONTROL AND CONSTRUCTION SEQUENCE NOTES.

# Property/2

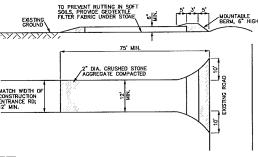
#### SOILS MAP

#### Map Unit Legend

Map Unit 3	probol Map Unil Name	Acres in AQ	Percent of AOI	
BLA	Boyer-Oshterno loanny sunds, 010 2 per cent stopres	1.8	56.8%	
BE	Boyer-Oshterno losmy sends. 18 to 25 percent slopes	1.4	42.0%	
NoE	Marni karn, 16 to 25 percent stopes	0.0	1.2%	
Totals for Area of	Interest	3.2	100.0%	

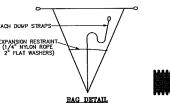


- 1. REPAIR AND REPLACE SILT FENCE AS NEEDED, INCIDENTAL.
- 2. FIELD LOCATE SILT FENCE TO FOLLOW CONSTANT CONTOUR ELEVATIONS.
- 3. DVERLAP FENCES AT JOINTS. 4. INSTALL FILTER BERM AT LOW POINTS WHERE INDICATED ON PLANS.



#### 14 MUD TRACKING CONTROL DEVICE

NOTE:
WHEN ACCEPTABLE TO ENGINEER, CONTRACTOR MAY INSTALL
STONE BELOW THE SUBGRADE ELEVATION; THUS STONE MAY
BE LEFT IN PLACE BELOW PAVEMENT.



58 INLET SEDIMENT FILTER



SOIL EROSION CONTROL AND CONSTRUCTION SEQUENCE:

Obtain all necessary Soil Erosion and Sedimentation Control related permits from the appropriate Local, County and/or State Agencies. Refer to the General Notes on the project plans for additional requirements.

Prior to commencement of any earth disruption, install Mud Tracking Control Devices and Inlet Sedims. Filters at existing storm sewer catch basins in accordance with the Soil Erosion and Sedimentation Control Plan and the Soil Erosion and Sedimentation Control Permit.

Inspect and maintain all Soil Erosion Control Measures daily. Maintain all Soil Erosion Control Measures as necessary and as directed by the Engineer and/or the Permitting Agency.

Perform demolition work. Install appropriate Soil Erosion Control Measures in accordance with the Soil
Erosion and Sedimentation Control Plan and/or as directed by the Engineer and/or the Permitting Agency.

 Construct building in accordance with the Site Plan and Architectural Plans. Install appropriate Soil
Erosion Control Measures in accordance with the Soil Erosion and Sedimentation Control Plan and/or as directed by the Engineer and/or the Permitting Agency.

6. Construct underground utilities including roof drains and underdrains. Install appropriate Soil Erosion Control Measures in accordance with the Soil Erosion and Sedimentation Control Plan and/or as directed by the Engineer and/or the Permitting Agency.

7. Install light pole bases and fixtures and underground electric. Install appropriate Soil Erusion Control Measures in accordance with the Soil Erusion and Sedimentation Control Plan and/or as directed by the Engineer and/or the Permitting Agency.

8. Construct curb & gutter, sidewalk and paved parking and roadway areas. Install appropriate Soil Erosion Control Measures in accordance with the Soil Erosion and Sedimentation Control Plan and/or as directed by the Engineer and/or the Permitting Agency.

9. Backfill curb and sidewalks and finish grade all disturbed areas outside of pavement areas, Install appropriate Soil Erosion Control Measures in accordance with the Soil Erosion and Sedimentation C Plan and/or as directed by the Engineer and/or the Permitting Agency.

10. Place topsoil, seed, and mulch within 5 days of finish grade for establishment of vegetative ground cover outside of pavement and decorative stone landscape bed areas. Install appropriate Soil Erosion Control Measures in accordance with the Soil Erosion and Sedimentation Control Plan and/or as directed by the 11. Landscape site in accordance with the Project Landscape Plan. Install appropriate Soil Erosion Control Measures in accordance with the Soil Erosion and Sedimentation Control Plan and/or as directed by the

Freineer and/or the Permitting Agency. 12. Following establishment of sufficient vegetative ground cover and receipt of approval from the Permitting Agency, remove all temporary Soil Erosion Control Measures, clean all storm sewer structures, and repair any permanent Soil Erosion Control Measures as directed by the Engineer and/or the Permitting

#### SOIL EROSION AND SEDIMENTATION CONTROL NOTES:

The Soil Erosion and Sedimentation Control Specifications of the appropriate Local, County and/or State Agencies are a part of this work. Refer to the General Notes on the Project Plans for additional

2. The Soil Erosion and Sedimentation Control (SESC) Permit Holder shall be responsible for compliance with the SESC Permit requirements for the duration of the project and until receipt of final approval from the Permitting Agency. For any site with an earth disturbance area of 1 acre or greater, the SESC Permit Holder shall retain a Certified Storm Water Operator in accordance with the SESC Permit requirements. The Certified Storm Water Operator shall perform routine inspections of the site and the SESC measures and file inspection reports in accordance with the SESC Permit requirements. For any site with an earth disturbance area of 5 acres or greater, the SESC Permit Holder shall file a National Pollutant Discharge Elimination System (NPDES) Notice of Coverage Form with the State DEQ prior to any earth disruption.

3. The Contractor shall install the appropriate Soil Erosion Control Measures in accordance with the Project Plans prior to massive earth disruption, including but not limited to; mud tracking control mats and sediment filters on existing stoms sewer structures. Demolition work may be necessary prior to installation of some soil ensoin control measures. In such eases, postpone installation of effected soil erosion control measures until immediately following demolition work. Refer to the Project Plans and the Soil Erosion Control and

4. The Contractor shall schedule work so as to minimize the period of time that an area is exposed and disturbed. The Contractor shall observe the grading limits and limits of disturbance in accordance with the Project Plans. The Contractor shall maintain an undisturbed vegetative buffer around the work when shown on the Project Plans.

5. The Contractor shall install and maintain Soil Erosion Control Measures in accordance with the Project Plans during the appropriate phases of construction. The Project Plans show the minimum requirements fo Soil Erosion Control Measures. The Contractor shall install additional Soil Erosion Control Measures as necessary due to site conditions and as directed by the Permitting Agency and/or Engineer. The Contractor shall perform noutine inspection and maintenance of all Soil Erosion Control Measures to ensure complians with the permit requirements and proper operation of the Soil Erosion Control Measures.

6. All disturbed areas outside of paved areas shall be restored within 5 days of finish grading. Proposed vegetative areas shall be restored with a minimum of 3-inches of topsoil, then seeded and mulched, unless noted otherwise on the Project Plans. During the non-growing season, temporary stabilization shall be provided using straw matting or as directed by the Permitting Agency and/or the Engineer.

7. Following complete site restoration and stabilization; sediment shall be removed from all storm sewer A Following complete as the resordance and Standardship, seculiaria shall be Under the day as a soft as a standard structure, passed areas and storm basins. The SESC Permit Holder shall contact the Permitting Agency to request closure of the SESC Permit. For any site with an earth disturbance area of 5 acres or greater, the SESC Permit Holder shall fine in APDES Notice of Termination Form with the State DEQ.

SOIL EDOSION AND SEDIMENTATION CONTROL MEASURES

SOIL EROSION AND S	REDIMENTATION CONTROL MEASURES
1 STREFAGE & STOOPLING REPORT	TOPSOL MOT BE STOOPELD ASONE BORROW AFEAS TO ACT AS A DALESSON STOOPFILE SHOULD BE TEMPORPALY SELECED
gatine and nation wolcon major	Facilities educionant of videlum cover official for devageous with low belong Dealy filed of Nami, owners of measurement presentel scolo bouch freshard resol hid
9 VERTANE BUTTER STOP	Sons ructy vidoty ners sement from ructy reducts vidure of racify on slopes
14	SURLEDES SOIL SUPPLIE, BUSINESSE BEGEN HOMES CONSTRUCION EMPTE IN ANAISSE BELIEFEZ MOT SE LISED AS PART OF PERSAMBOT BASE CONSTRUCTION OF PAND AFEAS
15	PROTECT MEAS WHICH CHANGE GREATERS BE PROTECTED, BUT MODIFIES EMERY, VOLUME MIC VELOCITY PRESELUM SURVAE WILL HALP SLOW YELLOCKY
55	ues generie, ac most de rols no el conselted de prepionado est to conselte no legge, as nedesant
58	CONTRACTOR OF STREET STORE OF STREET

T = TEMPORARY P = PERMANENTTOTAL DISTURBED AREA 1.32 AC.

#### MAINTENANCE NOTES FOR SOIL EROSION CONTROL MEASURES:

The Construction Site and all Soil Erosion Control Measures shall be inspected periodically The Construction Site and all Soil Erosion Control Measures shall be inspected periodically in accordance with the appropriate local runsiriopality/authority and the MDEQ NPDES rules and regulations. At a MINIMUM, inspections shall be performed once a week and within 24 hours following a storm event resulting in 1° or installal to greater. Inspections shall be performed throughout the duration of the construction process and until the site is completely stabilized. Following construction, the owner (or its assignes) shall periodically inspect all permanent soil erosion control measures to ensure proper operation.

BASIN PERFORATED STANDPIPES / CONTROL STRUCTURES: Standpipes shall be inspected for soil accumulation, soil caking and mechanical failure/damage. The filter stone around the standpipe shall be removed and replaced each time it becomes clogged with sediment. All mechanical failure/damage shall be repaired immediately.

CATCH BASINS: Catch basins shall be inspected for accumulation of solids and sediment Solids and sediment shall be removed from the catch basins by vacuum or adductor cleaning. Cleaning should be performed before the catch basin sumps are half full.

MUD TRACKING CONTROL DEVICE / CONSTRUCTION ACCESS: Mud tracking control devices shall be inspected for significant mud accumulation and to ensure the access is not croding into public rights of way or drainage features. Add additional layers of stone or remove and replace stone each time the stone becomes covered with mud. All sediment ropped or eroded onto public rights of way shall be removed immediately. Sweeping of the public rights or way and/or paved access route shall be performed as necessary to maintain the access route free of sediment and debris.

DETENTION BASIN (WET BOTTOM): Wet bottom detention basins shall be inspected to DETENTION BASIN (WET BOTTOM): Wet bottom detention basins shall be inspected to ensure erosion is not occurring along the inlet locations, banks and/or bottom of the basin and for sediment and/or algae accumulation. Regular maintenance of the basin includes routine mowing of the buffer/filter strip and side slopes and removal of litter and debris accumulation. Address vegetation and/or ensoin concerns as soon as weather permits. Remove sediment from basin every 5 to 10 years or sooner if sediment accumulation adversely affects the operation of the basin. Sediment that is removed shall be disposed of offsite or at an upland area and stabilized so that it does not re-enter the drainage course. Excessive algae shall be removed as necessary to prevent odors and to maintain nutrient removal carerity.

SEDIMENTATION BASINS: Sedimentation hasins shall be inspected to ensure crusion is not occurring along the inlet locations, banks and/or bottom of the basin and for piping, seepage, sediment accumulation and/or other mechanical damage. Regular maintenance of the basin includes routine mowing of the buffer/filter strip, side alopes and basin floor and memoral of litter and debris accumulation. Address vegetation and/or erosion concerns as soon as weather permits. Sediment shall be removed before it accumulates to 50% of the dasign depth of the basin. Sediment that is removed that be disposed of offsite or at an upland area and stabilized so that it does not re-enter the drainage course.

RIPRAP: Inspect riprap immediately following the first rainfall event following installation of the riprap. Continue to perform inspections of the riprap at each periodic site inspection. Riprap shall be inspected to ensure erosion is not occurring within and/or around the riprap. The discharge point shall be inspected to ensure that concentrated flows are not causing erosion downstream. Displaced riprap shall be removed from downstream locations and the riprap beds shall be repaired or replaced. Significant sediment buildup shall be removed from riprap beds. Repair or replace failing or displaced riprap immediately. Address vegetation and/or erosion concerns as soon as weather permits.

SEEDING: Newly seeded areas shall be inspected until substantial vegetative growth is obtained. Seeded areas shall be inspected to ensure crosion is not occurring in the seeded area and vegetative growth is promoted. Eroded areas shall be finish graded as necessary to removal erosion channels or gulleys and new seed placed as soon as weather permits.

SILT FENCE: Silt fencing shall be inspected for soil accumulation/clogging, undercutting, overtopping and sagging. Soil accumulation shall be removed from the face of the silt fence ach time it reaches half the beight of the fence. Removed sediment shall be disposed of in a stable uphand site or added to a spoils stockpile. When undercutting occurs, grade out mas of concentrated flow unstream of the silt fence to remove channels and/or gulleys and areas of concentration now upstucen in the star factor of tensor constitute and gardy's repair or replace silt fence ensuring proper trenching techniques are utilized. Silt fencing, which sags, falls over or is not staked in shall be repaired or replaced immediately. Silt fencing fabric, which decomposes or becomes ineffective, shall be removed and replaced with new fabric immediately. Silt fencing shall be removed once vegetation is well established and the up-slope area is fully stabilized

SOD: Newly sodded areas shall be inspected to ensure sod is maturing. Sod shall be inspected for failure, erosion or damage. Slipping or eroding sod on steep slopes shall be immediately repaired or replaced and staked in place. Damaged or failed sod shall be immediately replaced.

SPILLWAYS: Spillways shall be inspected to ensure that erosion is not occurring within and/or around the spillway. The discharge point shall be inspected to ensure that concentrated flows are not causing erosion downstream. Inspect the spillway for cracked concrete, uneven and/or excessive settling and proper function. Repair or replace failing spillways immediately. Address vegetation and/or erosion concerns as soon as weather

STOCKPILES: Temporary and permanent topsoil and spoils stockpiles shall be seeded to promote vegetative growth. Stockpiles shall be inspected to ensure excessive erosion has not occurred. When runoff or wind erosion is evident, reduce the side slopes of the stockpile or stabilize the stockpile with pieces of stakes ob laid perspectional ro the slope. When filter fencing is used around a stockpile, the fencing shall be inspected to ensure piping has not occurred under the fencing and to ensure the fencing has not collapsed due to soil slippage or access by construction equipment. Repair or replace damaged fencing immediately. Berms at the base of stockpiles, which become damaged, shall be replaced.

STORM STRUCTURE INLET FILTER: Inlet filters shall be inspected for sediment SIORM SIRCUTURE HILLS HILLS men inters and re inspect on a seminein securulation, clogging and damage. When stone is used in conjunction with inleft filter fabric, replace the stone each time it becomes clogged with sediment. Clean or replace the inleft filter fabric each time it becomes clogged with sediment. Existal or replace fallen filter fabrics immediately. Replace damaged filter fabrics immediately.

NOTE: SEE LANDSCAPE PLAN FOR SEED AND SOD



(810) 227-953 CIVIL ENGINEERS 2183 PLESS DRIV BRIGHTON, MICHIGAN 48114

FEB 21, 2018

SUED

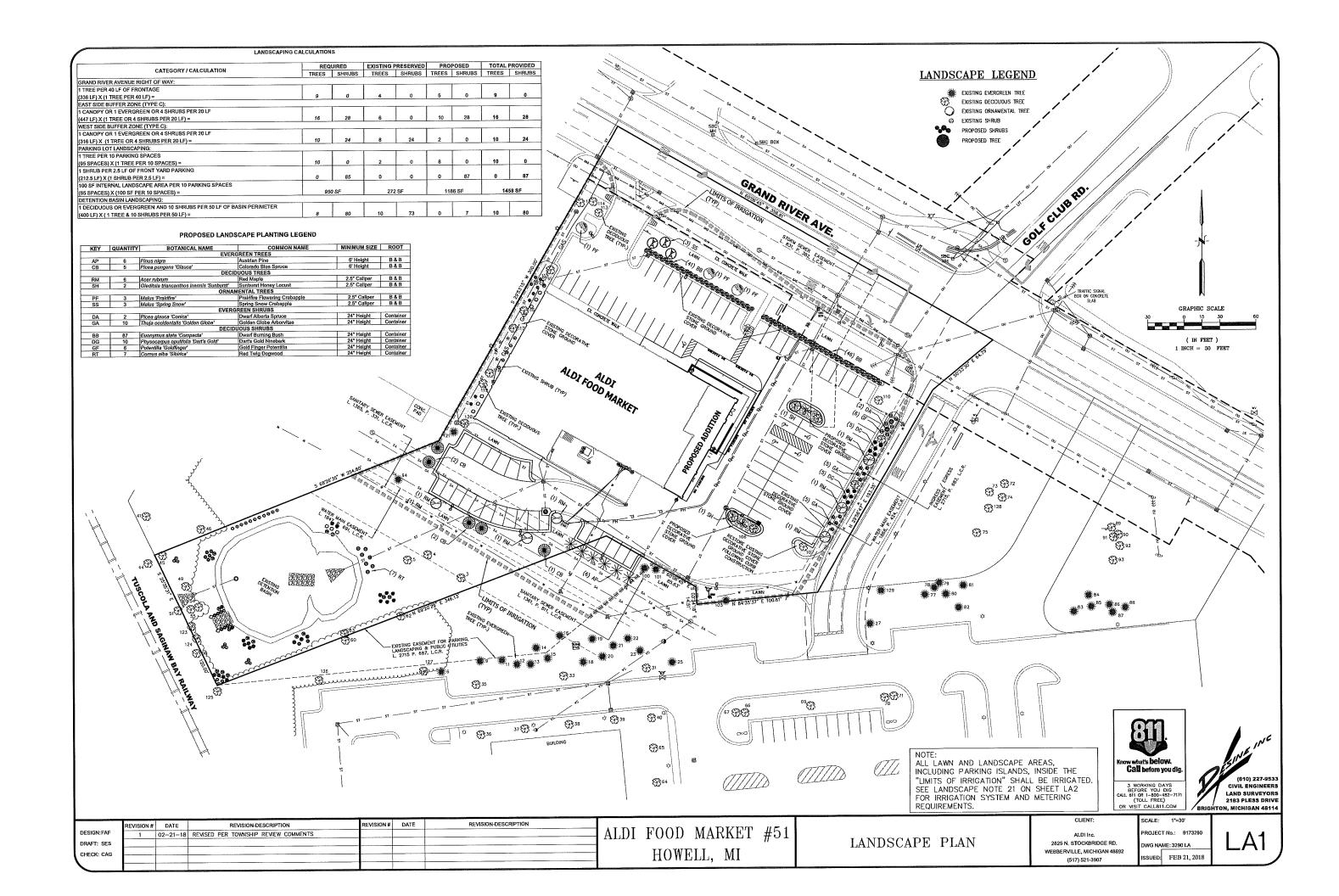
OR VISIT CALLBIT.COM CALE: ROJECT No.: 9173290 WG NAME: 3290 SE

REVISION-DESCRIPTION EVISION# DATE REVISION-DESCRIPTION EVISION # DATE ALDI FOOD MARKET #51 DESIGN: FAR 02-21-18 REVISED PER TOWNSHIP REVIEW COMMENTS DRAFT: SES HOWELL, MI CHECK: CAG

SOIL EROSION AND SEDIMENTATION CONTROL NOTES & DETAILS

2625 N. STOCKBRIDGE RD. WERBERVILLE, MICHIGAN 48892 (517) 521-3907

SE2



# REINFORCED RUBBER HOSE AT FIRST BRANCHING 2" X 2" B"-0" TREATED STAKE FOR TREES LESS THAN 3" CAL.; 2 OPPOSITE 1 #12 WIRE NEW GALVANIZED TREE WRAP TO SECOND BRANCHING 3 1/2" X 7 GALV. TURNBUCKLES 1 EACH GUY WRE FOR TREES 3" CAL & OVER 3-2" X 2" X 30" TREATED STAKES; DRIVE FLUSH WITH GRADE HIGH FARTH SAUCER, TYP. TREE AND SHRUB BED TO RECEIVE 4" DEPTH SHREDDED BARK HULCH REMOVE BURLAP & TIES FROM TOP 1/3 OF BALLED TREES AND SHRUBS UNDISTURBED EARTH

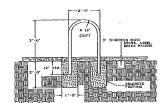
Easy to install, environmentally safe, nontoxic, and available in unlimited colors in matte or high-gloss finish. S&G Bike Racks are built to withstand the rigors of hard outdoor use yet aesthetically pleasing in any environment. Our bike racks are made from structural grade steel pipe coated with durable, weather-resistant PVC.

Material: 1 1/2 Schedule 40 1.90 O.D. Coating: PVC Thermoplastic over Thermoset Vinyl Prime

MAINTENANCE-FREE BIKE RACKS

Excellent Resistance to: Mildew Acid Abrasion Alkali Impact Installation Options SURFACE MOUNT IN GROUND MOUNT

Secured with two steel flange bases and three anchors at each base.



Model No. 2447-S Model No. 2447 How to Specify Bike Racks

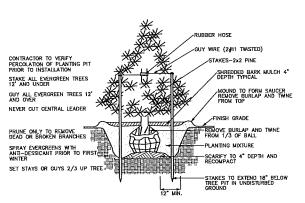
Bike racks to be Model No. 2447-S as manufactured by S&G Products, Dexter, Michigan (517-546-9240). 1 14 SCHD 40 PIPE (1.90" O.D.), rolled in the shape of an Inverted "U" to a 12" outside radius standing 3' high with less parallel in both planes. Mounting flanges t have minimum three holes for attachment. Coating to vinyl primer to ensure a complete crosslink bond. Thickness to be a minimum of 100 mils. Color selection

Bike racks to be Model No. 2447 as manufactured by S&G Products, Dexter, Michigan (517-546-9240). 1 1/1 " SCHD 40 PIPE (1.90° O.D.), rolled in the shape of an with legs parallel in both planes. Coating to be polyvinyl chloride thermoplastic over a thermoset viny primer to ensure a complete crosslink bond. Thickness to be a minimum of 100 mils. Color selection by owner



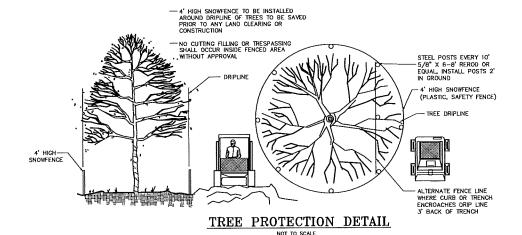
S & G Products
2055 N. Lima Center Rd, Dexter, Michigan 48130
PO Box 615, Manchester, Michigan 48158
Phone (517)546-9240 Fax (517)546-9720
Email: <u>sales@sqfabricators.com</u>
Website: sandgproducts.com

SEE SHEET SP FOR PROPOSED BIKE RACK LOCATIONS



TYPICAL TREE/SHRUB/PERENNIAL PLANTING

TYPICAL EVERGREEN TREE PLANTING



LANDSCAPING NOTES

EXISTING TREE SCHEDULE:

EXISTING TREE SCHE

3 18" ELM

4 24" COTTONWOOD

5 18" COTTONWOOD

6 9P INE

9 8" PINE

11 9" PINE

12 8" PINE

13 9" PINE

14 8" PINE

15 9" PINE

16 8" PINE

17 9" PINE

18 8" PINE

19 9" PINE

20 9" PINE

21 10" PINE

21 10" PINE

22 10" PINE

23 6" PINE

24 6" PINE

25 6" PINE

26 6" PINE

27 6" PINE

31 12" CRABAPPLE

36 8" CRABAPPLE

37 9" CRABAPPLE

38 9" CRABAPPLE

39 9" CRABAPPLE

39 9" CRABAPPLE

30 9" CRABAPPLE

30 9" CRABAPPLE

31 9" CRABAPPLE

31 9" CRABAPPLE

32 9" CRABAPPLE

34 9" CRABAPPLE

35 9" CRABAPPLE

36 9" CRABAPPLE

37 12" CRABAPPLE

38 9" CRABAPPLE

39 9" CRABAPPLE

30 9" CRABAPPLE

30 9" CRABAPPLE

41 10" BOX ELDER

45 11" BOX ELDER 6 12\* TWIN BOX ELDE 47 12" TWIN BOX ELDER

8 24" TWIN LOCUST

50 18" LOCUST - DEAD

6 13" TWIN BOX ELDER

60 12" POPLAR - DEAD

51 12\* POPLAR 52 12" POPLAR

4 20" POPLAR

55 22" POPLAR

62 10" APPLE

63 19" ASPEN

54 7" CRABAPPLE

65 7" CRABAPPLE 6 3" CHERRY

67 3" CHERRY 69 3" CHERRY

70 4" CHERRY 4 CHERRY

2 9" CRABAPPLE 3 5" CRABAPPLE

72 9' CRABAPPLE
74 9' CRABAPPLE
77 7' PINE
77 8' REPINE
78 8' PINE
80 8' PINE
81 8' PINE
81 8' PINE
82 6' FINE
83 6' TWIN PINE
85 9' PINE
85 9' PINE
86 9' PINE
87 9' PINE
87 9' PINE
88 9' 6' CRABAPPLE
91 6' CRABAPPLE
91 6' CRABAPPLE
92 6' CRABAPPLE
93 6' CRABAPPLE
94 6' PINE
95 6' PINE (IBR)
96 6' PINE (IBR)
97 6' PINE (IBR)
98 6' PINE (IBR)

99 6" PINE (TBR)

102 6" SPRUCE (TBR)

104 5" PEAR (TBR)

107 4" PEAR 108 5" GINKO

110 6" PEAR 111 4" LOCUST (TBR)

112 3" LOCUST 113 6" PEAF

113 6" PEAR 114 6" PEAR 115 5" PEAR 116 6" PEAR 116 6" PEAR 118 6" PEAR 120 6" PEAR 120 6" PEAR 121 6" SPRUCE (TBR) 121 6" SPRUCE (TBR) 123 11" TRIPLE DAK 125 9" TWIN OAK 125 9" TWIN OAK 127 12" TWIN OAK 127 12" TWIN OAK 127 12" TWIN OAK

105 4" LOCUST (TBR) 106 4\* LOCUST

- 1. All minimum planting sizes specified on the Project Plans shall be at the time of planting.
- All landscape materials shall be as specified on the Project Plans or approved equal. Substitutions shall not be made without prior written approval from the Project Engineer and receipt of the Owner's Authorization.
- 3. All plant material shall be free of disease and insects and shall conform to the American Standard of Nursery Stock of the American
- 4. All landscape plantings shall be planted and maintained in a healthy condition and shall be guaranteed by the Landscape Contractor and/or Supplier for a minimum period of 1 year from the time of planting. Any plantings that die or become diseased guarantee period shall be removed and replaced by the Landscape Contractor and/or Supplier at no cost to the Owner.
- 5. Excavations for container or balled plantings shall be no deeper than the root ball or container and shall be at least twice the
- 6. Excavations for bare root plantings shall be no deeper than the longest roots and shall be at least twice the diameter of the root
- 7. The sides of planting excavations in beavy and/or wet soils shall be scarified with a fork, pick or shovel to eliminate glazing.
- 8. Landscape planting backfill shall consist of a prepared mixture of peat moss, composted manure and topsoil or suitable excavated native soil material mixed with the appropriate soil conditioners that are compatible with the native soil and plant species. The type and mixture ratio of soil conditioners shall be in accordance with the Landscape Supplier's recommendations.
- 9. The Landscape Contractor shall stake and reinforce all trees to prevent wind damage. The Landscape Contractor shall remove all tree reinforcement and stakes upon expiration of the guarantee period.
- 10. Perennials shall be planted on a 3" minimum bed of prepared peat moss, composted manure and topsoil mixture
- 11. Landscape beds shall be separated from lawn areas with landscape edging. Landscape edging shall be black heavy-duty polyethylene type with UTV protection and a double V-lip bottom edge to prevent frost heave. Landscape edging shall be staked in accordance with the Manufacturer's recommendations to prevent frost heave. Landscape edging shall be installed in strict accordance with the Manufacturer's specifications and recommendations.
- 12. Ground cover within landscape beds shall be decorative stone. Decorative stone shall be 2" to 4" diameter washed river rock
- 13. Ground cover within landscape beds shall be placed over a landscape fabric weed barrier. Landscape fabric shall be non-woven, oz. per 50, yd. minimum weight, with UV protection. Landscape fabric shall be installed in strict accordance with the Manufacturer's oz. per sq. yd. minimum weight, with UV protection. Landscape fabric shall be installed in strict accordance with the Manu specifications and recommendations. Landscape fabric shall not be installed over or within 12 inches of perennial plantings.
- 14. Lawn areas shall be established with 3" minimum depth of prepared topsoil and hydroseed. The Landscape Contractor shall guarantee all lawn areas for a minimum period of 1 year from time of seeding. All lawn areas that do not take root or die during the guarantee period shall be re-hydroseeded as appropriate by the Landscape Contractor at no cost to the Owner. All lawn areas that become diseased during the guarantee period shall be removed and re-hydroseeded as appropriate by the Landscape Contractor at no
- 15. Topsoil shall be a dark, organic, natural surface soil free of clay lumps, peat, muck, subsoil, notious weeds and other foreign material such as roots, sticks and rocks over ½" diameter. Topsoil shall not be frozen or muddy. All earthen areas to receive tops shall be finish graded and properly trimmed. Topsoil shall be spread on the prepared areas to a depth of 3 inches. After spreading any large clock and fumps of topsoil shall be broken up and pulverized. Stones and rocks over ½" in diameter, rolliter and all foreign matter shall be raked up and disposed of by the Landscape Contractor. Seed and mulch shall be placed within 5 days of
- 16. Seed mixture for lawn areas shall consist of 10% Kentucky Blue Grass, 20% Perennial Rye Grass, 30% Hard Fescue and 40% Creeping Red Fescue. Hydroseed shall be placed within 5 days of topsoil placement and shall be placed to provide complete and uniform coverage. Fertilizer shall be placed at 80 pounds per acre, bydro mulch at 1,200 pounds per acre and water at 500 gallons per acre unless otherwise specified by the Seed Distributor/Manufacturer. All over spray areas shall be properly cleaned and restored at
- 17. Seed and mulch may be substituted for hydroseed when authorized by the Owner. Seed mixtures shall meet the require 11. Seed and mutter may be substituted for a purposeed when nationized by the Owner. Seet influences and meet an equationized soft laws areas as suitlined above. Seed shall be uniformly applied at a rate of 72.0 lbs per acre unless otherwise recommended by the seed Distributor/Manufacturer. Seed mixture shall be fertilized. Fertilizer shall be uniformly applied at of 240 pounds per acre of chemical fertilizer nutrients in equal portions (10-10-10) of Nitrogen, Phosphoric Acid and Potash.
- 18. All seeded areas with a stope less than 1:4 shall be stabilized with straw mulch placed at 2 tons per acre unless otherwise recommended by the seed Distributos/Manufacturer. Ensoin control blankets shall be substituted for straw mulch in roadway green-belts, lawn areas adjacent to heavy traffic, lawn areas analyzed to high winds, slopes of 1:4 or greater and withiches, swales and other areas exposed to concentrated overland storm water flow. Ensoin control blankets shall consist of 100% straw fiber matrix with photodegradable polypropylene netting and have a 12-month minimum longerivity rating. Ensoin control blankets shall be pinned with biodegradable pinu and shall be installed in accordance with the Manufacturer's recommendations.
- 19. Sod shall only be utilized where specified on the project plans. (Sod may be substituted for hydrosced when required by the Municipality or if necessary for site stabilization late in the growing season. Sod shall not be substituted without receipt of the Owner's Authorization.) Sod shall be a drought tolerant species consisting primarily of Pine Leafed Fescues including Red Fescue, Chewings Fescue and Hard Fescue with Kentucky Bluegrass filler for hardiness. Sod shall be placed on a prepared subgrade. Subgrade shall be finish graded and tilled to a depth of 4" to 6". All foreign material, roots, sticks, large soil clumps and rocks over 2" diameter shall be removed from the subgrade. Sod shall not be placed on from a resturated subgrade. Fertilizer, line and/or compost shall be placed over the prepared subgrade in accordance with the Sod Supplier/Manufacturer's recommendations. Sod shall be be placed in accordance with the Sod Supplier/Manufacturer's recommendations. Sod shall be installed with biodegradable stakes on slopes of 1:4 or greater and within ditches, swales and other areas exposed to concentrated overtand storm water flow. All sod shall be planted and maintained in a healthy condition and shall be guaranteed by the Landscape Contractor and/or Supplier for a minimum period of 1 year from the time of planting. Any sod that dies or become diseased during the guarantee period with the removed and period of I year from the time of planting. Any sod that dies or become diseased during the guarantee period shall be removed and aced by the Landscape Contractor and/or Supplier at no cost to the Owner.
- 20. The Landscape Contractor shall be responsible for watering non-irrigated plantings and sod during dry weather conditions throughout the guarantee period as necessary to promote growth and establishment.
- 21. The existing irrigation system shall be modified as a part of this project. The existing irrigation system shall be inspected and tested to determine the limits of irrigation and condition of the irrigation system. The irrigation system shall be modified as necessary to excommodate the proposed site improvements and to provide irrigation to all laws and landscape areas within the limits of irrigation system shall be modified as necessary to excommodate the proposed site improvements and to provide irrigation to all laws and landscape areas within the limits of irrigation system shall be repaired or replaced as necessary. All existing sprinker heads that are to remain shall be adjusted as necessary for proper operation and coverage. The Contractor shall submit an irrigation system design and shop drawings to the Owner for review and approval prior to installation. Irrigation systems shall be designed to utilize the minimum amount of water necessary to provide sufficient irrigation, satisfy the Local Municipal requirements and site conditions and shall include a rain sensor. A separate water metic participating and/or water value rain sensor, a separate water metic plant in a string shall be installed as a part of the irrigation system modification to allow for reduced rate metering by the Local Municipal requirements and be responsible for coordinating installation of irrigation lines, sleeves, plumbing connections, controls and apputremances at the appropriate stages of construction. All existing irrigations lines and systems that are to remain that are cut, plugged, spliced, damaged and/or otherwise modified during demolition and/or construction activities shall be properly repaired, replaced, reconnected and/or adjusted as necessary to ensure proper operation.
- 22. All existing on-site trees shall be trimmed / pruned as directed by ALDI Inc.





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	REVISION#	DATE	REVISION-DESCRIPTION	REVISION #	DATE	REVISION-DESCRIPTION
DESIGN:FAF	1	02-21-18	REVISED PER TOWNSHIP REVIEW COMMENTS			
DRAFT: SES						
CHECK: CAG						
CHECK: CAG						

ALDI FOOD MARKET #51 HOWELL, MI

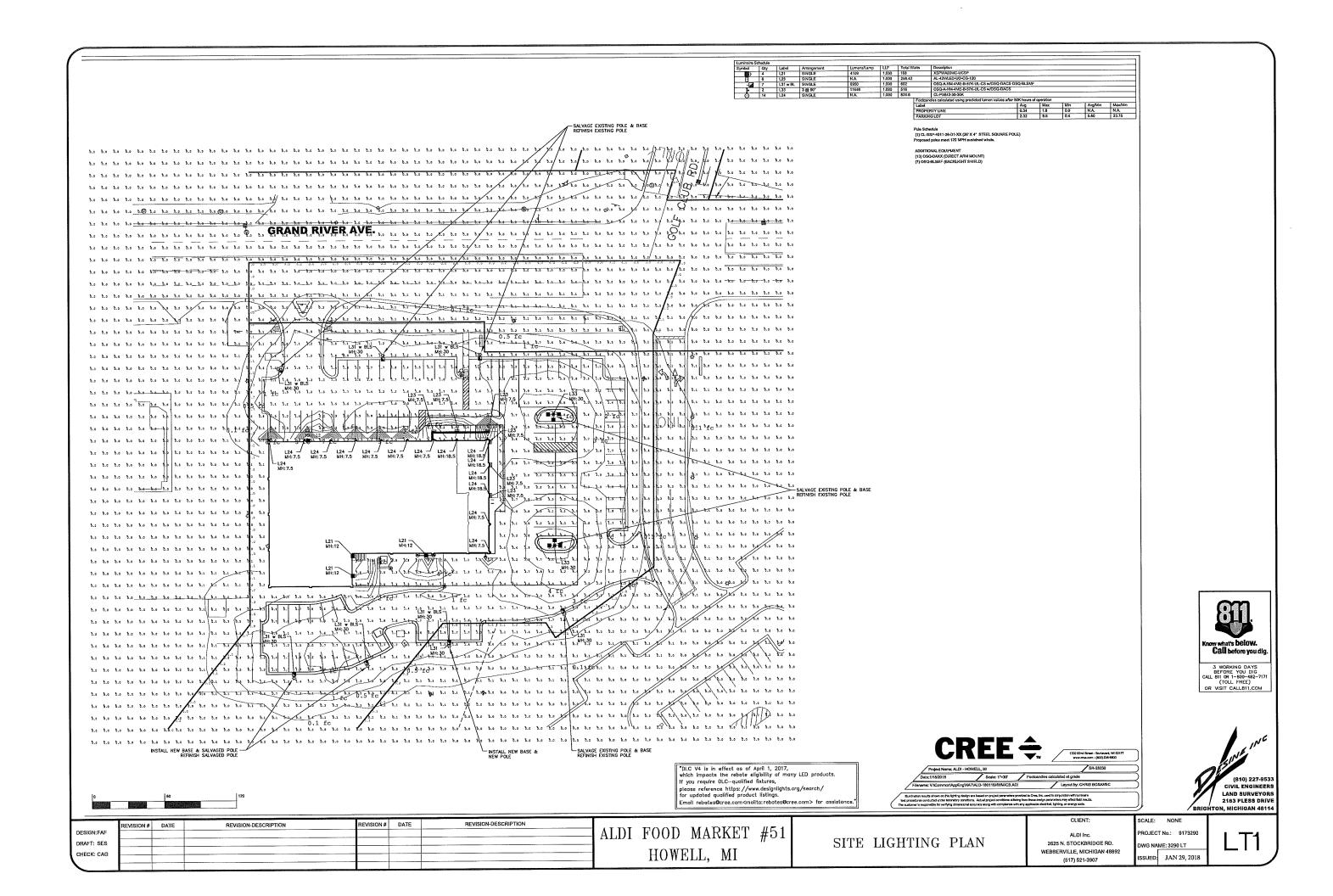
LANDSCAPE NOTES & DETAILS CHENT

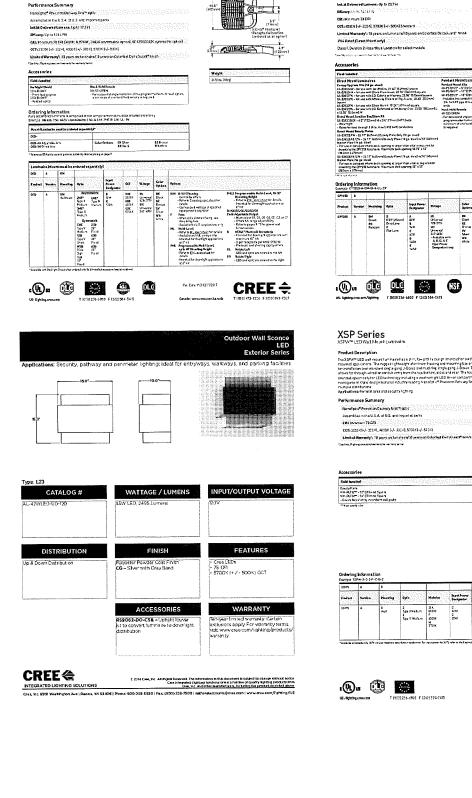
ALDI Inc 2625 N. STOCKBRIDGE RD. WEBBERVILLE, MICHIGAN 4889 (517) 521-3907

SCALE: AS NOTED PROJECT No.: 9173290

WG NAME: 3290 LA

SSUED: FEB 21, 2018





REVISION-DESCRIPTION

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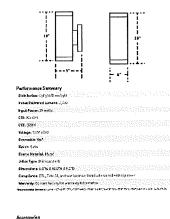
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#### SITE LIGHTING NOTES:

- All existing on-site pole mounted light fixtures are to be removed in accordance with the project plans. The
  existing light poles and bases are to be salvaged where feasible. The existing electric supply is to be salvaged
  unless noted otherwise on the Site Electrical Plan.
- 2. All existing light poles shall be refinished. Remove all existing rust, scale, chipped and/or peeling finish. Prepare the surface, prime and paint all light poles in accordance with the Project Manual painting specifications and requirements for exterior finishes. Paint color shall be slate gray.
- The proposed pole mounted light fixtures shall be mounted onto the existing light poles in accordance with the light fixture manufacturer's recommendations. Any new mounting holes shall be drilled and all other necessary modifications to the existing poles shall be performed prior to refinishing of the existing poles.
- 4. Existing exterior building mounted light fixtures shall be removed and replace in accordance with the project plans. See the building plans for additional information and specifications.
- 5. Contact the ALDI National Account Lighting Distributor to order fixtures, mounting accessories and pole(s):
  Mike Kreiner
  Strategie Director National Accounts
  Cree Lighting
  9201 Washington Avenue
  Racine, WI 34406
  Office: 262-504-5037

- Cell: 224-250-1561

#### mike kreiner@cree.com <mailto:mike kreiner@cree.com>

#### 6. CAUTION!

6. CAUTION!

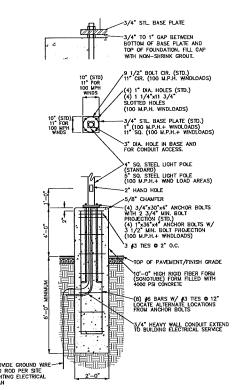
This site contains existing underground public and private utilities. See the project plans for locations of the known existing and proposed underground utility locations. Existing utility information provided on the project plans may be from information disclosed to this firm by the Utility Companies, Local, County or State Agencies, and/or various other sources. No guarante is given as to the completeness or accuracy thereof. The Site Electrical Contractor shall contact the 811 Underground Public Utility Locating System or other appropriate local underground utility locating Agency, a minimum of three (3) working days prior to performing the site electrical work. The Site Electrical Contractor shall field locate all potential utility conflicts and take the necessary precautions to avoid damage to the existing underground utilities. Any damage to the existing underground utilities and year damage to the existing underground utilities. Any damage to the existing underground utilities and year damage to the criticing underground utilities. Any damage to the existing underground utilities and year damage to the criticing underground utilities. Any damage to the Appropriate Utility Provider. If the existing site conditions create a conflict and/or prevent the Site Electrical Contractor from performing the site electrical work, then contact the Engineer of Record.

7. Any existing light pole that cannot be salvaged due to damage, corrosion and/or other conditions shall be replaced. Replacement poles shall be CREE Straight Steel Poles 26'x4', unless directed otherwise by CREE. The Contractor shall field measure the boil pattern of the existing hase and order the replacement pole with the correct base installed. The Contractor shall provide ALDI with an alternate bid price per pole to remove an existing pole and install a new replacement pole in lieu of refinishing. The bid price shall include a credit for light pole refinishing.

8. Any existing anchor bolts, within an existing light pole base that is to remain, that are in need of replacement due to corrosion, damage and/or other circumstances, shall be removed and replaced with retrofit anchor bolts (Hilli or equal) in accordance with the light pole and anchor bolt manufacturer's specifications and recommendations. The Contractor shall provide ALDI with an alternate bid price per anchor bolt to remove an existing anchor bolt from an existing light pole base and install a retrofit anchor bolt.

9. Any existing light pole bases that cannot be salvaged due to damage, corrosion and/or other conditions shall be replaced. Replacement bases shall be constructed in accordance with the specifications provided on the project plans. The Contractor shall provide ALDI with an alternate hid price per base to remove an existing light pole base and install a new cast in place concrete light pole base in accordance with details provided on

10. No Alternate Work shall be performed without written receipt of authorization from ALDI to complete said alternate work.



#### LIGHT POLE BASE STANDARD DETAIL

ROTES

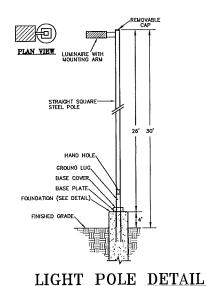
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OF SOLS WITH SOILS REPORT.

- 2. FOUNDATIONS SHALL EXTEND BELOW FROST DEPTH PER LOCAL CODES.
- 3. CONCRETE SHALL HAVE WIN 3000 PSI COMPRESSIVE STRENGTH AT 28 DAYS.
- 4. SEE SITE LIGHTING ELECTRICAL PLAN WITHIN ARCHITECTURAL PLANS FOR ADDITIONAL REQUIREMENTS.
- 5. VERSEY BASE PLATE BOLT PATTERN WITH POLE MANUFACTURER AND / OR SUPPLIER PRIOR TO CONSTRUCTION.







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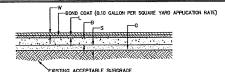
CLIENT ALDI Inc. 2625 N. STOCKBRIDGE RD. WEBBERVILLE, MICHIGAN 48892

(517) 521-3907

SCALE: N/A PROJECT No.: 9173290 WG NAME: 3290 LT

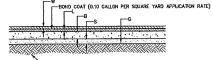
ALDI FOOD MARKET #51 HOWELL, MI

SITE NOTES AND DETAILS



#### STANDARD DUTY BITUMINOUS PAVEMENT CROSS SECTION

KEY	DESCRIPTION	MATERIAL SPECIFICATION	MINIMUM COMPACTED THICKNESS
₩	WEARING COURSE	MDOT 4E3 OR 1100T	1.5"
L	LEVELING COURSE	MDOT 13A	1.5"
8	AGGREGATE BASE	MDOT 21AA (SEE NOTE 4)	8*
s	GRANULAR SUBBASE	MOOT CLASS II (SEE NOTE 4)	12"
G	GEOGRID	N/A	N/A



N/A

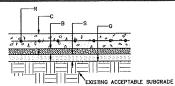
#### HEAVY DUTY BITUMINOUS PAVEMENT CROSS SECTION

		NOT TO SOME	
KEY	DESCRIPTION	MATERIAL SPECIFICATION	MINIMUM COMPACTED THICKNESS
w	WEARING COURSE	MDOT 4E3 OR 1100T	1.5*
L	LEVELING COURSE	MDOT 13A	4*
В	AGGREGATE BASE	MDOT 21AA (SEE NOTE 4)	8"
_		(655 1/675 1/	10*

#### BITUMINOUS PAVEMENT CROSS SECTION NOTES:

G GEOGRID

- The construction specifications of the Local Municipality are a part of this work. Refer to the General Notes and the Bituminous Payement Cross Section Details on the Project Plans for additional requirements.
- 2. The Geotechnical Evaluation Report for the project site is a part of this work. The General Contractor, Earthwork Subcontractor, and s Pavement Subcontractor shall obtain, review, and become familiar with the Ge
- 3. The bituminous pavement cross section specifications are based on typical weather conditions during the June through September Construction Season. If the bituminous parking area and/or bituminous driveways are to be constructed during any other time of the year and/or if weather conditions are unassoanoistly wet, then modifications to the bituminous pavement cross section efficiency are also if the bituminous pavement cross section. necessary. If either of these conditions exists, then contact the Material Testing Engineer and/or the Project Engineer for additional
- 4. The existing granular subbase and aggregate base materials are to be left in place and salvaged for reuse to the greatest extent feasible. The existing aggregate base material shall be finish graded and compacted to a minimum of 93% of the maximum unit weight, Modified Proctor, prior to placement of the bituminous leveling course. In areas of new pavement and in locations where the existing granular subbase and/or aggregate base materials cannot be salvaged, provide the appropriate subbase/base materials in accordance with the Bituminous Pavement Cross Section Details and Specifications provided on the Project Plans. In locations where proposed changes in surface grades require the placement of additional aggregate base material, provide the appropriate aggregate base material in accordance with the Bituminous Pavement Cross Section Details and Specifications provided on the Project Plans.
- 5. Any existing subgrade soils that are exposed during construction procedures shall be prepared in accordance with the Geotechnical Evaluation Report prior to placement of the granular subbase material, including fine grading and compaction to a minimum of 95% of the maximum unit weight, Modified Proctor. Unsuitable soils found within the 1 on 1 influence zone of the propostyment areas, such as muck, peet, toposil, mark, sill or other unstable materials shall be excavated and replaced with structural fill. Structural fill shall be MDOT Class II granular material placed in accordance with the General Notes on the Project Plans and the Geotechnical Evaluation Report. The bituminous pavement subgrade shall be proof rolled in accordance with the General Installed Evaluation after the Material Testing Engineer and/or the Project Engineer shall observe the subgrade proof roll. Areas of subgrade that do not pass a proof roll inspection shall be undercut in accordance with the Subgrade Undercut Notes and Details on the Project Plans. Alternative means of subgrade stabilization may be considered when recommended by the Material Testing Engineer. Alternative methods shall not be ned without receipt of the Owner's Authorization.
- 6. The bituminous payement granular subbase material shall be MDOT Class II sand. No granular subbase material substitutions shall be permitted without prior written approval of the Project Engineer and receipt of the Owner's Authorization. The granular subbase shall be compacted to a minimum of 95% of the maximum unit weight, Modified Proctor.
- 7. The bituminous pavement aggregate base material shall be MDOT 21AA crushed angular limestone or crushed angular natural stone aggregate material. Crushed concrete shall NOT be utilized for the standard or heavy duty bituminous pavement aggregate base. No aggregate base material substitutions shall be permitted without prior written approval of the Project Engineer and test pict of the Owner's Authorization. The aggregate base shall be compacted to a minimum of 95% of the maximum unit weight, Modified Proctor.
- 8. The bituminous pavement leveling course material shall be MDOT 13A bituminous material placed in 1 lift. The bituminous pavement wearing course material shall be MDOT 4E3 or MDOT 1100T bituminous material placed in 1 lift. The bituminous pavement leveling and wearing courses shall NOT be combined into a single course. No bituminous material substitutions shall permitted without prior written approval of the Project Engineer and receipt of the Owner's Authorization. Comparion of the levelings and the project in the description of the leveling course shall be achieved prior to placement of the wearing course. Any sediment, soil, debris and other foreign materials that accumulate on the leveling course shall be removed prior to placement of the wearing course. The bond coat shall be sprayed on leveling course within 24 hours of placement of the wearing course. The bituminous pavement material shall be compacted to a minimum of 93% of the 50-blow Marshall Density.
- 9. Placement of the bituminous pavement leveling course and bituminous pavement wearing course shall be performed in two separate mobilizations. Placement of the bituminous pavement wearing course shall be postponed as directed by the General Contractor and/or the Owner until the majority of the construction net/vitics are complete. Repair of the bituminous leveling course may be necessary du to construction traffic and/or any delay in placement of the bituminous wearing course. The bituminous leveling course shall be epaired as directed by Material Testing Engineer and/or Owner prior to placement of the bituminous wearing cours
- 10. The existing bituminous payement to remain shall be resurfaced where specified on the project plans. In areas where the proposes 10. The existing bituminous pavement to remain shall be resurfaced where specified on the project plans. In areas where the proposed top of pavement elevation is less than 1.5" above the existing top of pavement elevation, the existing bituminous pavement shall be milled to a depth of 1.5" below the PROPOSED top of pavement elevation. In areas where the proposed top of pavement elevation is 1.5" or greater above the existing top of pavement elevation, no milling shall be required. In locations where the proposed top of pavement elevation is 2.5" or greater above the existing top of pavement elevation, a leveling wedge of bituminous pavement leveling course material shall be placed over the existing bituminous pavement in accordance with bituminous pavement revening course. In locations where the compacted thickness of the leveling wedge will exceed 2.5", then the leveling wedge shall be placed in multiple courses not to exceed 2.5" compacted thickness of the leveling wedge will exceed 2.5", then the leveling wedge shall be placed in multiple courses not to exceed 2.5" compacted thickness per course.
- 11. Bituminous mix designs shall be developed in accordance with the MDOT HMA Production Manual. The Contractor shall submit the bituminous pavement mix designs to the Material Testing Engineer for review and approval a minimum of 3 business days prior to use. Bituminous pavement work shall not commence without receipt of the Material Testing Engineer's approval to bituminous mix designs. The bituminous pavement wix design shall be a virgin mix. RAP mixtures shall not be utilized without prior written approval of the Material Testing Engineer and receipt of the Owner's authorization. RAP mixtures, it ambituated without prior written approval of the Material Testing Engineer and receipt of the Owner's authorization. RAP mixtures, it ambitured, with both prior written approval of the Material Testing Engineer and receipt of the Owner's authorization. RAP mixtures, it ambitured, with MDOT Tier I or Tier II RAP Mixture Specifications. In no instance shall MDOT Tier III or non-MDOT RAP

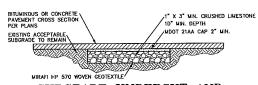


#### HEAVY DUTY CONCRETE PAVEMENT CROSS-SECTION

KEY	DESCRIPTION	MATERIAL SPEC.	MIN. THICKNESS
R	REINFORCEMENT	SEE NOTE 8	SEE NOTE 8
С	CONCRETE	MDOT P1-1A - 6 SACK	8"
В	AGGREGATE BASE	N/A	N/A
s	GRANULAR SUBBASE	MDOT CLASS II	12"
G	GEOGRID	N/A	N/A

#### CONCRETE PAVEMENT CROSS SECTION NOTES:

- l. The construction specifications of the Local Municipality are a part of this work. Refer to the General Notes and the Heavy Duty Concrete Pavement Cross Section Detail on the Project Plans for
- 2. The Geotechnical Evaluation Report for the project site, is a part of this work. The General Contractor, Earthwork Subcontractor and Concrete Pavement Subcontractor shall obtain, review and become familiar with the Geotechnical Evaluation Report.
- 3. The concrete pavement cross section specifications are based on typical weather conditions during the June through September Construction Season. If the concrete pavement areas are to be constructed during any other time of the year and/or if weather conditions are unseasonably wet, then modifications to the concrete pavement cross section specifications may be necessary. If either of these conditions exists, then contact the Material Testing Engineer and/or the Project Engineer for additional
- 4. The existing subgrade soils shall be prepared in accordance with the Geotechnical Evaluation Report. Unsuitable soils found within the 1 on 1 influence zone of the proposed pavennent areas, such as muck, peat, topsoil, marl, silt or other unstable materials shall be excavated and replaced with structural fill. Structural fill shall be MDG1 Class II granular material placed in accordance with the General Notes on the Project Plans and the Geotechnical Evaluation Report.
- 5. The concrete pavement subgrade shall be prepared and proof rolled in accordance with the Geotechnical Evaluation Report. The Material Testing Engineer and/or the Project Engineer shall observe the subgrade proof roll. Areas of subgrade that do not pass a proof roll inspection shall be undercut in accordance with the Subgrade Undercut Notes and Details on the Project Plans. Alternative means of subgrade stabilization may be considered when recommended by the Material Testing Engineer. Alternative methods shall not be performed without receipt of the Owner's Authorization.
- 6. The concrete pavement compacted subbase material shall be MDOT Class II granular material. No subbase material substitutions shall be permitted without prior written approval of the Project Engineer and receipt of the Owner's Authorization. The subbase shall be compacted to a minimum of 95% of the materials are useful. Most of the project the Owner's Authorization. n unit weight, Modified Proctor.
- 7. Concrete material shall be MDOT P1 (I-A) 6.0 sack concrete pavement mixture with a minimum 28-day design compressive strength of 4,000 PSI and 6.5% (+i-1.5%) carrained air. The Contractor shall submit concrete mix design and aggregate mechanical analysis report to the Material Testing Engineer for review and approval prior to use.
- 8. Heavy Duty Concrete Pavement placed within the Truck Well shall be Reinforced with epoxy coated deformed #5 bars at 12" on center each way placed at mid-depth of the concrete, unless noted otherwise the concrete with the concrete win the concrete with the concrete with the concrete with the concr
- 9. Install transverse contraction joints and longitudinal contraction joints at the locations specified on the Project Structural Plans. Joints shall be 2" deep, unless noted otherwise on the Project Structural Plans. Tool joints in fresh connected or saw cut width 4 bours after placement with soft cut saws.
- 10. Provide 1" asphalt fiber control joint between concrete pavement and all other concrete structures such as concrete building foundations, concrete curb and concrete sidewalks.
- 11. The Concrete Payement shall not be exposed to vehicular traffic until the concrete has reached at least 75% of the design flexural strength

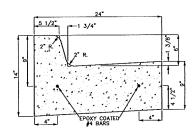


#### SUBGRADE UNDERCUT AND REPLACEMENT CROSS-SECTION

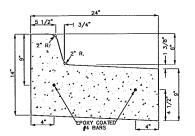
PAVEMENT SUBGRADE UNDERCUT NOTES:

- FAVEMENT SUBJURADE UNDERCUT NOTES:

  A reas of payenent subgrade that do not pass a proof roll inspection shall be undercut when directed by the Material Testing Engineer and/or Project Engineer. All undercut work shall be witnessed and field measured by the Material Testing Engineer and/or Project Engineer. Copies of the field notes depicting the field measurements of the undercut ereas shall be provided to the General Contractor and/or the Materia eld measurements of the undercut as work Subcontractor and ALDI Inc.
- 2. Undercut areas shall be excavated to a depth of 12" below the proposed subgrade elevation using an Excavator or Backhoe with a Smooth Edged Ditching Bucket so as not to searify the underlying soils. Undercut areas shall remain few of all construction traffic and equipment to avoid ruting and/or tracking
- 3. Mirafi HP 570 Woven Geotextile Fabric (or approved equal) shall be placed over all undercut areas per the Manufacturer's specifications. Overlap all seams a minimum of 12<sup>nt</sup> unless specified otherwise by the Manufacturer.
- 4. Backfill the undercut areas with 1" x 3" minimum size crushed angular limestone up to the proposed subgrade elevation. Crushed concrete material shall NOT be substituted for crushed limestone material subgrade elevation. Crushed concrete material shall NOT be substituted for crushed limestone material The backfill material shall be spread with a Wide Track Dozer to minimize loading on the underlying soils. Static roll the backfill material with a large smooth drum roller.
- 5. Construct the appropriate Bituminous or Concrete Pavement Cross Section over the undercut areas pe
- 6. The General Contractor and/or Earthwork Subcontractor shall provide ALDI Inc with unit pricing perform subgrade undercut work per square yard (SY) of undercut area. Undercut Unit Pricing SHAI include excavation, loading, hauling and offsite disposal of excess spoils, placement of generated and the pricing of the p and backfill including all labor, equipment and materials necessary to complete pavement subgrade undercut work as specified on the Project Plans.



#### MDOT TYPE F4 CURB

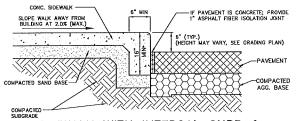


#### MDOT TYPE F4 CURB REVERSE PITCH

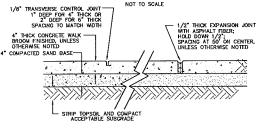
CONCRETE CURB NOTES:

1. Refer to the project plans for the proposed locations of the specific curb

- 2. The construction specifications of the appropriate Local Municipality are a part of this work. Refer to the General Notes and Curb Cross Section a part of this work. Refer to the General Tolling Details on the project plans for additional requir
- 3. Extend the base and/or subbase material of the appropriate adjacent pavement cross-section borizontally to 1 foot behind the back of curb. Concrete curb shall be constructed on no less than 6" of combined depth of
- 4. Concrete material shall be MDOT P1 (I-A) 6.0 sack concrete pavement 4. Concrete material small be POOD 11 (147) 00 583. Consider parameter mixture with a minimum 28 day design compressive strength of 4,000 PSI and 6.5% (+/-1.5%) entrained air. Contractor shall submit concrete mix design and aggregate mechanical analysis report to the Local Municipality and Engineer for review and approval prior to use.
- 5. Install transverse contraction control joints in concrete curb with 1' num depth at 10' on center. Tool joints in fresh concrete or saw cut
- 6. Install transverse expansion control joints in concrete curb as follows: 400' maximum on center, at spring points of intersecting streets and within 10' on each side of catch basins. Transverse expansion control joints shall be 1" thick asphalt fiber joint filler matching entire curb cross section.
- 7. Provide 1" asphalt fiber control joint between back of curb and all other
- Curb Cootractor shall provide final adjustment of catch basin castings in curb line. Castings shall be tuck pointed to structure water tight with concrete or mortar inside and outside of casting.
- 9. Install curb cuts for all existing and proposed sidewalks and pedestria ramps in accordance with the American Disabilities Act and the Barrier Free Design requirements of the appropriate Local, County and/or State Agency. Refer to MDOT Standard Plan R-28, latest revision. Install on uts for all existing and proposed. isting and proposed vehicular ramps and drives as noted nn the



#### SIDEWALK WITH INTERGAL CURB & ISOLATION JOINT DETAIL



#### SIDEWALK CROSS SECTION

SIDEWALK CROSS SECTION NOTES:

- The construction specifications of the Local Municipality are a part of this work. Refer to the General Notes and the Sidewalk Cross Section Details on the Project Plans for additional requirements.
- 2. Sidewalk widths may vary. See the Project Plans for the proposed sidewalk width at each locatio
- 3. The existing subgrade soils shall be prepared prior to placement of the granular subbase. Unsuitable soils found within the 1 on 1 influence zone of the proposed sidewalk areas, such as muck, peat, topsoil, marl, silt or other unstable materials shall be excavated and replaced with structural fill. Structural fill shall be MDOT Class II granular material placed in accordance with the General Notes on the Project Plans.
- 4. The sidewalk compacted subbase material shall be MDOT CL II sand. No subbase material substitution shall be permitted without prior written approval of the Project Engineer and receipt of the Owner's Authorization. The subbase shall be compacted to a minimum of 95% of the maximum unit weight, modified
- 5. Concrete material shall be MDOT P1 (I-A) 6.0 sack concrete pavement mixture with a minimum 28 day design compressive strength of 4,000 P81 and 6.5% (4/-1.5%) entrained air. The Contractor shall submit the concrete mix design and aggregate mechanical analysis report to the Material Testing Engineer and/or Project Engineer for review and approval prior to use.
- 6. Install transverse contraction control joints in accordance with the Sidewalk Cross Section Detail. Space contraction control joints to match sidewalk width, but no greater than 10' on center. Tool joints in fresh
- 7. Install transverse expansion control joints in accordance with the Sidewalk Cross Section Detail. Space expansion control joints at 50 feet on center maximum. Transverse expansion control joints shall be 1/2\* thick asphalt fiber joint filler matching entire sidewalk cross section.
- 8. Provide 1" asphalt fiber control joint between concrete sidewalks and all other concrete structures, such as
- 9. Construct all Barrier Free Sidewalk Ramps in accordance with the American Disabilities Act and the Barrier Free Design Requirements of the appropriate Local, County or State Agency with jurisdiction over the project. Refer to MDOT Standard Plan R-28, latest revision.
- 10. The Concrete Pavement shall not be exposed to vehicular traffic until the concrete has reached at least 75 % of the design flexural strength.

- 1. Contractor shall perform the work in accordance with the requirements of the appropriate Local, County and State Agencies and all other Government and Regulatory Agencies with jurisdiction ov the project. Contractor shall notify the appropriate Agencies in advance of each stage of work in accordance with each Agency's requirements.
- 2. Contractor shall compty with all permit, insurance, licensing and inspection requirements associated with the work. Prior to construction, Contractor and Owner/Developer shall determine who is responsible for obtaining each required permit. Contractor shall verify that the each required permit has been obtained prior to commencement of the stage of work associated with the required permit(s).
- 3. Contractor shall furnish liability insurance and property damage insurance to save harmless the Owner, Developer, Architect, Engineer, Surveyor and Government Agencies for any accident occurring during the construction period. Refer to the appropriate Local, County and State Agencies for additional requirements. Copies of insurance certificatious shall be made available to the
- 4. Contractor shall conduct and perform work in a safe and competent manner. Contractor shall perform all necessary measures to provide for truffic and pedestrian safety from the start of work and through substantial completion. Contractor shall determine procedures and provide safety equipment such as traffic controls, warning devices, temporary pavement markings and signs an needed. Contractor shall comply with the safety standards of the State Department of Labor, the occupational health standards of the State Department of Labor, and appropriate Local, County, State and Federal Agencies. Refer to the safety specifications of the appropriate Local, Agencies. The Contractor shall designate a qualified employee with complete jobs tis authority over the work and safety precautions; said designated employee shall be on site at all times during the work.
- 5. Contractor shall coordinate scheduling of all work in the proper sequence, including work by Subcontractors. Additional costs due to improper planning by Contractor or work done out of sequence as determined by standard acceptable construction practices, shall be Contractor's
- 6. Contractor shall contact the 811 Public Underground Utility Locating System or other appropriate local underground utility locating Agency, a minimum of three (3) working days prior to construction. Existing utility information on the project plans may be from information disclosed to this firm by Utility Companies, Local, County or State Agencies, and/or various other sources. No guarance is given as to the completeness or accuracy thereof. Prior to construction, locations and depths of all existing utilities (in possible conflict with the proposed improvements) shall be verified in the field.
- 7. Contractor shall coordinate scheduling a Pre-Construction Meeting with Engineer prior to
- 8. The Local Municipality, County and/or State in which the project is located may require an Engineer's Certification of construction of the proposed site improvements. Contractor shall verify the certification requirements with Engineer prior to commencement of work. Contractor shall coordinate construction staking, testing, documentation submittal and observation with the appropriate Agency, Surveyor and/or Engineer as required for Engineer's Certification and Government Agency Acceptance. All materials used and work done shall meter of secree due requirements of certification and acceptance, the contract documents and the material specifications noted on the project plans. Any materials used or work done that does not meet said requirements, contract documents and/or specifications shall be replaced and/or redone at Contractor's expense. The Owner/Developer may wait for test results, certifications and/or Agency reviews prior to accepting work. ments. Contractor shall verify the
- 9. Engineer may provide subsurface soil evaluation results, if available, to Contractor upon reque-9. Engineer may provide subsurtace soli evaluation results, it availates, it obtained to plan request. Subsurface soil evaluation results, soils maps and/or any other documentation does NOT guarantee existing soil conditions or that sufficient, acceptable on-site granular material is available for use as structural fill, pipe bedding, pipe backfill, nod whabase or use as any other granular material specified on the project plans. On-site granular material that meets or exceeds the material specifications noted on the project plans may be used as structural fill, pipe bedding, pipe backfill and/or road subbase material. On-site granular material shall be stockpiled and tested as acceptable to the appropriate Agency and/or Engineer prior tn use.
- 10. During the performance of their work, Contractor shall be solely responsible for determining soil conditions and appropriate construction methods based on the actual field conditions. Contractor shall furnish, install and maintain sbeeting, shoring, bracing and/or other tools and equipment and/or construction techniques as neceded for the safety and protection of the workers, pedestrians and vebicular traffic and for protection of adjacent structures and site improvements.
- 11. Contractor shall install temporary and permanent soil crosion and sedimentation control devices at the appropriate stages of construction in accordance with the appropriate regulatory Agencies. Refer to Soil Erosion and Sedimentation Control Plans and Notes on the project plans.
- 12. Structural fill shall be placed as specified on the project plans and within the 1 on 1 influence zone of all structures, paved areas and other areas subject to vehicular traffic. Structural fill shall be placed using the controlled density method (12" maximum lifts, compacted to 95% maximum unit weight, modified proctor). Fill material shall meet or exceed the specifications noted on the project plans or as directed by Engineer when not specified on the project plans.
- 13. All existing monuments, property coroers, ground control and benchmarks shall be protected and preserved, and if disturbed by Contractor, shall be restored at Contractor expense. Contractor shall notify Surveyor of any conflicts between existing monuments, property corners, ground control and/or enchmarks and the proposed site improvemen
- 14. Contractor shall notify Owner/Developer and Engineer immediately upon encountering any field conditions, which are inconsistent with the project plans and/or specifications.
- 15. When noted on the project plans for demolition and/or removal. Contractor shall remove existing es, building and debris and recycle and/or dispose of in accordance with Local, County, State
- 16. Contractor shall remove excess construction materials and debris from site and perform restoration in accordance with the project plans and specifications. Olipsosing of excess materials and debris shall be performed in accordance with Local, County, State and Federal regulations.
- 17. Construction access to the site shall be located as acceptable to the Owner/Developer and to the appropriate Local, County and/or State Agency with jurisdiction over the road(s) providing access to the site. Construction access shall be maintained and cleaned in accordance with the appropriate Local, County and/or State Agencies and as directed by Owner/Developer and/or Engineer.
- 18. Contractor shall take necessary precautions to protect all site improvements from heavy and construction procedures. Damage resulting from Contractor actions shall be repaired at Contractor's expense.



(810) 227-9533 CIVIL ENGINEER 3 WORKING DAYS BEFORE YOU DIG CALL 811 OR 1-800-482-71 (TOLL FREE) LAND SURVEYORS BRIGHTON, MICHIGAN 4811 OR WEST CALLESS COL

SCALE: NO SCALE

REVISION-DESCRIPTION DATE REVISION# DATE REVISION-DESCRIPTION EVISION# DESIGN: FAE DRAFT: SES CHECK: CAG

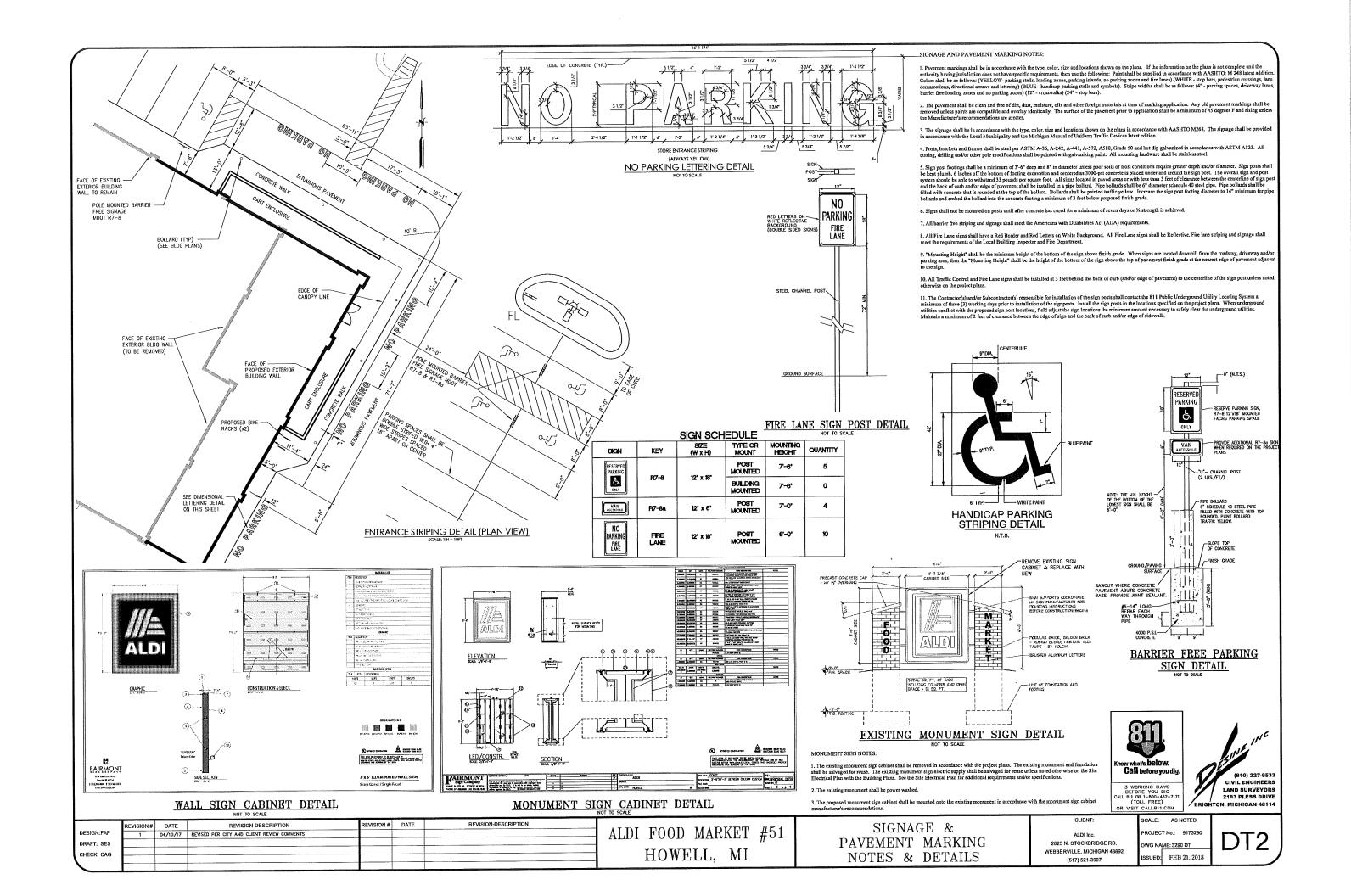
ALDI FOOD MARKET #51 HOWELL, MI

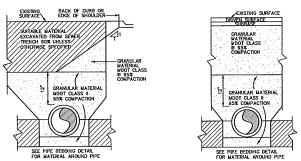
SITE PAVEMENT NOTES & DETAILS

ALDI Inc. 2625 N. STOCKBRIDGE RD. WEBBERVILLE, MICHIGAN 48892

CLIENT:

WG NAME: 3290 DT SSUED: JAN 29, 2018





TRENCH A - PIPE UNDER OR WITHIN INFLUENCE OF DRIVEN SURFACE
NOT TO SCALE

NOTES: 1. COMPACTION PRESENTED AS STANDARD PROCTOR VALUES. 2. SOIL TYPES

RANGEL SANDY (SW)

GRAVEL SANDY (SW)

SANDY SELTY (ILL)

SALDY SELTY (ILL)

SALDY SELTY (ILL)

AS, A6, A7

FOR INTERIOR AND LOWER BET MALES , WATERIALS AROUND THERWO. PLASTIC PIPE WITH DIAMETER 6 INCHES SHALL PASS 0.5 INCH SEVE. WATERIALS AROUND OTHER PIPES SHALL PASS 1.5 INCH SEVE. EXISTING SURFACE OVERFILL



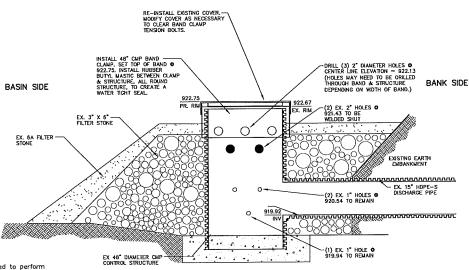
(DRIVEN SUFFACE IS DRIVENAY, PARKING AREA, ROAD BED OR SHOULDED TRENCH B - PIPE NOT UNDER DEIVEN SURFACES
NOT TO SCALE

-- 6" THICK MEXIT CONCRETE (8"x8" square) HOX MOOT-35P1-CONCRETE (8's5' equate) - PAVEMENT PER CROSS-SECTION CATCH BASIN COLLAR

TRENCH DETAILS

SEWER AS CALLED

PIPE REDDING DETAIL
NOT TO SCALE



Any existing filter stone removed to perform structure modifications shall be replaced upon completion of structure modifications.

CS-101 NOTES:

(8" MBL F ROCK IS FOLKS)

2. Place 1" additional washed MDOT 6A stone over the top of the filter stone to match new rim

3. Crushed concrete is not acceptable for use as filter stone.

4. Contact Engineer if conflict exists.

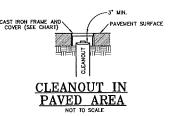
CONTROL STRUCTURE CS-101 MODIFICATION DETAIL

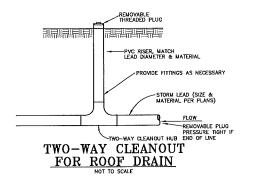
CROSS-SECTION

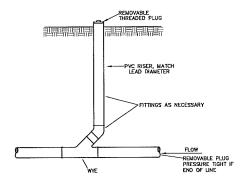
CLEANOUT DIA. FRAME AND COVER

4\*-8\* EJW 1578Z-A

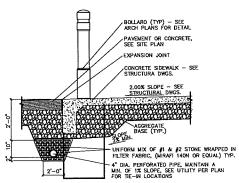
10\*-18\* EJW 1040Z-A-STORM







STANDARD CLEANOUT
FOR ROOF DRAIN
NOT TO SCALE



PERFORATED UNDERDRAIN DETAIL NOT TO SCALE
FOR USE AT BUILDING ENTRANCE AREA ONLY

STORM SEWER NOTES:

- 1. The storm sewer and stormwater management specifications of the Local Municipality are a part of this work. Refer to the General Notes on the project plans for additional information and requirements.
- 2. Storm sewer work shall include clearing of vegetation and tree stumps, stripping and stockpiling of topsoil for reuse, excavation of pipe trench, placement of pipe bedding, placement of pipe and structures including castings, connection to existing structures, tuck pointing of structures, backfill of pipe trench, compaction of backfill, finish grading to provide positive drainage to structures, adjustment of castings to match finish grade, topsoil placement, seed & mulch, site cleanup and restoration, and other storm sewer related work as shown on the project plans and specifications.
- 3. Existing and proposed grades shown in profile view, when provided on the project plans, may be in relation to the centerline of road or item other than the centerline of pipe. The pipe lengths and grades shown in profile view on the project
- 4. RCP when shown on the project plans shall be reinforced concrete pipe and shall conform to the specifications for reinforced concrete pipe per ASTM C76. RCP pipe joints shall be hell-and-spigot with rubber gaskets conforming to ASTM C433. Non-gasketed joints shall only be utilized when authorized by the Owner, Engineer AND Municipality. Non-gasketed joints of pipe having a diameter of 30 inches or greater shall be tuck-pointed on the inside with cementar after the backfill process is complete. Install reinforced concrete end sections incidental to work. Saw cut pipes to length as needed. When pipe class is not shown on the project plans, provide the following:

  Pipe cover to proposed grade: 0 to 4 feet Class V

  4.1 to 10 feet Class III\*

  10.1 to 18 feet Class IV

18.1 feet and greater Class V

\* Use Class IV under paved surfaces

5. CMP when shown on the project plans shall be corrugated metal pipe and shall conform to the specifications for corrugated metal pipe per AASHTO Designation M36. CMP shall be 16-guage steel minimum for 24 inch diameter or smaller and 14-guage steel minimum for 30 inch diameter or greater. Install galvanized steel end sections and connection bands, incidental to work. Connection bands for CMP pipe joints located under paved surfaces shall be gasketed couplers. Saw cut

6. HDPE - Type S when shown on the project plans shall be high density polyethylene pipe with a smooth interior and shall conform to the specifications for high density polyethylene pipe per AASHTO Designation M252 Type S for pipes of 3\* to 10\* diameter and per AASHTO Designation M254 Type S for pipes of 10\* to 60\* diameter. HDPE - Type S pipe joints shall be bell-and-spiget type conforming to ASTM D3212 with rubber gaskets conforming to ASTM F477. Tamp backfill at spring line of HDPE - Type S pipe. Install high density polyethylene end sections incidental to work. Saw cut pipes to length as

- 7. HDPE Type C when shown on the project plans shall be high density polyethylene pipe with a corrugated interior and shall conform to the specifications for high density polyethylene pipe per AASHTO Designation M292 for pipes of 3" to 10" diameter and per AASHTO Designation M294 for pipes of 12" to 60" diameter. HDPE Type C pipe joints shall belil-and-spigot type conforming to ASTM D3212 with rubber gaskets conforming to ASTM F477. Tamp backfill at spring line of HDPE Type C pipe. Install high density polyethylene end sections incidental to work. Saw cut pipes to length as
- 8. CPVC when shown on the project plans shall be corrugated polyvinyl chloride pipe and shall conform to the specifications for corrugated polyvinyl chloride pipe per ASTM F794 and F949. CPVC pipe joints shall be bell-and-spigot type conforming to ASTM D3212 with rubher gaskets conforming to ASTM F477. Tamp backfill at spring line of CPVC pipe. Install high density polyethylene end sections incidental to work. Saw cut pipes to length as needed.
- 9. PVC when shown on the project plans shall be polyvinyl chloride pipe and shall conform to the specifications for polyvinyl chloride pipe per ASTM D2751, maximum SDR of 26. PVC pipe joints shall be bell-and-spigot type conforming to ASTM D3212 with rubber gaskets conforming to ASTM F477 or solvent welded type conforming to ASTM D2564. Tamp backfill at spring line of PVC pipe. Saw cut pipes to length as needed.
- 10. Concrete storm structures shall be pre-cast and shall conform to the specification of pre-cast concrete structures per ASTM C478. Joints of concrete storm structure sections shall be bell-and-spigot with rubber gaskets conforming to ASTM C433. Brick, concrete block or cast in place storm structures may be substituted for pre-cast storm structures ONLY when authorized by the Owner, Engineer AND Municipality; refer to MDOT standard plan R-1, latest revision. All pile openings in pre-cast structures shall be factory installed and shall include a rubber boot resilient pipe to manhole connector conforming to ASTM C1478-07. All clamps, bands and hardware shall be stainless steel or other non-corrosive material. Provide the appropriate adapter(s) as necessary for corrugated pipe. Pipe to storm structure connections shall be performed in accordance with the rubber boot connector manufacturer's recommendations. All temporary openings and seams in storm structures shall be tuck-pointed watertight with tement mortar. Refer to MDOT standard plan R-2, latest revision, for alternate on-line storm structure details when pipe exceeds 42 inch diameter.
- 11. Tap existing structures as acceptable to the Engineer and Municipality, incidental to work. All temporary openings in storm structures shall be tuck-pointed watertight with cement mortar.
- 12. Backfill all storm sewer in accordance with the Pipe Trench details provided on the project plans. Provide pipe bedding that meets or exceeds both the specifications of the Pipe Trench details on the project plans and the recommendation of the
- 13. When edge drains and/or under drains are shown on the project plans, connection to storm structures is incidental to work
  During storm sewer construction, install first 10 linear feet of edge drain and/or under drain from the storm structures in each Specified direction and install temporary cap at end. Complete installation of edge drain following preparation of the subgrade when under paved surface or following finish grade when not under paved surface.
- 14. Install removable plugs in storm sewer stubs as acceptable to Engineer and Municipality, incidental to work. Mark the end of all storm sewer stubs with a 2" x 4" wooden stake extending a minimum of 12" above finish grade, incidental to work.
- 15. Storm structure eastings shall be coated with water based asphaltic paint by the manufacturer. Seams and temporary openings between storm structures and eastings shall be tuck-pointed water tight with cement montar. Coordinate correct curb box/bood/"T" back as needed to match curb profile. See easting schedule on project plans for additional requirements.
- 16. Provide 3.5' minimum cover from the top of pipe of all roof drain pipes to the proposed finish grade when site conditions allow. When pipe cover is less than 3.5', install 2" thick by 24" wide Styrofoam insulation centered over the top of pipe at 12" above top of pipe or as required by the Local Municipality.





CLIENT: CALE: NO SCALE ALDI. Inc. 2625 N. STOCKBRIDGE RD.

WEBBERVILLE, MICHIGAN 48892

ISSUED: FEB 21, 2018

REVISION-DESCRIPTION EVISION# DATE REVISION-DESCRIPTION DATE DESIGN:FAF 02-21-18 REVISED PER TOWNSHIP REVIEW COMMENT DRAFT: SES CHECK: CAG

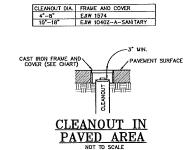
ALDI FOOD MARKET #51 HOWELL, MI

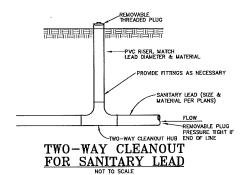
STORM SEWER NOTES & DETAILS

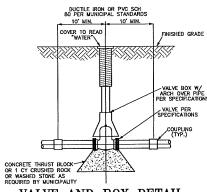
# COLF, EMERGENCY VEHICLE CIRCULATION PLAN

#### SANITARY SEWER LEAD NOTES:

- The Local Plumbing Code and sanitary sewer specifications of the M.H.O.G. Sewer and Water Authority are a part of this work. Refer to the General Notes on the project plans and the M.H.O.G. Standard Details for additional information and requirements.
- 2. Sanitary Sewer Leads shall be PVC pipe conforming to ASTM D3034, maximum SDR of 23.5. Pipe joints shall be push on bell-and-spigot type joints conforming to ASTM D3212 with factory installed flexible elastomeric gaskets conforming to ASTM F477. Solvent exemented joints shall only be used when noted on the project plans for specific applications and shall conform to ASTM D2855. Provide pipe diameter and slope per project plans.
- 3. Contractor shall field locate all existing utilities prior to work. Contractor shall provide all bends and fittings as needed, incidental to work, to install the sanitary sewer leads and to provide the required clearance between the sanitary sewer leads and all existing and proposed utilities while maintaining the proposed minimum pipe slope and proposed lead end invert elevation. Contractor shall notify the Engineer immediately of any utility crossing conflicts.
- 4. Provide 4.0' minimum cover from the top of the sanitary sewer lead pipe to the proposed finished grade when site conditions allow. When pipe cover is less than 40', install 2" thick by 24" wide Styrofam installation centered over pipe at 12" above top of pipe or as required by Local Code. Backfill all sanitary sewer leads in accordance with the trench details on the project plans.







VALVE AND BOX DETAIL

NOT TO SCALE

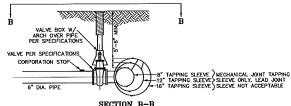
- NOTE:

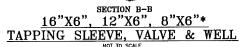
  1. FOR PLASTIC PIPE, WRAP TRACER WIRE AROUND TOP OF VALVE BOX.

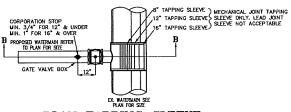
  2. INSTALL COUPLINGS/FITTINGS AS NEEDED.

  3. PROVIDE MECHANICAL JOINT RESTRAINTS FOR ALL GATE VALVES.

- The water main specifications of the M.H.O.G. Sewer and Water Authority are a part of this work. Refer the General Notes on the project plans and the M.H.O.G. Standard Details for additional information and
- 2. DIP shall be ductile iron pipe conforming to ANSI/AWWA C151/A21.51-02. DIP shall have a cement lining conforming to ANSI/AWWA C104/A21.4-95. DIP shall be class 52 unless noted otherwise on the project plans. Provide polyethylene wrap is accordance with M.H.O.G. standards and specifications.
- 3. Fittings shall be cast ductile iron Class 52, or Pressure Class 350 and shall conform to ANSI/AWWWA C153/A21.53-00 and shall have a cement lining conforming to ANSI/AWWA C104/A21.4-95.
- 4. Joints shall push-on type conforming to ANSI/AWWA C111/A21.11-00. Sealing gask and lubricants shall be in accordance with the pipe manufacturer's specifications.
- 5. Provide approved mechanical restraint systems at all bends of 11.25 degrees or greater, tees, crosses and bydrant shoes in accordance with the M.H.O.G. Standard Details. Thrust blocks shall only be utilized when uthorized by M.H.O.G..
- Water gate main valves shall be nonrising-stem, resilient-seated gate valves with box, conforming to AWWA C515-01, 250 psig minimum working pressure rating with interior coating conforming to AWWA C59. All gate valves shall open counter clockwise (left).
- 7. Fire Hydrant Assemblies shall include all necessary piping and fittings for a complete assembly. Fire hydrants shall conform to AWWA C502. Fire hydrants shall be East Jordan Iron Works 5BR-250 Waterm with an integrated 5" Storz coupling or as otherwise specified by M.H.O.G. and/or the Brighton Area Fire Authority. Provide 5'-6" minimum cover.
- 9. Connect to existing water mains in accordance with the project plans and M.H.O.G. standards and specifications. Provide all materials and labor required for a complete waterlight connection, incidental to work. Taps to existing water main shall be performed under pressure and without interruption of service.
- 10. Backfill all water main in accordance with the Pipe Trench details provided on the project plans. Provide pipe bedding that meets or exceeds both the specifications of the Pipe Trench details on the project plans and the recommendation of the pipe manufacturer, incidental to work. Provide 5'-6" minimum cover for all water
- 11. Provide 10' minimum horizontal separation and 1.5' minimum vertical separation hetween water main and both sanitary sewer and storm sewer.
- 12. Contractor shall flush, test and chlorinate the water main in accordance with M.H.O.G. standards and







#### PLAN TAPPING SLEEVE

- 2. ALL JOINTS TO BE RESTRAINED PER MUNICIPAL STANDARDS OR TO MANUFACTURER'S SPECIFICATION, WHICHEVER IS CREATER.



OR VISIT CALLBIT.COM



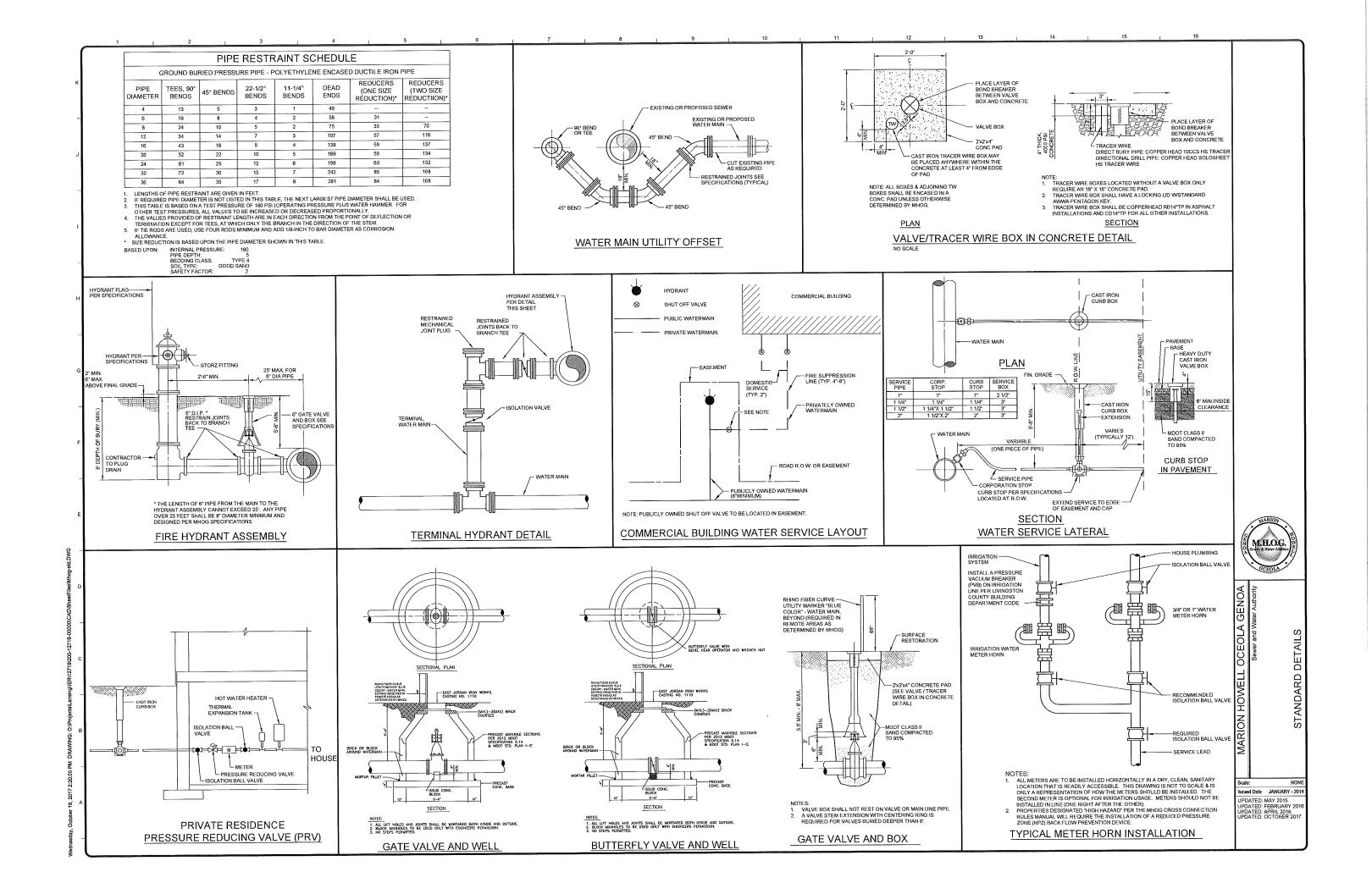
WATER MAIN NOTES & DETAILS CLIENT: CALE: NO SCALE

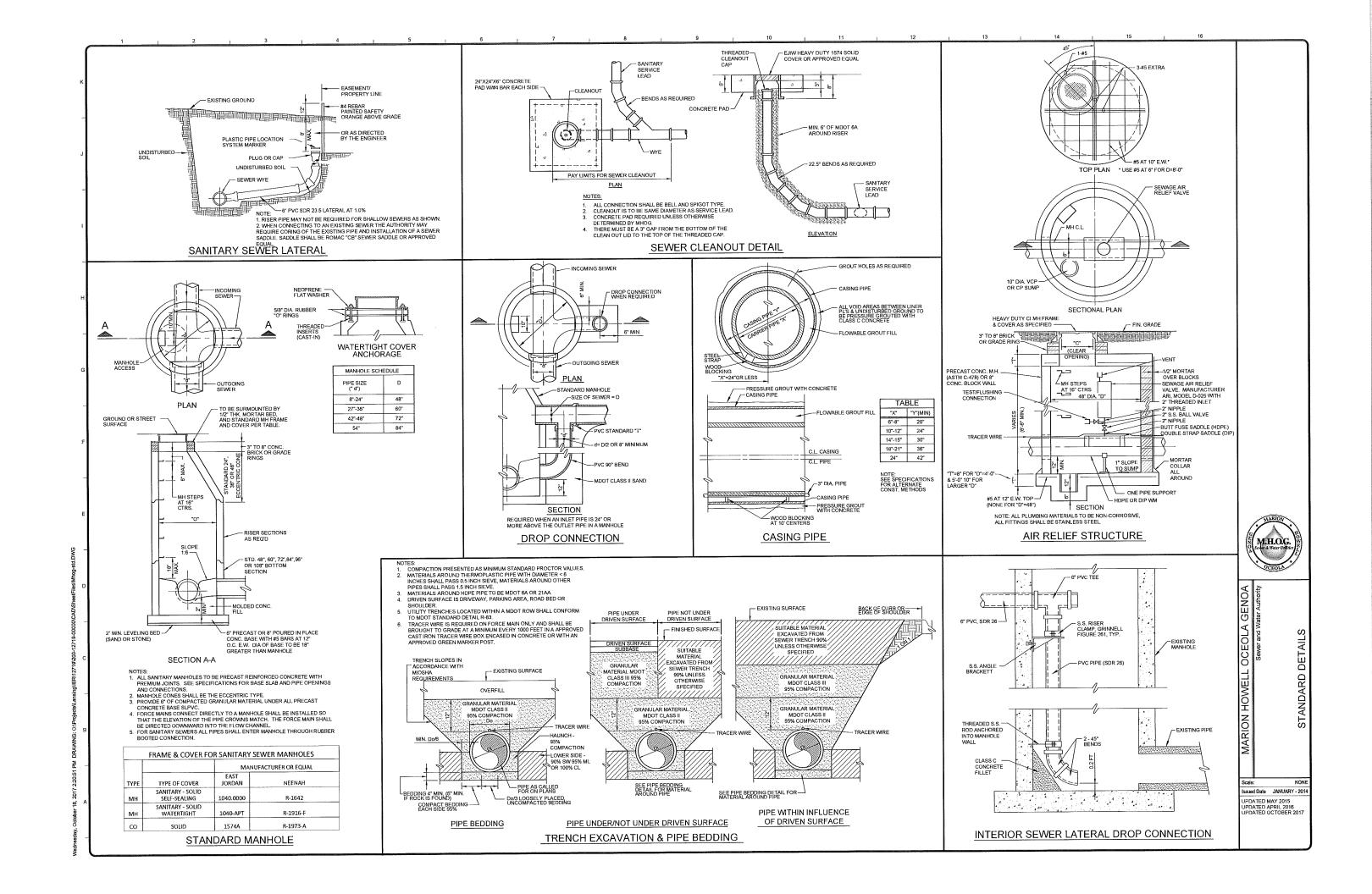
ALDI, Inc. 2625 N. STOCKBRIDGE RD. VG NAME: 3290 DT WEBBERVILLE, MICHIGAN 48892 SSUED: FEB 21, 2018

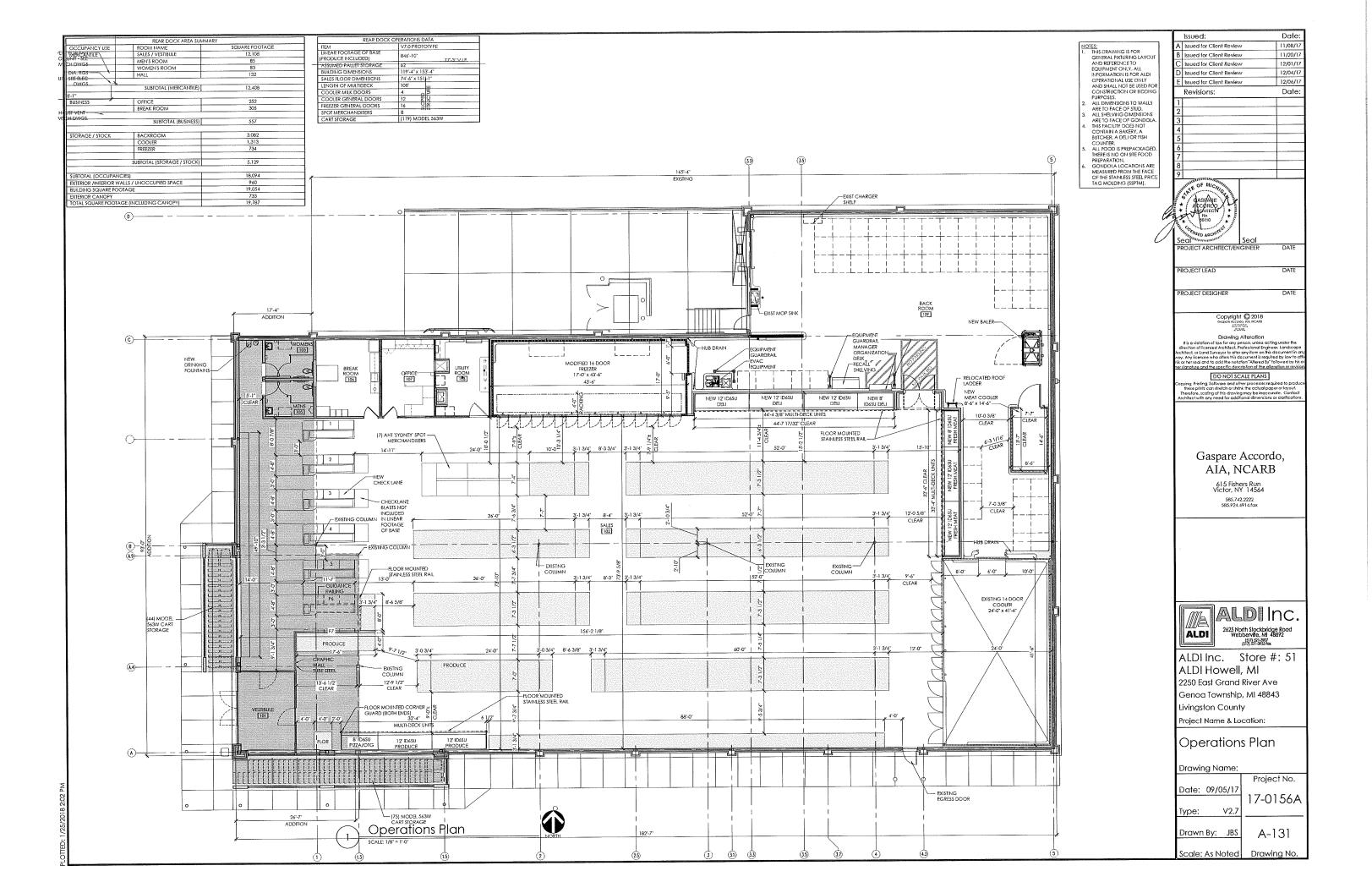
DT4

REVISION-DESCRIPTION DATE FVISION# DATE REVISION-DESCRIPTION ALDI FOOD MARKET #51 DESIGN:FAF DRAFT: SES HOWELL, MI CHECK: CAG

AND SANITARY SEWER





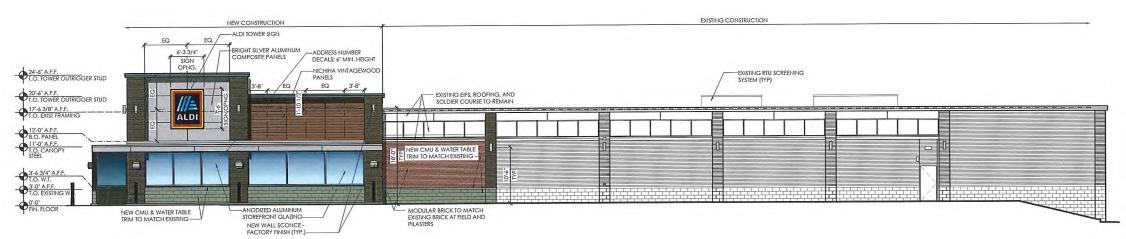


SIGNAGE				
DESCRIPTION	QUANTITY	SQ. FT. PER SIGN	TOTALS	
TOWER SIGN	2	46.6	93.2	
TOTAL SIGNAGE				

Please note that the colors shown here are a graphical representation to show contrast in materials only. Due to the nature of electronic media, colors may vary depending on computer or printer used.

For review of actual colors, a material sample board should be created on a project specific basis.





Front Elevation - East Grand River Rd

Date: Issued: A Issued for Client Review 11/20/13 3 Issued for Client Review 12/01/17 Issued for Client Review 12/04/13 Issued for Client Review 12/06/1 E Issued per Township Com 02/21/18 Revisions: Date:



PROJECT LEAD

PROJECT DESIGNER

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Drawing Alteration

DO NOT SCALE PLANS

Gaspare Accordo, AIA, NCARB

615 Fishers Run Victor, NY 14564



ALDI Inc. Store #: 51 ALDI Howell, MI

2250 East Grand River Ave Genoa Township, MI 48843 Livingston County

Project Name & Location:

Exterior Elevations Drawing Name:

Project No. Date: 09/05/17 17-0156A V2.7 Type:

Drawn By: VLV A-201

Scale: As Noted Drawing No.